Mt Morgans Gold Works Approval Application Supporting Document

Prepared for Mount Morgans WA Mining Pty Ltd

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Contents

Figures

Tables

[redacted]

Project Overview

Mount Morgans WA Mining Pty Ltd, a wholly owned subsidiary of Genesis Minerals Limited (Genesis), is proposing to construct an additional tailings storage facility (TSF) cell to provide ongoing tailings storage. A new landfill within an existing Mt Marven Waste Rock Dump (WRD) is also proposed as the existing Jupiter landfill runs out of capacity.

An Environmental Licence (L9010/2016/1) exists for the Mt Morgans Gold Project (MMGP).

This Works Approval application is seeking approval for construction and time-limited operations for the activity categories shown in **Table 1**.

Table 1: Proposed categories

Category	Activity description	Design capacity (current Licence)	Design capacity (proposed)
Category 5: Processing or beneficiation of metallic or non-metallic ore	Construct new TSF Cell 3 for additional tailings deposition capacity.	3,500,000 tonnes per annum	3,500,000 tonnes per annum (no change)
Category 64: Class II or III putrescible landfill site	Increase the allowable landfill trenching area within Mt Marven and Jupiter waste rock landforms.	4,500 tonnes per annum	4,500 tonnes per annum (no change)

This document provides supporting information and attachments and should be read alongside the Works Approval application form.

4 Attachment 2: Premises Maps

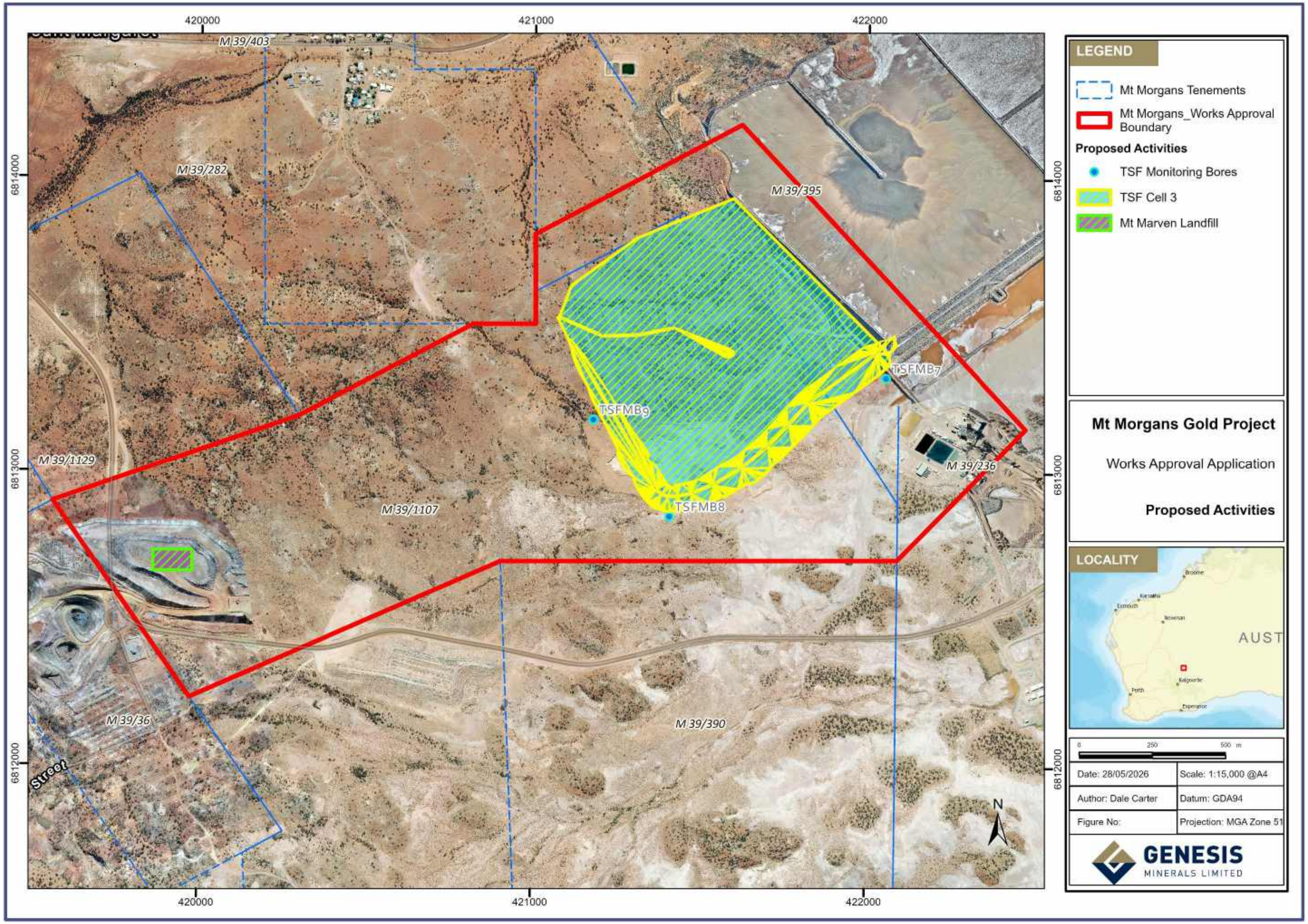

Figure 2: Proposed Activities

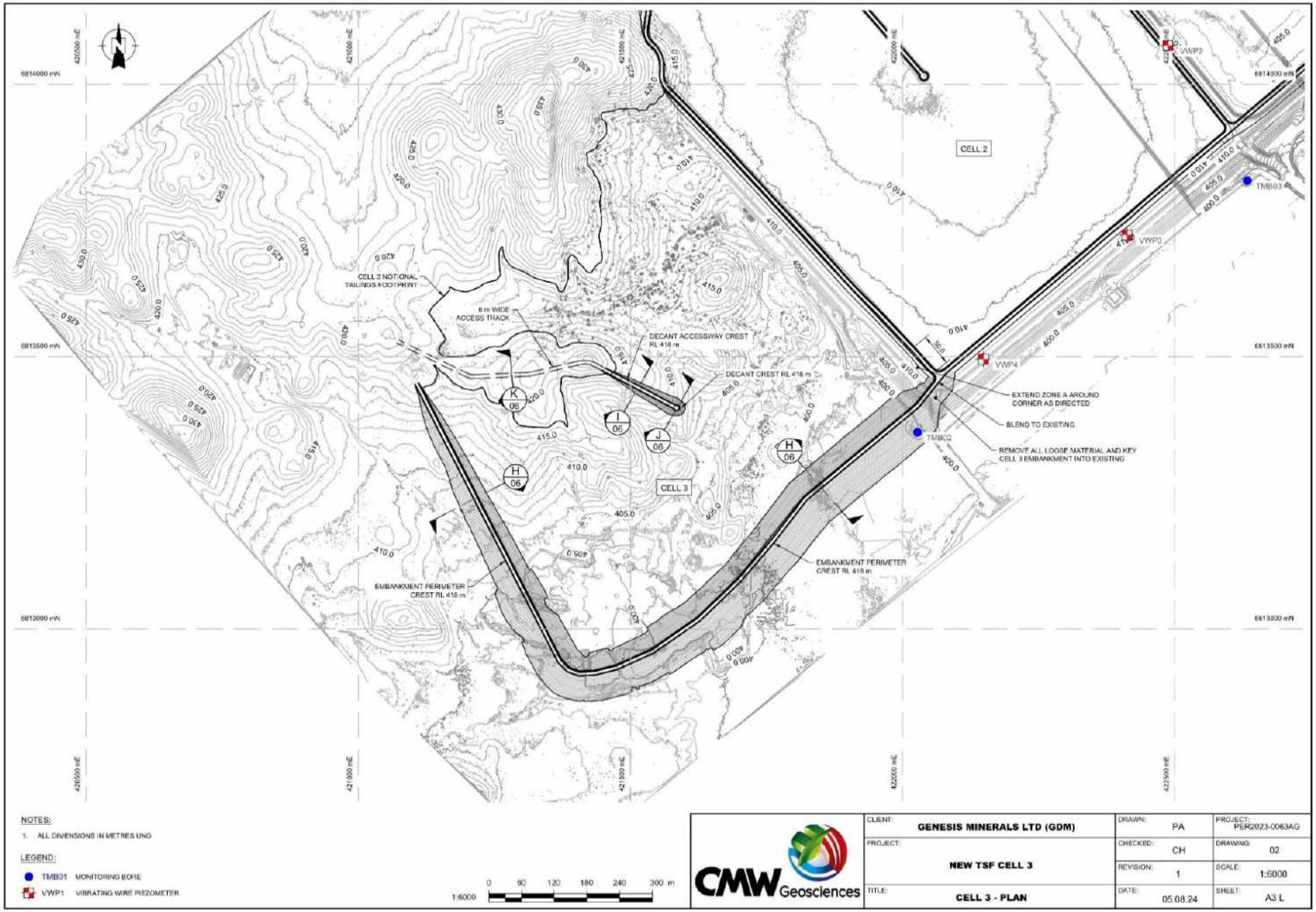

Figure 3: TSF Cell 3 plan

5 Attachment 3B: Proposed Activities

This Works Approval is seeking approval for construction and time limited operations for four categories as listed under Schedule 1 of the *Environmental Protection Regulation 1987*. Description of proposed activities are provided in the following sections with timeframes for construction and commissioning and design capacity summarised in **Table 2** below.

Table 2: Timeframes and design capacity

Item	Response
Estimated operating period of the project / premises (e.g. based on estimated infrastructure life):	25 years
Proposed date(s) for commencement of works:	June 2026
Proposed date(s) for conclusion of works construction:	December 2027
Proposed date(s) for environmental commissioning of works:	December 2027
Proposed date/s for commencement of time limited operations under works approval:	December 2027 – June 2028
Maximum production or design capacity for each category applied for (based on infrastructure operating 24 hours a day, 7 days a week):	Category 5 – 3,500,000 tpa Category 64 – 4,500 tpa
Estimated / actual throughput for each category applied for:	Category 5 – 3,500,000 tpa Category 64 – 4,500 tpa

5.1 Category 5: Ore Processing

Genesis is not seeking any change to the MMGP Processing Plant throughput, licenced to process 3.5 million tonnes per annum (Mtpa). The existing facility consists of a carbon-in-leach (CIL) plant include crushing, grinding, a gravity circuit, carbon-in-leach circuit and a carbon stripping and gold room circuit.

Tailings are discharged to hill side paddock style tailings storage facility (TSF) situated immediately north of the processing plant. The TSF currently consists of two cells (Cell 1 and 2), covering an area of approximately 153 hectares (ha). To support ongoing tailings storage requirements at the mine, Genesis is proposing to construct a new TSF cell (Cell 3) adjacent to the existing cells.

5.1.1 Construction Activities

5.1.1.1 Tailings Storage Facility

Genesis is proposing to construct a new tailings cell (Cell 3) adjacent to the existing TSF to provide ongoing tailings storage at Mt Morgans. The existing TSF consists of two cells (Cell 1 and 2), covering an area of approximately 153 ha. The proposed Cell 3 will abut the south-west wall of Cell 1 and will be constructed as a hill side paddock style TSF, similar to the existing cells (**Figure 3**).

The engineering design for Cell 3 has been prepared by CMW Geosciences and the TSF Design Report (CMW, 2024) is provided as **Attachment 8A.** The following sections describe key design considerations for TSF Cell 3. For more detailed information, refer to the TSF Design Report.

Cell 3 will have a tailings impoundment area of 50 ha and be constructed to a crest RL418 m AHD, equivalent to a maximum embankment height of approximately 19 m, which will provide a storage volume of 4.4 million m^3 (Mm^3) and a capacity of approximately 6.2 Mt. This corresponds to an additional storage life of approximately 2 years and 1 month, assuming a production rate of 3 Mtpa and tailings in-situ density of 1.4 t/m3 (dry), a beach slope of 1% and a minimum embankment freeboard of 0.5 m. This information is summarised in **Table 3.**

Table 3: TSF storage characteristics

Stage	Crest RL (m AHD)	Impoundment Area (ha)	Storage volume (Mm^3)	Storage capacity (Mt)	Cumulative storage life (years)
Cell 3	418	50	4.4	6.2	2.2

5.1.1.1.1 Topography

Topography in the area comprises low hills and ridges surrounded by flat alluvial floodplains. The floodplains drain to a flat salt-lake system (playa) which forms the northern extent of the Lake Carey playa. The surface elevation in the area ranges from approximately 399 m AHD in the playa to about 440 m AHD along the ridges and hills forming the northern boundary of the TSF.

At the proposed location of TSF Cell 3, the surface comprises surficial gravelly deposits on the ridges with sparse spinifex grass vegetation, interspersed by rock outcrops. To the south and east, the playa has no vegetation with a clayey surface and occasional areas of bare rock exposure.

Trees up to 3 m high are present on sand islands in the playa and along the natural creek lines and on south and west facing valley sides.

5.1.1.1.2 Subsurface and Foundation

The subsurface and foundation profiles for the proposed location of TSF Cell 3 were established based on geotechnical investigations carried out by CMW in 2023 (CMW, 2023). These investigations included seventeen test pits with bulk sampling for laboratory testing that comprised materials characterisation tests, compaction tests, laboratory permeability and triaxial tests as well as eighteen boreholes and in-situ permeability tests at six locations.

The ground conditions encountered and inferred from the investigations were considered to be generally consistent with the published geology for the area. Laboratory permeability tests indicated in situ surficial soil permeabilities ranging between 3.5×10^{-9} and 5×10^{-9} m/s when, respectively, the Sandy Silt/Clay and Silty/Clayey Gravelly Sand materials were compacted to 95% of maximum dry densities (Standard). In-situ permeability tests undertaken inside the boreholes drilled during the investigation indicated permeabilities of between 7.0×10^{-7} and 5.3×10^{-6} m/s within the upper 15 m, and 1.1×10^{-6} and 3.2×10^{-5} m/s within the upper 1 m of horizons. This means that with engineered in situ soils it is expected that permeability will achieve approximately 3.5×10^{-9} m/s across TSF3, and that the naturally low permeability of 10^{-6} to 10^{-7} m/s in the first 15 m further reduces the risk of significant seepage.

5.1.1.1.3 Embankments

The perimeter embankment of TSF Cell 3 will be constructed with an upstream slope of 1V:2.0H, a downstream slope of 1V:3H (interim) and a crest width of 7 m. Embankments will be constructed from mine waste sourced from mining operations and existing waste dumps at Mt Morgans, estimates for material volumes provided in **Table 4.**

The embankment zoning comprises an upstream (or inner) zone of compacted clayey borrow or mine waste (Zone 1) which will be a low permeability zone; and a downstream (or outer) zone of traffic compacted mine waste (Zone 2) which provides bulk/strength and buttresses the low permeability upstream zone.

The compacted upstream zone will be a minimum of 5 m. The upstream embankment crest will have a 2% cross-fall towards the upstream side, 1 m (min.) high mine waste windrow at the downstream crest, and above ground tailings pipeline at the upstream crest. The embankment incorporates a cut-off trench excavated to a nominal depth of 1.5 m within the underlying weathered Meta-Basalt in order to reduce seepage losses. A typical cross section of TSF Cell 3 embankments is shown in **Figure 4.**

Table 4: Estimated construction material volumes

Stage	Compacted tailings or clayey borrow / mine waste (m^3)	Mine waste incl. select filter mine waste (m^3)	Mine waste capping for erosion protection (m^3)
Cell 3	127,000	979,830	23,000

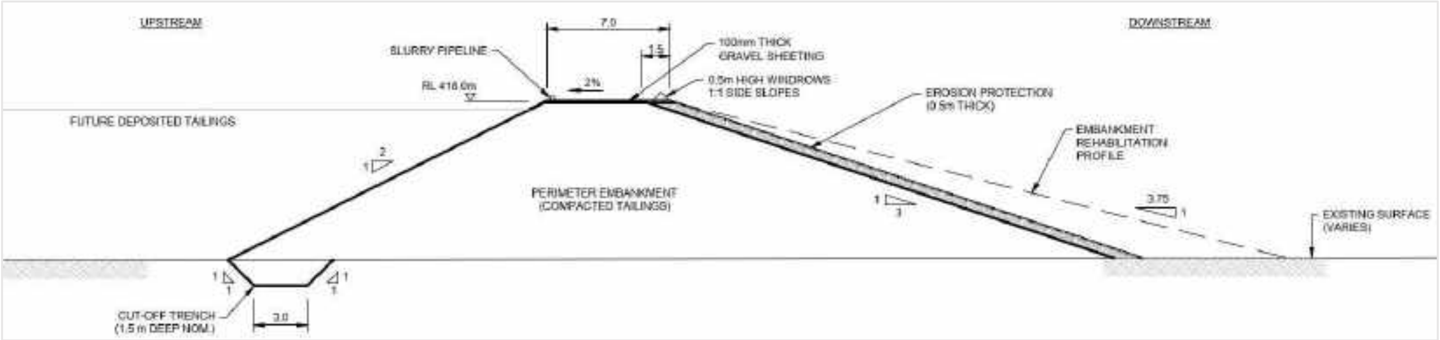

Figure 4: Typical cross-section of TSF Cell 3 perimeter embankment

5.1.1.1.4 Tailings Delivery System

Tailings will be discharged from the perimeter embankment of each cell as a slurry at 45% to 50% solids density. The discharge will be operated through multiple rotating spigots creating a concave tailings surface within which a decant pond will be developed at the centre of the cells. The spigot interval will be approximately 36m and the number of operating spigots and their sequencing will be determined by hydraulic system considerations and beach development objectives. Pipelines around Cell 3 will tie into the existing tailings discharge line at Mt Morgans.

Once the Cell 3 starter embankment construction is completed, the slurry distribution pipework will be assembled on the crest of the embankment and extend around the western side of the cell (**Figure 5**). An access track will be required to be constructed to extend the pipeline through this area and provide access for daily inspections and pipe maintenance.

5.1.1.1.5 Decant

Surface water will be removed from the TSF by a decant pump deployed within a central decant structure in each cell. TSF supernatant water will be pumped directly to the process plant for re-use. The water recovery system, pumps and piping must be designed for a minimum recovery of not less than 100% of the slurry water, including the additional capacity for storm events (8,200 tonnes per day (tpd) for tailings production of 3 Mtpa at 50% solids).

The decant accessways have design slopes of 1(V):1.5(H) and 7 m crest width. The accessways will be constructed using traffic compacted mine waste and sheeted with 0.1 m (nom.) wearing course sheeting. The crests will have 0.5 m (min.) high rock/mine windrows on both sides, with breaks in the windrow on the low side to allow surface water to run off.

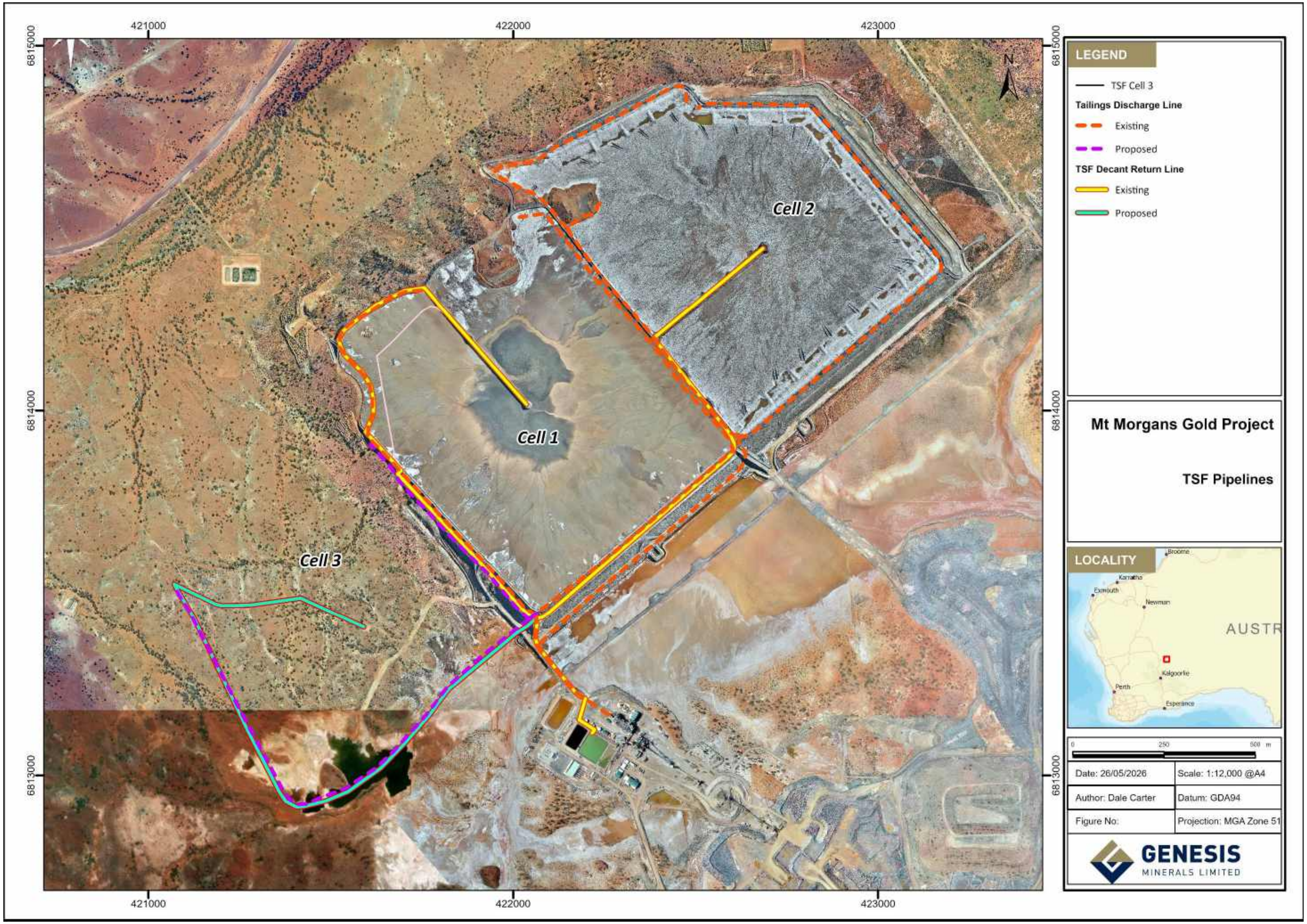

Figure 5: Tailings Pipelines

5.1.1.1.6 Underdrainage

As with the existing TSF Cells 1 and 2, the design for the new TSF Cell 3 includes an underdrainage system. It will comprise an underdrainage line grading to an outfall pipe, which drains into a sump downstream of Cell 3. It will be installed inside the TSF basin 10 m away from the perimeter embankment upstream toe, where the segregation of the tailings is greatest i.e. where coarse-grained tailings would be located thus allowing more leachate into the underdrainage lines.

The underdrainage line grades to an HDPE outfall pipe with an outer diameter of 140 mm. The pipe will be installed within a trench, inside the embankment, 1 m wide and 1 m deep. The trench will be backfilled with compacted dried tailings (upstream zone materials) in order to reduce potential for creating a preferential flow path, sealed with bentonite collars at the location of the cut-off trench and at 10 m away from the downstream of the cut-off trench.

The collection sump will be nominally 10 m wide at the base and 1.5 m high with 1:2 (v:h) side slopes, and it will be lined with HDPE liner. A pump will be deployed at the sump to recover water back onto the tailings beach. A remote water level monitor will be installed for the operation of the sump, and it will include high-level and low-level alarms and switches. Water will then be pumped back from the collection sump to the process plant or utilised in construction.

Sizing of the underdrainage system is based on seepage analyses, also allowing the capacity for an inflow equivalent to 1% of slurry water inflow (i.e. nominally 1 L/s, system capacity).

5.1.1.1.7 Vibrating Wire Piezometers

Three pairs of new vibrating wire piezometers are to be installed around TSF Cell 3. Installation of vibrating wire piezometers (VWP) will need to be developed internally within TSF Cell 3 to enable the phreatic surface within its embankment to be monitored and stability analyses to be validated in the future. The VWP will also give early warning of seepage from the cell.

VWP are to be installed as part of the embankment construction. Each pair will have VWP installed in a trench 0.5 m wide and 0.3 m deep, with one VWP installed at nominally 10 m away from the upstream toe of the embankment, and another VWP directly underneath the upstream crest of TSF Cell 3 embankment. Bentonite collars will be required to reduce the potential for the development of a seepage pathway. The read-out cables are to be installed in PVC electrical conduit to ensure they are not damaged during deposition. A terminal box containing the data loggers is to be placed downstream of the final embankment toe to avoid vehicular damage.

5.1.1.1.8 Monitoring Bores

Three new groundwater monitoring bores are to be installed around TSF Cell 3. The recent geotechnical investigation (CMW, 2023) encountered groundwater locally at depths of approximately 1.9 mbgl and 2.2 mbgl. An allowance should be made for the new TSFMB to be installed to a minimum depth of 20 mbgl for the purpose of performance monitoring. Indicative locations of proposed TSFMB are shown in **Figure 2. T**he originally proposed TSFMB locations from the CWM TSF design report were considered too close to the TSF Cell 3 embankment. Accordingly, these bores have been repositioned to a minimum distance of 50 m from the proposed TSF wall. Bores will be constructed and installed in accordance with Minimum Construction Requirements for Water Bores in Australia, 4th Edition.

The bores are intended to serve a dual purpose by supporting trigger groundwater level monitoring while also being suitable for conversion to seepage recovery bores should trigger water levels be exceeded in any bore.

The construction of TSF Cell 3 would require the decommissioning of TSFMB2, TSFAB03, an existing sump SP01 and seepage intersection trenches to the west-southwest of TSF Cell 1. TSFMB2 and TSFAB03 must be 'sealed' using grout. SP01 and the seepage intersection trenches must be backfilled using compacted dried tailings (upstream materials) to prevent these from becoming a path of least resistance for seepage.

Additionally, should there be open exploration boreholes within the footprint of the proposed TSF Cell 3 location, these must be 'sealed' using the same approach to the decommissioning of TSFMB2 and TSFAB03.

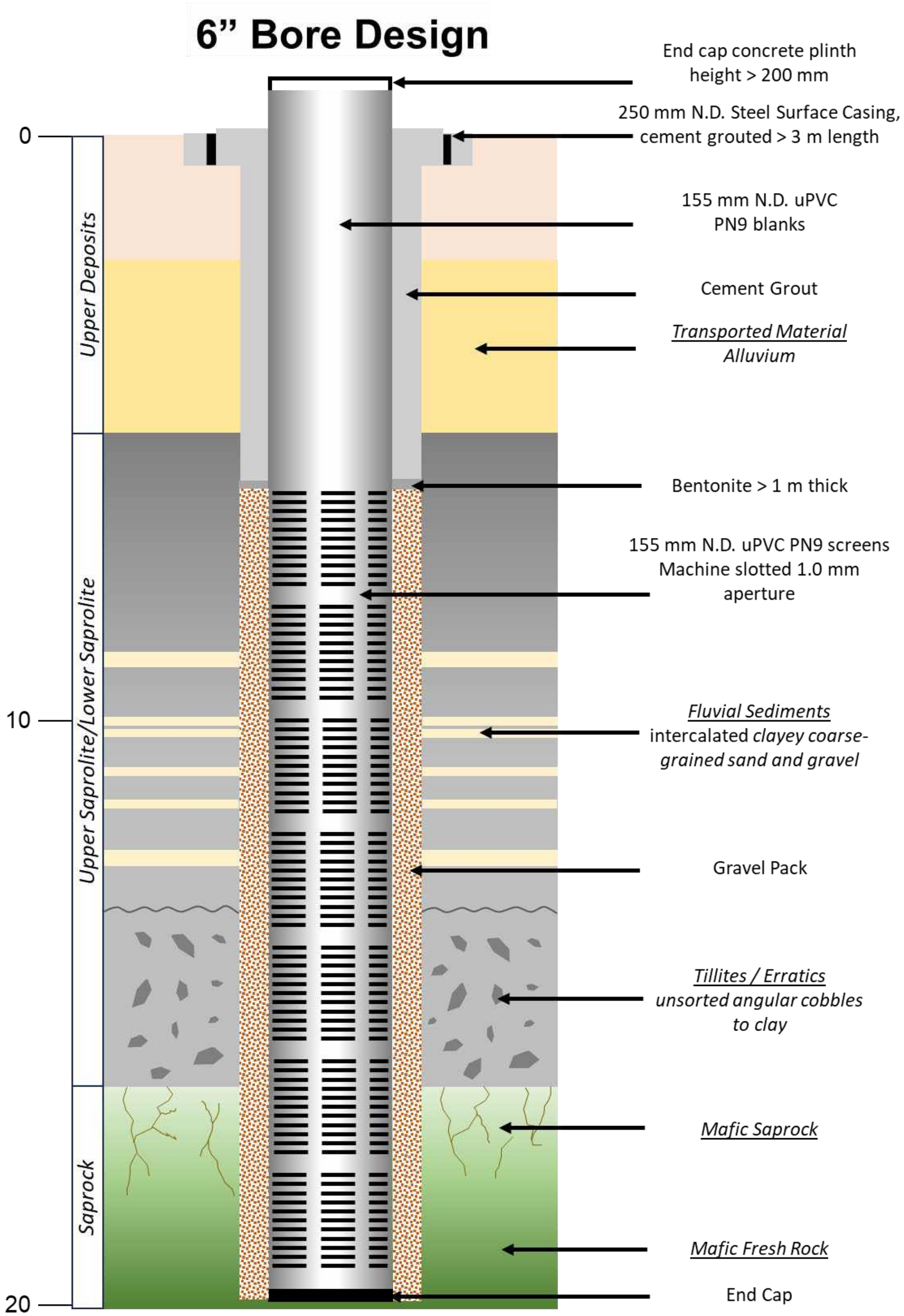

Figure 6: Generalised construction specifications for the MMGO TSF monitoring bore

5.1.1.1.9 Groundwater & Seepage

Two hydro stratigraphic units were identified in the area namely fractured bedrock aquifer associated with the meta-basalt units and aquitard clay units associated with the saprolite, saprock and saturated playa deposits.

The fractured bedrock aquifer is associated with discontinuities within the Meta-Basalt units, which underlies the saprolite and clay deposits. Borehole logs indicate that the Meta-Basalt is "weathered to very weathered" indicating that the fractures might be filled with clay with a suspected low hydraulic conductivity. The fractured bedrock aquifer is considered a poor water resource because of the natural high groundwater salinity due to the proximity of the playa system.

Geotechnical investigations carried indicated that the saturated playa deposits have a very low hydraulic conductivity of about 2.6 x 10^{-8} m/s (0.002 m/d). The assessment also indicated that the saprolite comprises clay, hence the hydraulic conductivity of the saprolite is also likely to be low at about 0.01 m/d. The thickness of the aquitard varies between 13 m at TSF MB1 to 3 m at TSF MB3. Groundwater was encountered at approximately 2.2 mbgl and 1.9 mbgl (RL 396.8 m and RL 398.8 m) in two of the seventeen test pits, and 11.5 mbgl to 13 mbgl (RL 388.15 m to RL 396.05 m) in five of the six borehole locations during the recent geotechnical investigations. Another borehole location had inferred perched groundwater at approximately 0.5 mbgl (RL 398.4 m). It was noted that the two test pits were located on the south of the proposed TSF Cell 3 site, near the playa.

Prior to tailings deposition, groundwater flowed in a south-easterly direction towards the playa lake, which is a groundwater sink. Seepage was noted adjacent to discrete locations adjacent to the southwest corner of Cell 1 and along the southern embankment of Cell 1 in 2019. More recently minor seepage was noted adjacent to the eastern side of Cell 2. In response to this seepage, GRM was engaged to conduct hydrogeological assessments and compile a groundwater management plan. The groundwater management plan outlines trigger levels and groundwater management responses including recovery bores. The plan also outlines ongoing monitoring, responsibilities, reporting and review of the plan.

A TSF Solute Transport Model (Pennington Scott, 2025e) was completed, modelling the potential seepage and mounding pathway from TSF operations. A base seepage rate of 530 m3/day per TSF cell was modelled to assess potential impacts on the surrounding hydrogeological system, including any risks to the Healing Pool.

Modelling identified seepage infiltration creates localised groundwater mounding beneath and immediately surrounding each TSF cell, with groundwater then migrating laterally toward zones of lower hydraulic head. Although TSF seepage (at 50,000 mg/L 120,000 mg/L) is less saline than that of the salt lake receiving environment (250,000 to 300,000 mg/L), it can contain additional solutes such as weak-acid dissociable (WAD) cyanide, nitrates, and trace metals. Metals tend to precipitate out of solution or adsorb to clays and organic matter within short distances of the TSF, whereas conservative (non-reactive) solutes, particularly chloride and nitrate, remain mobile and migrate the furthest along flow paths, with attenuation governed primarily by advection and dispersion processes.

Key findings from the solute transport modelling are summarised as follows:

- Migration direction: Solute transport from TSF Cells 1 and 2 occurs predominantly southward toward the Double Jay and Heffernan's Pits within the Jupiter Pit Complex (**Figure 7**), which functions as the principal hydraulic sink under continued dewatering.
- Hydraulic interception: Toe drains and collection sumps along the southern TSF embankments capture a portion of seepage; however, they do not fully offset the persistent drawdown gradients that drive groundwater flow toward the pits.
- Limited eastward migration: Solute movement toward the Healing Pool is negligible due to the very low transmissivity and subdued hydraulic gradients within the playa sediments and Lower Saprolite, combined with effective local drainage pathways.

During construction of Cell 3, the foundation will be compacted with in-situ clay to improve stability and reduce seepage to groundwater. The underdrainage system will be installed along the southern perimeter embankment, with drainage lines discharging under gravity into an external HDPE lined sump that will be fitted with a remote water level monitor including high-level and low-level alarms. The recovered underdrainage water will then be pumped back into the TSF.

Three pairs of new vibrating wire piezometers are to be installed around TSF Cell 3 to monitor phreatic surface level to manage stability and seepage risks. Three new groundwater monitoring bores are also to be installed around TSF Cell 3. Monitoring will be carried out in accordance with **Table 5** to observe any impacts from seepage. Prior to operating Cell 3, samples will be collected from each bore to establish baseline parameters.

Table 5: Groundwater monitoring

Monitoring Point	Parameter	Units	Frequency	Averaging period
TSFMB08, TSFMB09, TSFMB10	Standing water level	Metres below ground level	Monthly	Spot sample
	pH	pH units	Quarterly	
	Total dissolved solids	mg/L		
	WAD Cyanide	mg/L		
	Arsenic, Antimony, Cadmium, Chromium, Cobalt, Copper, Iron, Lead, Manganese, Mercury, Nickel, Selenium, Sulfate, Zinc, Thallium.	mg/L		

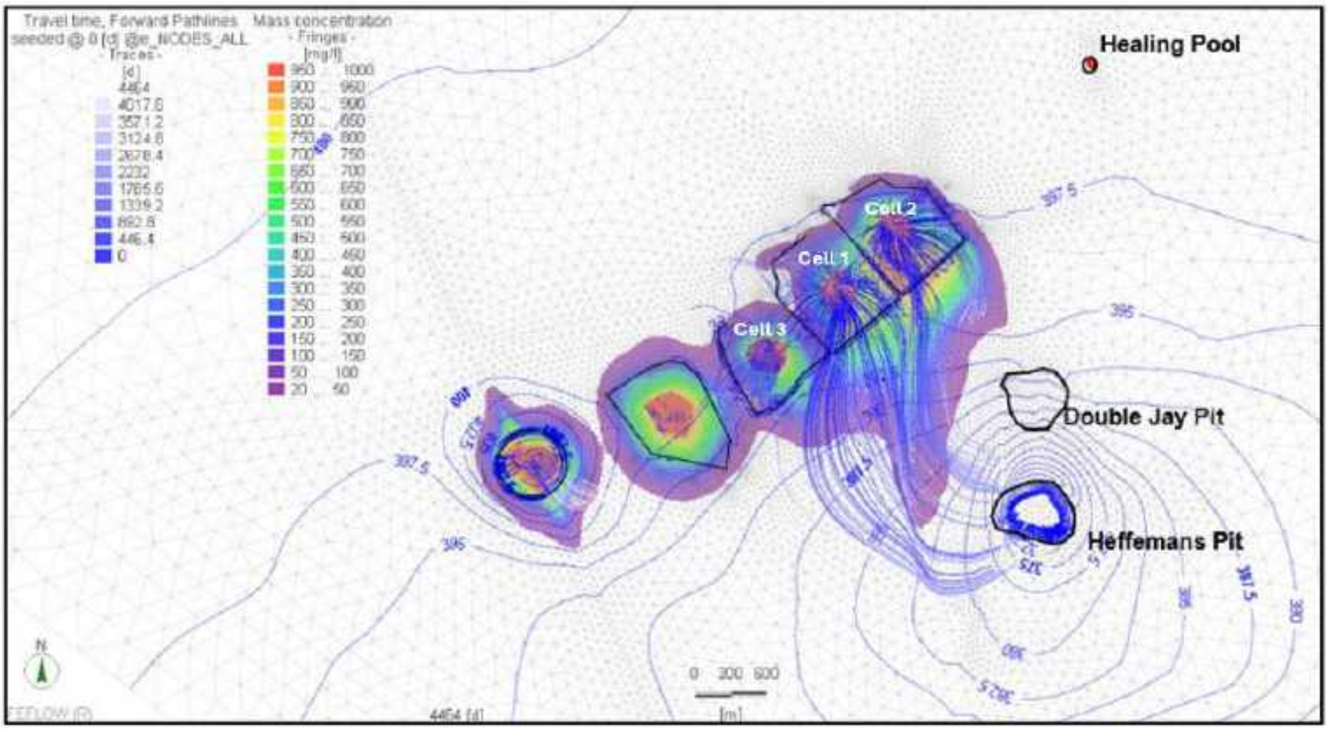

Figure 7: Groundwater levels, contaminant concentrations and pathlines

5.1.1.1.10 Surface Water

The site lies within the Lake Carey catchment and the lake forms the most dominant hydrological feature near the site. The lakeshore is located approximately 2.5 km to the south of the Jupiter prospect and is separated by a banded iron formation ridge approximately 80 m high. All the watercourses and drainage lines in the vicinity of the project are ephemeral, however, flooding may occur during the summer months between January and March when high intensity rainfalls take place.

The results of surface water modelling indicate that the 100-year ARI flood level is approximately 0.5 m above the playa surface or approximately RL398 m. In a extreme event, the Lake Carey level could potentially rise to RL399 m.

Existing flood bunds have been constructed around the existing TSF Cells 1 and 2 to approximately RL398.5 m. Cell 3 has been sized such that it does not block catchment flows from the northwest and the southern embankment is aligned with the Southern TSF Cell 1 and 2 embankments adjacent to the playa lake.

5.1.2 Environmental Commissioning Activities

No environmental commissioning activities are proposed.

5.1.3 Time Limited Operations

To streamline the approvals process and enable proposed activities to commence following construction and commissioning, Genesis requests that the activities are authorised as Time Limited Operations (TLO). The TLO period is requested to be set at 350 calendar days.

Acknowledging that this is longer than standard, it will allow sufficient time for preparation and assessment of a Licence Application and reduce administrative burden for all parties.

5.2 Category 64 Landfill

Genesis is proposing a landfill within the waste rock landform (WRL) at Mt Marven (**Figure 9**) as the existing landfill area at Jupiter WRLs runs out of capacity. The landfill will accept inert waste type 1, putrescible waste, clean fill and inert waste type 2 (tyres) generated on site. All other waste materials will be removed from site for appropriate disposal. The landfill will have a maximum operations capacity of 4,500 tonnes per annum (no change to existing licensed capacity).

5.2.1 Construction Activities

To construct the landfill, a tipping trench with a designated tipping area of maximum 30 m in length, will be established within the existing waste rock landform. Multiple adjacent trenches may be constructed, however only one trench will be opened at a time for waste tipping (**Figure 8**). Waste rock landforms at Mt Marven are approximately 35 m above natural ground elevation at current and may increase as mining progresses. Given landfill trenches will be excavated within the surface of the waste rock landform, there is low risk to groundwater.

Trenches for putrescible wastes will be covered by the end of the month in which the waste was deposited with sufficient quantities of Type 1 inert waste, clean fill or other appropriate cover material to prevent wind-blown waste as well as the spread of fire and disease vectors.

The landfill facility will be fenced with 15-150-15 farm fencing erected to a height of 1.8 m, incorporating one row of barbed-wire along the top, to deter stock access and contain wind-blown waste. A lockable 6 m wide gateway will be installed to control vehicle access. At the entrance gate a fixed sign will clearly identify the facility as a landfill, specify the waste streams permitted, and highlight the prohibited waste types. These controls ensure operational clarity, security and compliance with the statutory requirement that waste is contained within the site boundary and wind-blown material is prevented from escaping.

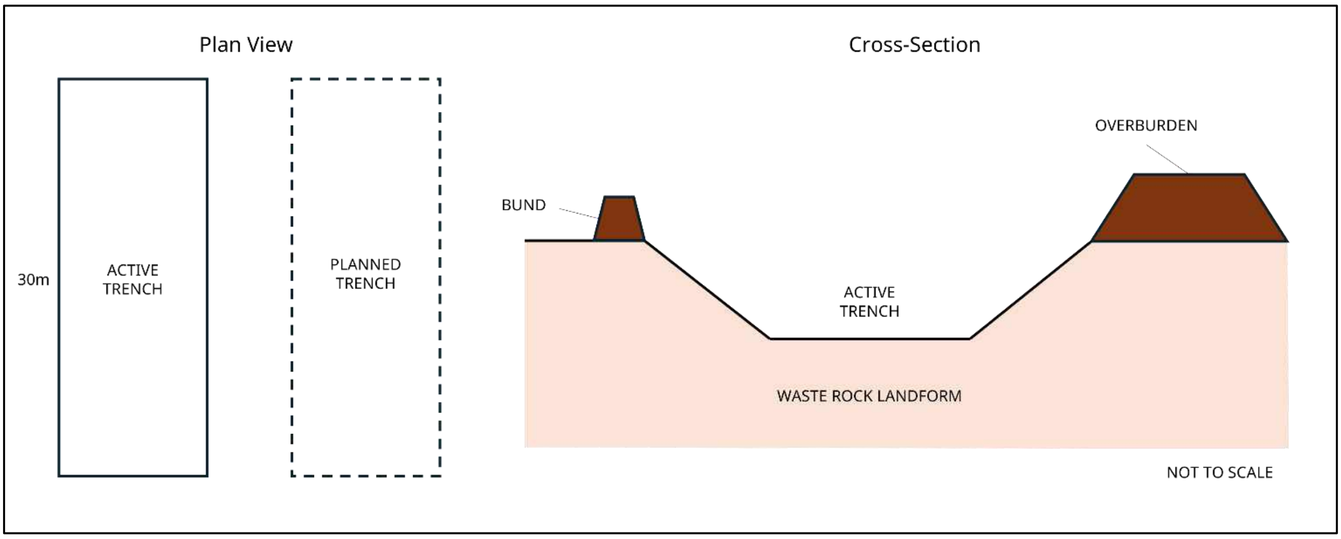

Figure 8: Landfill schematic

5.2.2 Environmental Commissioning Activities

No environmental commissioning activities are proposed.

5.2.3 Time Limited Operations

To streamline the approvals process and enable proposed activities to commence following construction and commissioning, Genesis requests that the activities are authorised as Time Limited Operations (TLO). The TLO period is requested to be set at 350 calendar days. Acknowledging that this is longer than standard, it will allow sufficient time for preparation and assessment of a Licence Application and reduce administrative burden for all parties.

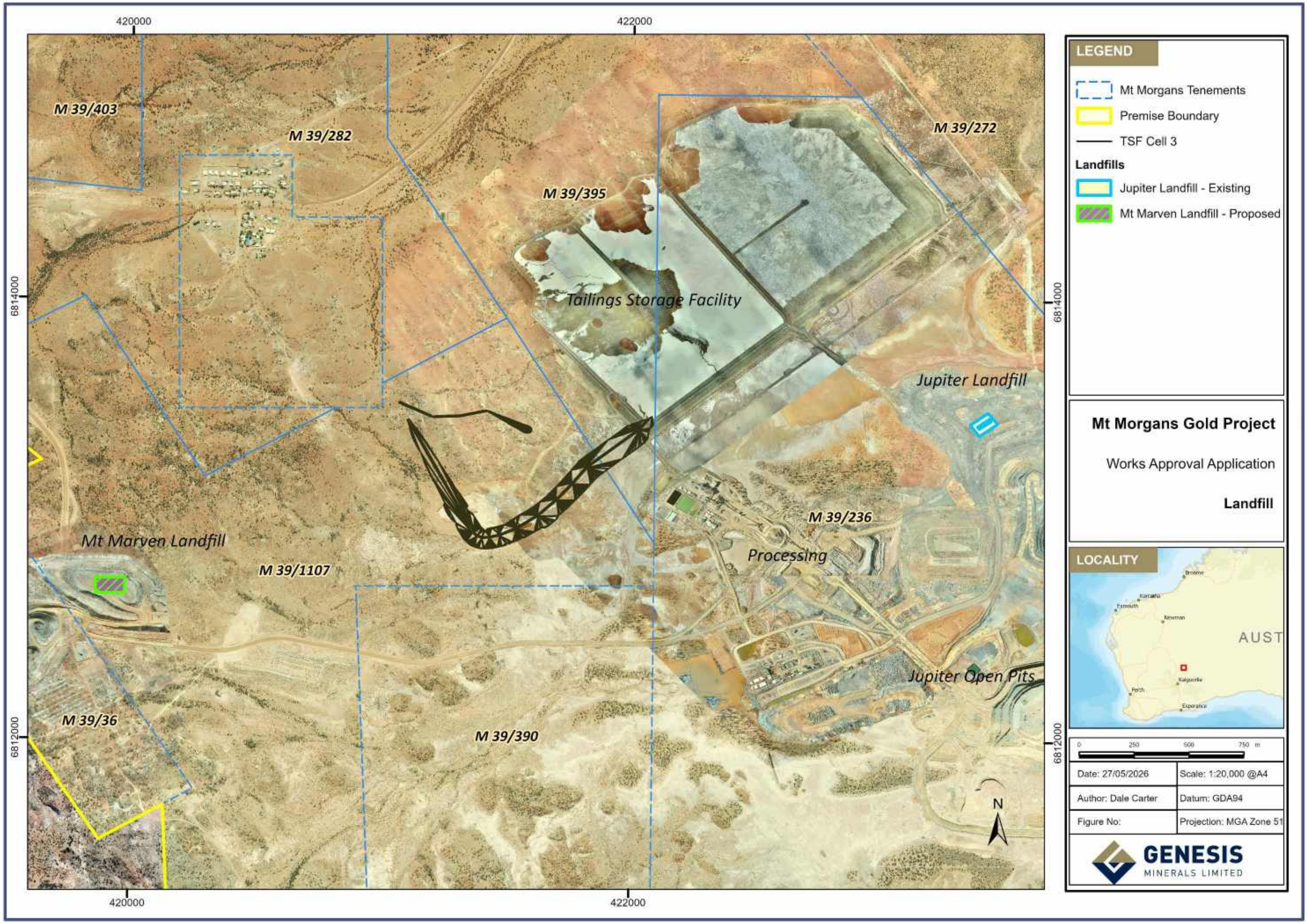

Figure 9: Category 64 Landfill Locations

6 Attachment 3C: Map of Area Proposed to be Cleared

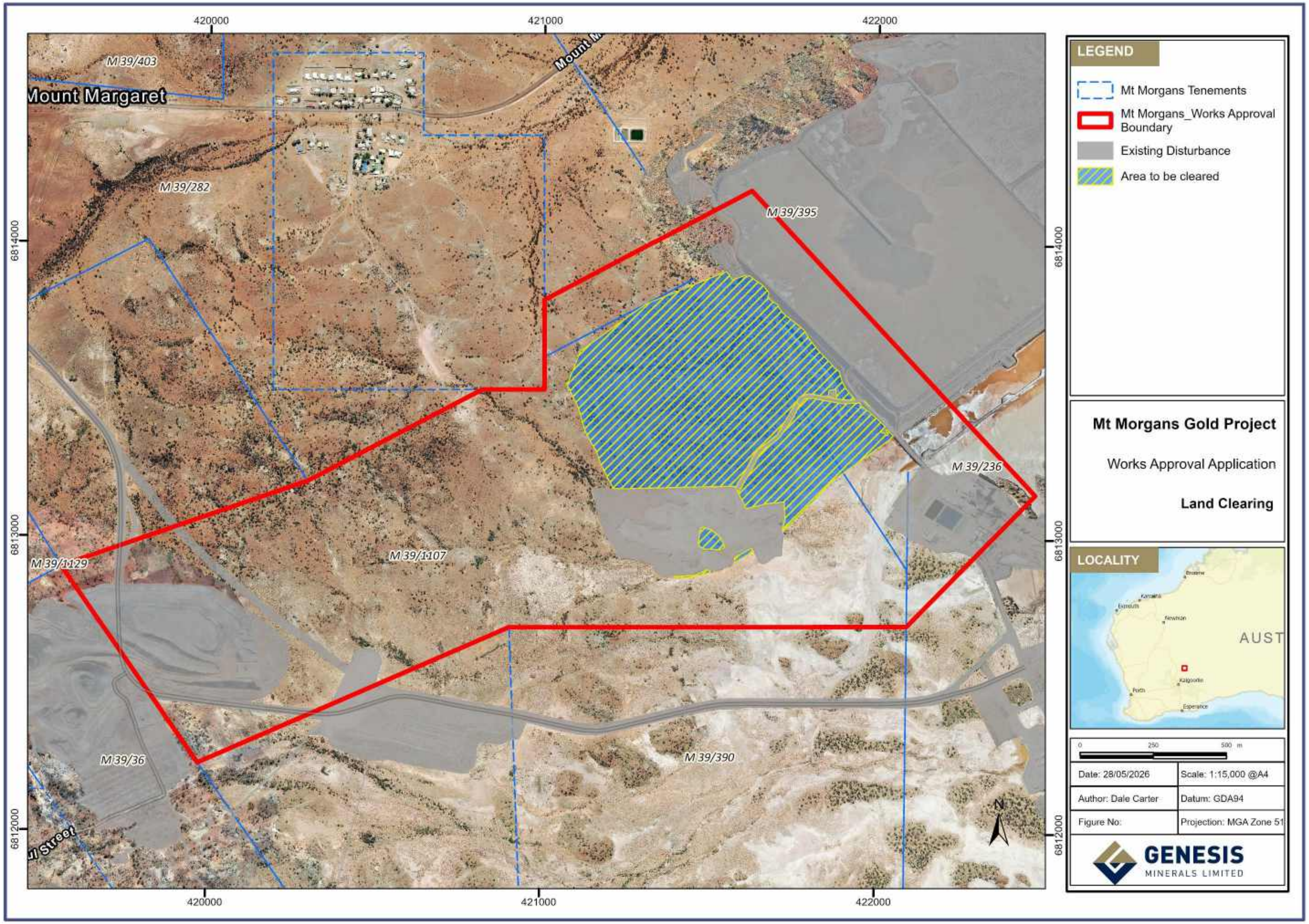

Figure 10: Clearing Area

7 Attachment 5: Other Approvals and Consultation

7.1 Environmental Protection Act 1986 – Part IV

The project is not considered to have a significant environmental impact and therefore has not been referred to the Environmental Protection Authority (EPA) for assessment under Part IV of the *Environmental Protection Act 1986*.

7.2 Environmental Protection Act 1986 – Part V

Clearing will be undertaken in accordance with the *Environmental Protection Regulations (Clearing of Native Vegetation) 1994*, via a Purpose Permit or relevant exemption.

Native Vegetation Clearing Permits CPS 7408/5 and CPS 7428/2 exist for the project, authorising combined clearing up to 828 ha. A new application or amendment will be sought as these permit's do not cover the entirety of proposed TSF Cell 3.

7.3 Mining Act 1978

A Mining Development and Closure Proposal NDCO-206730 is currently under assessment, covering works included in this application.

7.4 Rights in Water and Irrigation Act 1914

Groundwater Licences GWL 169901 and GWL 183915 exists for the project, authorising a combined annual water entitlement of up to 4,900,000 kL.

7.5 Consultation

Genesis is committed to ongoing stakeholder consultation in line with their stakeholder engagement strategy. Key stakeholders include the Mount Margaret Aboriginal Community, Shire of Laverton and Nyalpa Pirniku Native Title Group Traditional Owners.

Wangkatja Tjungula Aboriginal Corporation (WTAC) is the body corporate that represents Nyalpa Pirniku Native Title Group. In December 2025, Genesis signed a Native Title Mining and Heritage agreement with (WTAC), on behalf of the Nyalpa Pirniku native title holders. The agreement covers the MMGO operations. A key element of the agreement is the establishment of structured processes to support meaningful Nyalpa Pirniku involvement. This includes the delivery of Cultural Awareness Training by WTAC to Genesis employees, contractors, and personnel working on Nyalpa Pirniku country, the establishment of an Employment and Procurement Committee to guide employment, procurement, and contracting participation strategies for Nyalpa Pirniku Common Law Holders, as well as the creation of a Nyalpa Pirniku mentor role, filled by Common Law Holders, to support recruitment, retention, relationships, and career development of Nyalpa Pirniku people. A meeting was also held 19 of January 2026 between Genesis and WATC representatives, including the chairperson, to present on planned future works at MMGO. In principle, WTAC are supportive of Genesis operations within the Native Title area. Meetings with the Mt Margaret Community are held on a quarterly basis.

Genesis recently presented to the Shire of Laverton on 6 February 2026 an update on current and future operations, along with potential synergies. To date, the Shire has been supportive of MMGO.

8 Attachment 6A: Emissions and Discharges

A summary of emissions, discharges and pollution control equipment is provided in **Table 6.** This is discussed in detail in the following sections.

Table 6: Emissions and discharges

#	Source	Emission or discharge type	Volume and frequency	Proposed controls	Location
1.	TSF overtopping	Contaminated stormwater	Nil	Maintain minimum freeboard 0.7 m from embankment crest. See **Section 8.1.1.**	TSF Cell 3
2.	TSF construction and deposition	Dust	Low	Dust suppression via water cart. Dust monitor at Mount Margaret Community with set compliance limit. See **Section 8.1.2.**	TSF Cell 3
3.	TSF construction	Noise	Low to moderate	Noise during TSF construction will be managed by restricting construction to daylight hours only, regular vehicle and plant maintenance and muffler/noise attenuating equipment installed where practicable. See **Section 8.1.3.**	TSF Cell 3
4.	Pipelines carrying tailings	Leaks and spills	Nil	All pipelines containing tailings and return water will be either equipped with telemetry, automatic cut-outs, or provided with secondary containment. Pipelines visually inspected for integrity. See **Section 8.1.4.**	TSF Cell 3
5.	Landfill	Odour	Low	Putrescible wastes will be compacted and covered by the end of the month in which waste was deposited. See **Section 9.2.1**	Landfill

8.1 Category 5: Ore Processing

8.1.1 Contaminated Stormwater

This area experiences a semi-arid climate, with low and highly variable annual rainfall, typically around 250–300 mm per year, falling mostly during irregular storm events. Surface water flows are therefore ephemeral, occurring only after significant rainfall.

The proposed TSF Cell 3 has been designed such that a 1:100 years AEP, 72-hour duration storm event can be temporarily stored on top of the facility, assuming that all operational controls are adhered to, and water is continually removed from the facility, such that minimum freeboard allowances are maintained.

Provision of a minimum of 0.7 m freeboard comprising a minimum operational freeboard (vertical height between the tailings beach and embankment crest) of 0.3 m plus a minimum beach freeboard of 0.2 m and the allowance for the 1:100 years AEP, 72-hour event of approximately 0.2 m (**Figure 11**).

ANCOLD guidelines (2019) also recommend an allowance for wave run-up for 1:10 AEP wind for a 'High C' consequence category TSF. However, it is expected that with perimeter tailings deposition and an expected beach slope of 1%, the separation distance between the perimeter embankments and design storm pond will be adequate to prevent wave action reaching the embankments.

Based on intensity-frequency-duration (IFD) data pertaining to the site a 1:100 years AEP, 72-hour duration rainfall depth of 195 mm was adopted for the design. Temporary storage of stormwater volume should cater for approximately 97,500 m^3 (i.e. approx. 50 ha x 195 mm) on top of TSF Cell 3. Upstream catchment area has been assumed as 10 ha in the estimate. This stormwater volume will occupy approximately 2% to 5% of the TSF basin. TSF Cell 3 also has sufficient capacity to contain the probable maximum flood (PMF) of approximately 275,000 m^3 (i.e. 50 ha x 550 mm).

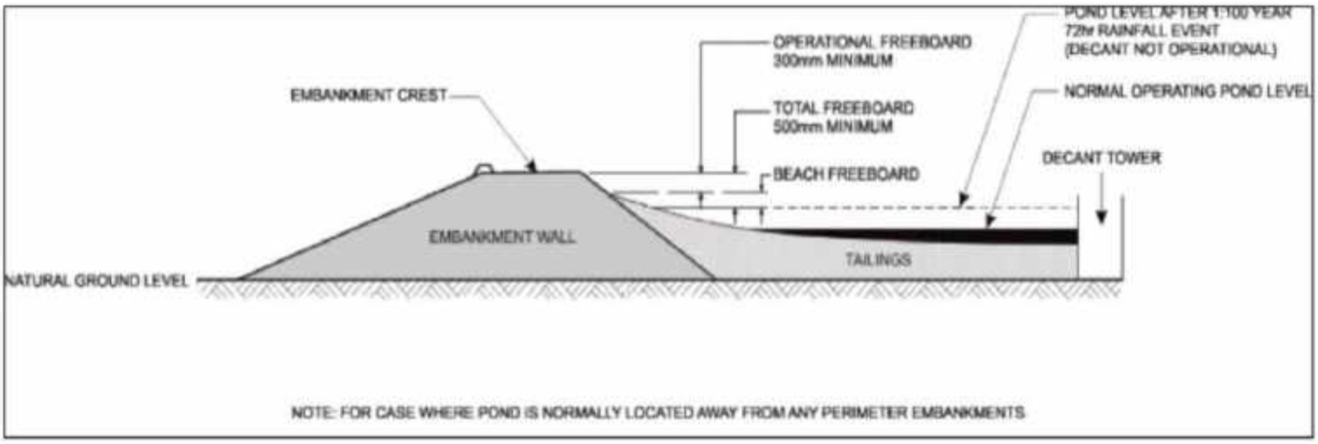

Figure 11: TSF freeboard

8.1.2 Dust

Dust will be generated from clearing, topsoil harvest and earthworks for TSF Cell 3. To manage dust, water carts will be used for suppression by regularly spraying water over exposed areas (such as stockpiles, embankments, and access roads). Clearing will be undertaken in low wind conditions to reduce excessive airborne dust. Dust emissions are expected to peak during clearing, topsoil harvest and earthworks and should reduce once base compaction and embankment construction is underway. Once operating, minimal dust is expected to be generated from Cell 3 and any incidental dust generated will be easily managed using the water cart.

During operations the process water used in the plant is saline, which would result in a crust of salt and gypsum forming on the surface in any scenario in which the TSF did become dry. This would inhibit saltating particles during high wind speed events which would ensure that the TSF is not a source of dust.

Mount Margaret Aboriginal Community is located 800 m north-west of proposed TSF Cell 3. An existing dust monitor is situated at the community (**Figure 12**). Ambient PM_{10} is monitored with a 24-hour averaging period, with a compliance limit of 50 µg/m³ (**Table 7**).

If a 24-hour sample exceeds the 50 µg/m³ limit, an investigation must be undertaken to determine the root cause(s) of the exceedance and identify any common or contributory factors (e.g. operational changes, weather conditions, maintenance issues), so that corrective management actions can be developed.

Sampling is done in accordance with recognised Australian standards: AS 3580.9.8 (continuous direct-mass TEOM method), AS 3580.9.11 (beta-attenuation monitors), or AS 3580.9 (general PM_{10} sampling). The monitoring station is sited according to AS 3580.1.1, which provides guidance on proper location to ensure valid, representative measurements.

Table 7: Monitoring of airborne dust

Monitoring point	Parameter	Limit	Units	Sampling duration	Applicable standards
Located to measure airborne dust exposure levels at the Mt Margaret Community	PM_{10}	50	µg/m³	24 hours	Monitoring methods: AS 3580.9.8, AS 3580.9.11, or AS 3580.9.6 Siting: AS 3580.1.1

Results from dust monitoring during construction of TSF Cells 1 and 2 (2017-18) showed a daily average ambient PM_{10} of 8 µg/m³, with just one recorded exceedance to the 50 µg/m³ compliance limit (52 µg/m³, 16/12/2017) (**Figure 13**), due to the wide north-westerly wind direction at the time, it is unlikely the exceedance was due to TSF construction activities. Since commencement of operations in October 2024, there have been no exceedances due to mining activity. This information suggests that current controls are adequate for construction and operation of TSF Cell 3.

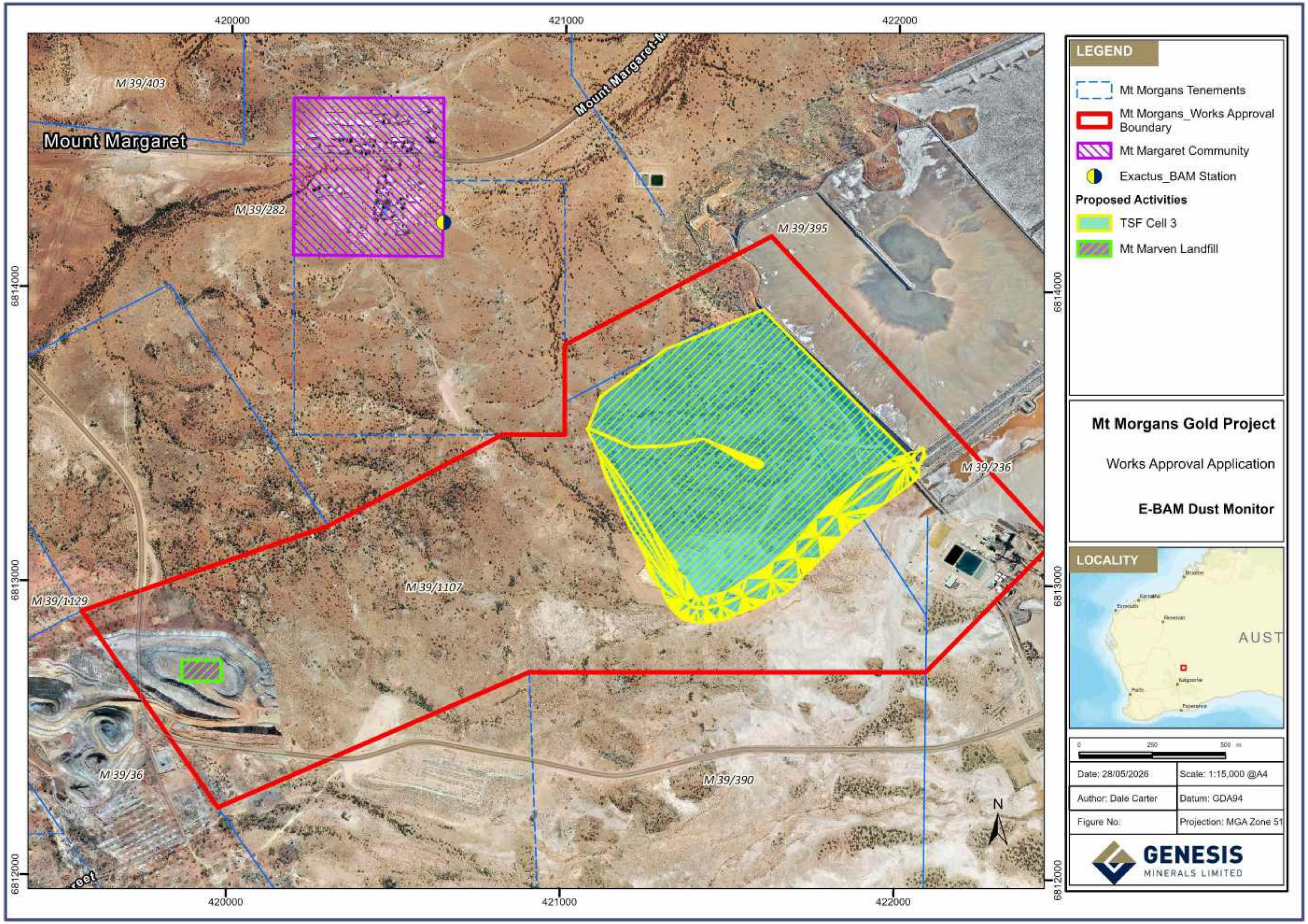

Figure 12: E-BAM Dust Monitor Location

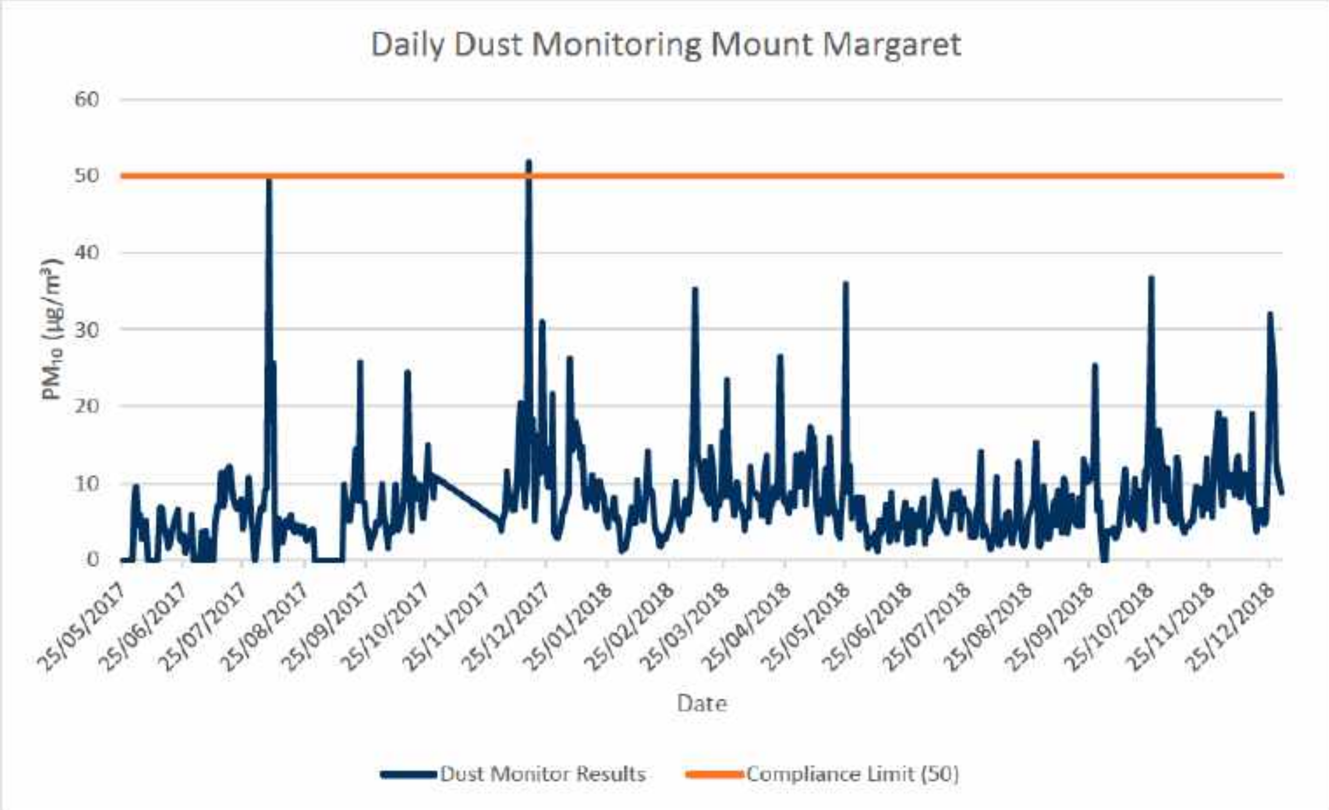

Figure 13: Dust monitoring during construction of TSF Cells 1 & 2 (2017-18)

Vegetation within the zone of influence of the TSF will continue to be monitored on an annual basis in accordance with current licence conditions. The assessment shall:

(a) photograph and record the presence and condition of key vegetation features within the zone of influence;
(b) compare the results of the assessment against previous years assessments and identify whether any deterioration in the presence and/or quality of vegetation has taken place; and
(c) be undertaken by a person qualified in vegetation identification and sampling.

Recent vegetation monitoring determined that all assessment areas showed low vegetation densities that are comparable to the undisturbed vegetation (baseline) as well as the broader region. These low vegetation densities are typical of the arid northern Goldfields region of Western Australia. The long-term normalised difference vegetation index (NDVI) time series shows significant variation in vegetation density over time and by season. All assessment areas, including baseline areas, reported a negative trend in vegetation density until January 2024. Elevated rainfall was recorded between January and July 2024 which has been attributed to an overall increase in vegetation across the region, including vegetation within the TSF zone of influence (RPM, 2025).

The existing vegetation monitoring area located along the south-western wall of TSF Cell will be relocated as a result of new TSF Cell 3. New vegetation monitoring sites will be established around the perimeter of TSF Cell 3. Results of vegetation monitoring will continue to be reported in the Annual Environmental Report.

8.1.3 Noise

Mount Margaret Aboriginal Community is located 800 m north-west of proposed TSF Cell 3. Noise will be generated predominantly from clearing and earthworks during construction of Cell 3. This will be managed though:

- Construction activities associated with the TSF will only occur during daytime hours.
- All vehicles and plant equipment will be regularly maintained to ensure they are operating efficiently and are not unduly noisy.
- Where possible, mufflers and other noise attenuating equipment will be installed and maintained on plant, vehicles and equipment to reduce exposure to occupation noise.
- Standard Personal Protection equipment will used by employees where required to reduce exposure to occupational noise.

8.1.4 Leaks and Spills

All pipelines containing tailings or process liquors will be either equipped with telemetry systems and pressure sensors along pipelines to allow the detection of leaks and failures; equipped with automatic cut-outs in the event of a pipe failure; or provided with secondary containment sufficient to contain any spill for a period equal to the time between inspections. Tailings and return water/decant pipelines will be visually inspected for pipeline integrity every 12 hours when operating and weekly during non-operation.

9 Attachment 6B: Waste Acceptance

A summary of waste types and details is provided in **Table 8.** This is discussed in detail in the following sections.

Table 8: Waste types and details

#	Waste type	Quantity	Waste activity infrastructure	Monitoring	Location
1.	Tailings	6.2 Mt (4.4 Mm^3) total storage capacity	TSF Cell 3. See **Section 5.1.1** and **Attachment 8A.**	Monitoring will include: • Volumes of tailings deposited, water recovered and seepage recovered/reused. • Decant pond WAD cyanide and TDS. • Monitoring bores SWL, pH, TDS, WAD cyanise and metals. See **Table 9.**	TSF Cell 3
2.	Inert waste type 1, putrescible waste, clean fill, and inert waste type 2 (tyres)	Up to 4,500 tpa	Landfill trenches. See **Section 5.2.**	Putrescible wastes compacted and covered by the end of the month in which waste was deposited.	Landfill

9.1 Category 5: Tailings

9.1.1 Deposition

TSF Cell 3 will have a maximum storage capacity of approximately 6.2 Mt. This corresponds to an additional storage life of approximately 2 years and 1 month assuming a production rate of 3 Mtpa and tailings in-situ density of 1.4 t/m^3 (dry). However, in practice deposition will cycle between TSF Cell 1, 2 and 3 which will ensure that slurry material has time to consolidate and efficiently reach the designed in-situ density.

Tailings in the form of slurry will be discharged sub-aerially and cyclically into TSF Cell 3 in thin discrete layers, not exceeding 0.3 m thickness, from multipoint spigots in order to allow optimum density and strength gain by subjecting each layer to a drying cycle. Deposition will take place via multiple spigots located on the upstream perimeter embankment crest.

Tailings deposition will be carried out such that the supernatant pond is maintained within and around the decant. The pond is to be maintained away from the perimeter embankments at all times. Water will be removed from the facility and pumped back to the mill via a decant pump located within the slotted concrete pipes in the decant and underdrainage tower.

MBS Environmental (2016) conducted geochemical characterisation testing in 2016 on tailings generated from samples of ore from various pits at Mt Morgans (**Attachment 8E**). Results indicated that tailings samples were non-acid forming (NAF), however, samples from Allanson underground were PAF. It was noted that Allanson is not in the 5 year mine plan to be mined. Overall, the tailings were expected to be NAF. Testing indicated that decant water would be alkaline and saline, but with a salinity less than that of groundwater at the TSF site (MBS, 2016).

Tailings deposition and decant will be monitored in accordance with **Table 9**.

Table 9: Process monitoring

Process description	Parameter	Units	Frequency (during operation)	Frequency (during non-operation)	Method
Tailings deposition	Volumes of tailings deposited into the TSF	Tonnes	Continuous	Continuous	None specified
	Volumes of water recovered from the TSF	kL			
	Volumes of seepage recovered and reused on the process plant	kL			
TSF Cell 3 decant pond	Total dissolved solids	mg/L	Quarterly	Quarterly	AS/NZS 5667.1
	WAD cyanide				

9.2 Category 64: Landfill

9.2.1 Waste

The Mt Marven landfill, situated within the Mt Marven waste rock landform, will accept site generated inert waste type 1, putrescible waste, clean fill, and inert waste type 2 (tyres). All other waste types will be removed from site for disposal at licensed facilities, ensuring the landfill only receives appropriate materials. Standard management practices, including compaction and segregation, will minimise environmental risks, and a minimum 2 m separation between the landfill base and the highest groundwater level will protect local groundwater resources. Putrescible wastes will be compacted and covered by the end of the month in which the waste was deposited with sufficient quantities of Type 1 inert waste, clean fill or other appropriate cover material.

10 Attachment 7: Siting and Location

10.1 Location

The Mount Morgans Gold Project is located approximately 30 km south-west of Laverton, in the north-eastern Goldfields. The site has historically been operated since the 1980s by several companies prior to Genesis acquiring it in 2023. The site was in care and maintenance for a period between 2023 to 2024 before production recommenced in October 2024.

The project is situated in the Shire of Laverton and comprises two key mine areas: Westralia and Jupiter. Mount Margaret Aboriginal Community is situated approximately 600 m from the premises boundary. A ridgeline approximately 30 m high separates the community from the Jupiter area. The site of the Community was founded as a mission by the United Aborigines Mission in 1921 and drew in Aboriginal people from surrounding areas. Approximately 85 to 100 people currently reside in the settlement (Department of Planning, 2012). The Community is managed through its incorporated body, the Aboriginal Movement for Outback Survival (AMOS). AMOS was incorporated under the Aboriginal Councils and Associations Act 1976 on 12 September 1997.

The project is partially situated on Glenorn Pastoral Lease, held by Minara Resources. Glenorn is an active pastoral station running sheep and beef cattle. The Glenorn homestead is located approximately 50 km south-west from the project.

Mining has been a land use in the area since the 1890s and remains an active industry with several operational mines occurring within a 100 km radius, including Granny Smith, Sunrise Dam, Murrin Murrin and Lancefield.

10.2 Climate

The area experiences mean maximum temperature of 35.8˚C and mean minimum temperature of 5.9˚C. Annual average rainfall is 274.0 mm, with a mean of 34.4 days of rain per year (≥1 mm). Rainfall is highest in February at 49.1 mm and lowest in September at 7.5 mm. Mean annual evaporation is approximately 2,800 mm. Mean monthly rainfall and maximum temperatures are provided in **Figure 14** (BOM, 2025).

The rainfall intensity-frequency-duration (IFD) chart pertaining to Mt Morgans is presented in **Figure 15.** Based on the IFD chart, a 1:100-year annual exceedance probability, 72-hour storm event can be expected to generate approximately 195 mm of rainfall (GRM, 2020).

Across the average year, humidity levels are highest in June (am 65%, pm 47%) and lowest in December (am 33%, pm 20%). Morning wind conditions measured are predominantly easterly, between 9 and 13 km per hour (km/hr). Average afternoon wind direction is more variable, with easterlies slightly prevailing, between 8 and 12 km/hr (BOM, 2025).

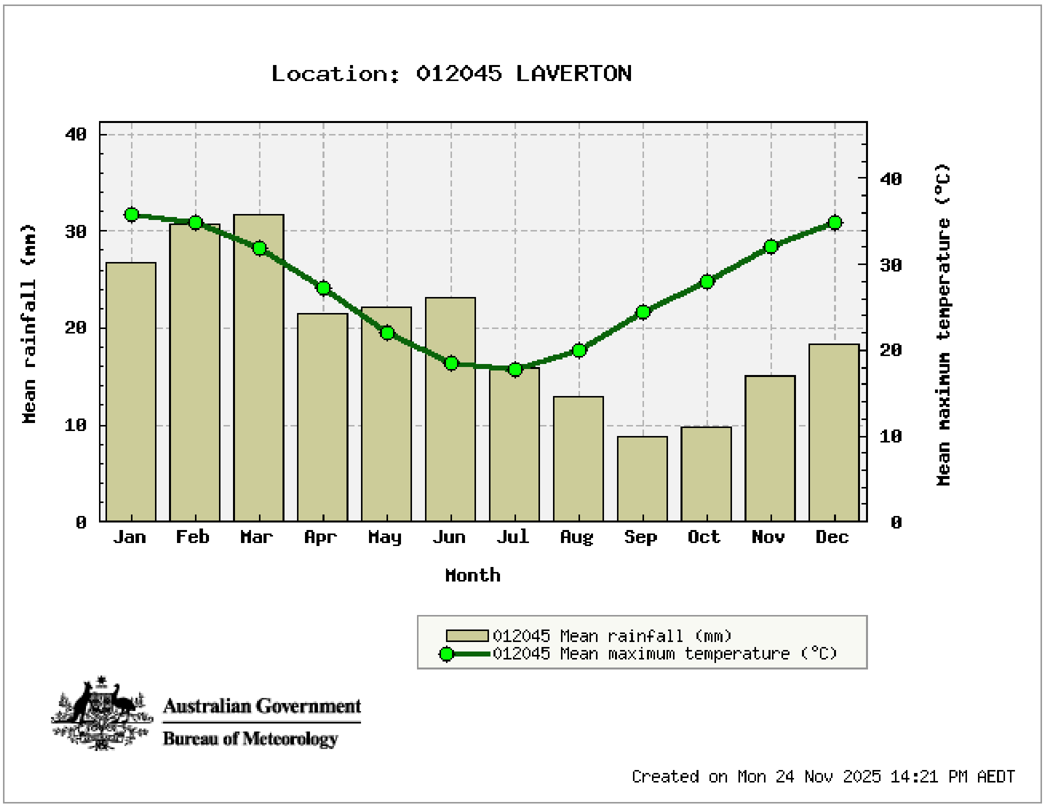

Figure 14: Mean monthly rainfall and maximum temperature

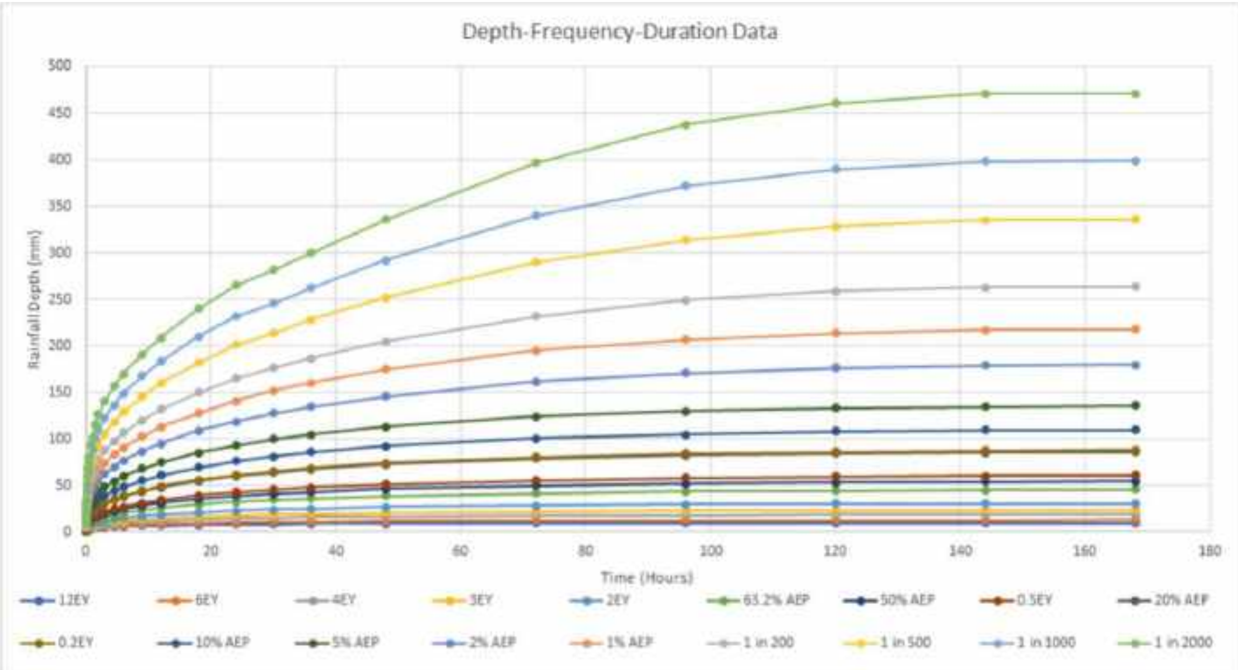

Figure 15: IFD data (GRM 2020)

10.3 Biodiversity

Several flora and fauna surveys have been undertaken over the last ten years (Native Vegetation Solutions (NVS), 2016; 2019; Western Wildlife 2016; 2020). Vegetation in the area is characterised by a mix of Mulga woodlands, hummock grasslands, saltbush, *Tecticornia* shrublands and bare salt lakes. These plant communities include a variety of *Acacia* species, such as *Acacia aneura* and *Acacia mulganeura*, often with *Eremophila* and *Triodia* grasses in the understory, as well as *Tecticornia* and other salt-tolerant species in lower-lying areas. All vegetation types defined are well represented throughout the broader project area and region. Vegetation condition ranged from Very Good to Degraded (Keighery, 1994).

The majority of fauna habitats found within the area are relatively common and widespread in the region. Western Wildlife concluded that potential impacts to fauna are generally likely to be minor and it is unlikely that vegetation represents significant habitat for fauna in a regional context (Western Wildlife, 2016).

There are no Conservation Reserves or Environmentally Sensitive Areas within 50 km of the project. No Threatened Ecological Communities (TEC) have been identified within the project area however the survey area falls within the buffer region of the Priority 1 Priority Ecological Community (PEC) named "Mount Morgan calcrete groundwater assemblage type on Carey palaeodrainage on Mt Weld Station". This PEC is designated to protect the subterranean fauna community identified within the buffer zone. Given this, proposed clearing is not likely to impact this community (NVS, 2016).

Bennelongia (2017) conducted a desktop subterranean fauna assessment of the project to determine if field assessment was required to understand community assemblages. The likelihood of supporting stygofauna and troglofauna was considered low at Jupiter due to the hypersaline nature of groundwater, low permeability geology and limited suitable habitat.

10.4 Surface Water

The project is located within the internally draining Lake Carey catchment which has a catchment area of approximately 113,900 km^2 and forms part of the DWER Salt Lake Basin (total catchment area 441,000 km^2). This basin comprises several large and broad, sub-parallel, southeast trending salt-lake drainage systems. These drainages have low gradients and at intervals contain small to large playa lakes such as Lake Carey (with an area of approximately 1,000 km^2). During occasional intense rainfall events the lakes may fill, and in rare events some may overflow, link-up, and discharge on to the Nullarbor Plain through Ponton Creek. Water ponding in the playa lakes is hypersaline, particularly when levels are low. Salinity decreases following significant rainfall events, but the salt levels rapidly increase as water levels drop from evaporation.

The ground elevation slopes gently from approximately 490 mAHD at Mount Phoenix adjacent to Westralia down to lakeshore elevations of 399 mAHD some 12 km to the southeast i.e. an average ground slope of less than 0.8% (GRM, 2016).

There are no major river systems in the vicinity of the project although there are several ephemeral creeks which generally drain in a north to south-east direction towards Lake Carey, which is the most dominant local hydrological feature. The bulk of run-off from around the Mt Morgans reports to Lake Carey via a combination of surficial and shallow baseflow along the salt-lake drainage system.

Jupiter is situated on an upstream tributary to Lake Carey. A BIF ridge extending up to about 80 m above the lakebed lies between the Jupiter deposits and Lake Carey. Anecdotally water depths of up to about 600 mm have been witnessed on Lake Carey and other nearby playa lakes. Runoff entering the lake at this northern end continues down gradient towards the south, albeit at a slow rate due to the low gradient. To the west of Jupiter, undulating hills run in a northeast – southwest direction range in height between 425 mAHD – 444 mAHD (i.e. about 20 – 45 m above the height of the player lake surface). The TSF is situated against these hills, using the natural topography for containment (**Figure 16**).

A flood risk assessment was completed by GRM in 2016 (GRM, 2016). The assessment included design of surface water management infrastructure for the project by a civil engineer. Within the Jupiter area, intersection of ephemeral lake tributaries poses the greatest flood risk to the proposed open pits and infrastructure. Flows will occur periodically, particularly during the summer months from January to March, when the potential exposure to high intensity rainfall is greatest. Consequently, runoff will report to Lake Carey and the watercourses in the vicinity of the project, and, on occasion, flows may be high and may cause flooding if appropriate measures are not in place (GRM, 2016).

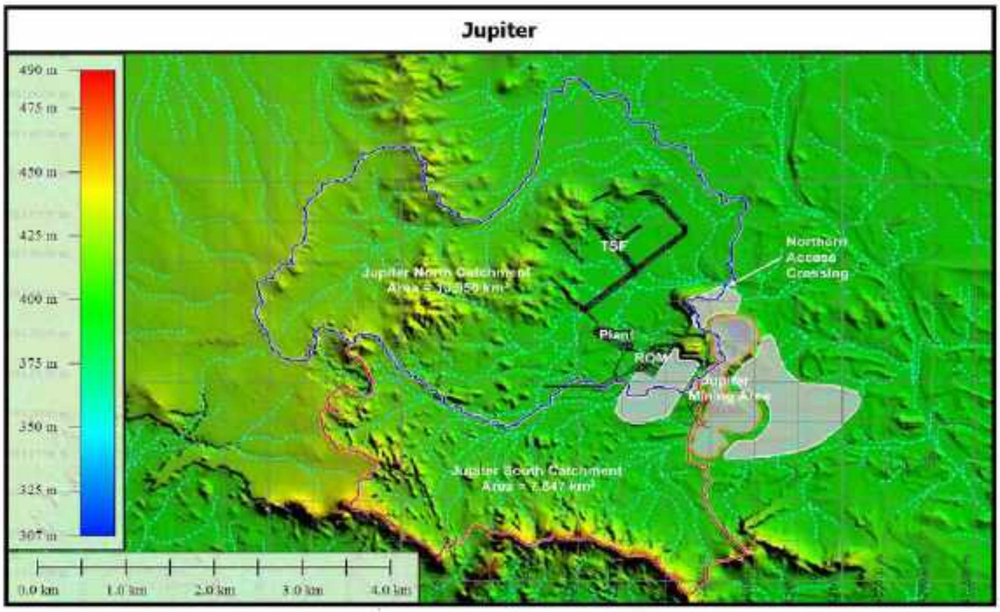

Figure 16: Jupiter surface water drainage and topography

10.5 Groundwater

10.5.1 Mt Morgans TSF

Due to the remote nature of the project, there are limited uses of groundwater in the region outside mining and pastoral purposes. The nearest public drinking water sources are located 25 km northeast (Laverton Water Reserve and Catchment Area) and 60 km west (Leonora Water Reserve).

Mount Margaret Aboriginal Community source their drinking water from groundwater bores situated approximately 4 km north of the Jupiter complex, upstream from the mining operations.

Genesis also operate a borefield in this vicinity to source water for processing operations which includes monthly monitoring of production bores as well as several monitoring bores in the area. It is in Genesis best interest to maintain groundwater quality to ensure effective processing operations.

According to the Livestock Drinking Water Guidelines (Draft) (ANZG, 2023), ideal drinking water is up to 4,000 mg/L TDS, however livestock may tolerate up to 10,000 mg/L.

Vegetation in the region is considered unlikely to be groundwater dependent and instead adapted to arid soils and draw on non-saturated soil moisture (i.e. rainfall, shallow soil water).

A TSF Groundwater Management Plan exists for the Mt Morgans TSF and has been provided as **Attachment 8B.** This plan provides a detailed overview of groundwater processes, impact assessment and management at Mt Morgans and will be updated to include TSF Cell 3.

10.5.2 Jupiter and Westralia

Information regarding groundwater at Jupiter and Westralia are provided in **Section 5.1.1.1.9** and the 2022-25 Triennial Aquifer Review in **Attachment 8C.**

10.6 Aboriginal Heritage

A search of the Department of Planning, Lands and Heritage (DPLH) Aboriginal Cultural Heritage Inquiry System (ACHIS) identified several registered and lodged sites within the premises boundary. It's important to note that the site boundaries shown on ACHIS are considered buffers to maintain site sensitivity and are not considered actual site boundaries. A number of Aboriginal heritage surveys have been completed within the project area, dating back to the 1980s, culminating in the identification of several heritage sites in the vicinity of the project.

Genesis understands the cultural and historical importance of preserving Aboriginal heritage and is committed to is committed to ensuring the site operates with consideration to heritage values. Heritage sites have been considered when designing mine activities to ensure impacts are minimised as far as practical and consultation with Traditional Owners is maintained. There are no Aboriginal Heritage sites located within the footprint of proposed TSF Cell 3 and it is not expected that the construction and operation of this facility will have an impact on surrounding heritage sites (groundwater, surface water, topography etc.).

Concerns have been raised around impacts to the Healing Pool, a culturally significant site 1.6 km north east of the TSF. This includes an appeal against a previous Works Approval to construct a TSF lift, on the basis that seepage may impact the Healing Pool. Partible Tracing was modelling (Section 5.1.1.1.9), demonstrating the negligible risk to the Healing Pool from tailings deposition.

11 Attachment 8: Additional Information

11.1 Attachment 8A: TSF Cell 3 Design Report

11.2 Attachment 8B: TSF Groundwater Management Plan

11.3 Attachment 8C: Triennial Aquifer Review (2022-25)

[INSERT PAGES]

11.4 Attachment 8D: TSF Solute Transport Model

11.5 Attachment 8E: Tailings Geochemical Characterisation (MBS Environmental, 2016)

12 Attachment 9: Category Checklists

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