



TSF4 Cell 1 Design to RL 1277.5m

**Addendum to RL 1275m Raise Detailed
Design Report**

Talison Lithium Pty Ltd

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→ The Power of Commitment



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Executive summary

This report documents the redesign of the Cell 1 raise of the Tailings Storage Facility 4 (TSF4) at the Talison Lithium Pty Ltd (Talison) mine site in Greenbushes, Western Australia.

GHD originally design the raise of both Cell 1 and Cell 2 to the crest RL 1275 m (GHD, 2025a), Cell 2 is currently under construction. Talison requested update of the Cell 1 design, to achieve the crest elevation of RL 1277.5 m to allow deposition in each cell for one year, starting and ending during the wet season, uninterrupted construction during the dry season and sufficient time for approval application (by others).

This report is an Addendum to the original design report (GHD, 2025a), summarising changes related to the crest elevation update. Items unaffected or unchanged are not include in the addendum, therefore both documents shall be read together.

The key items affected and summarised in the addendum are:

- Storage capacity review, with redesigned raise allowing more than 1 year of capacity in Cell 1 (total pf 14-15 months of capacity expected when comparing to January 2026 beach survey)
- Review of the 2025 groundwater monitoring data provided by Talison – no continuous or progressive increases in metal concentrations were observed over the monitoring period, suggesting that the existing barrier is functioning as intended and maintaining consistent containment performance. There is no impact on the redesign
- Raise of the northern embankment of Cell 1 to the RL 1277.5 m exceeds the adjacent Tailings Storage Facility 1 (TSF1) crest elevation. The raise geometry of the northern Cell 1 embankment has been updated, to avoid the TSF4 crest encroaching on the TSF1 upstream tailings.
- Adjacent TSF1 is currently being excavated (remining operations) with tailings elevations lower than assumed in the original design, and to be excavated further. GHD assumed that at the time of the TSF4 Cell 1 raise, TSF1 tailings had been excavated to the RL1264 m. TSF1 will require infilling at the interface with TSF4 to support stability as shown in Drawings (Appendix A). It shall be noted that rate of the loading with Infill is to be determined and in not in scope of this addendum. Talison is planning a field trial to support the future assessment of the loading rate. Infilling shall not be placed without the Engineer of Record (EoR) approval.
- Stability assessment update (required Factors of safety are met).
- Embankment geometry updates as shown in Drawings (Appendix A)

This addendum is subject to and must be read in conjunction with the original design (GHD, 2025a), limitations set out in Section 1.6, Section 2 and throughout the report.

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1. Introduction

1.1 Project background

To meet ongoing operational requirements Talison Lithium Limited (Talison) is proposing to raise the existing Tailing Storage Facility 4 (TSF4) at the Greenbushes Lithium Mine.

TSF4 comprises two (2) cells that are raised alternately. The Cell 1 and Cell 2 raises to RL 1270 m were completed in Quarter 1 and Quarter 2 of 2025. GHD issued design for the RL 1275 m raise design in May 2025 (GHD, 2025a). Cell 2 and the divider embankment between the two cells are currently being raised to RL 1275 m in accordance with this design.

To increase capacity and support the subsequent approvals, deposition and construction schedules Talison requested that the crest elevation of Cell 1 be increased to provide a minimum of 12 months storage capacity.

The increased crest height will

- Increase storage capacity to at least 12 months in one cell and will consist of increasing the Cell 1 crest from RL 1275 m to RL 1277.5 m)
- Enable construction during the dry season
- Allow beach to develop to the full design height and allow sufficient time for TLO approval
- Minimise the required harvesting of the coarse tailings for sand platforms for future raises
- Minimise cost of construction and impacts on operating cells
- Enable construction during the dry season

This report documents a redesign for Cell 1 (Cell 1 only) to minimise the new crest of RL 1277.5 m, assuming Cell 2 is currently being constructed to RL 1275 m as per the original design (GHD, 2025a).

1.2 Redesign objective

The objective of the TSF4 Cell 1 redesign is to:

- Increase the storage capacity of Cell 1 to at least 1 year from levels in the most recent survey. Required volume is noted in Table 2.1
- Retain the raise of the TSF4 Cell 1 northern embankment of the RL1277.5 m raise on the existing TSF1 southern embankment crest to minimise encroachment into TSF1

1.3 Purpose of this report addendum

Purpose of this Addendum Report is to document the changes to the RL 1275 m design associated with the increase in the Cell 1 crest elevation change from RL 1275 m to RL 1277.5 m.

1.4 Report structure

This report is an addendum to the TSF4 RL 1275 m Raise Detailed Engineering Design Report (GHD, 2025a) and should be read in conjunction with the RL 1275 m raise design report. The RL 1277.5 m raise design report documents all changes to the previous Cell 1 RL 1275 m raise design report. Where items in the RL 1275 m raise design report has remained unaffected by the increase in the crest height the reader is referred to the TSF4 RL 1275 m Raise Detailed Engineering Design Report.

1.5 Scope

The following key tasks were undertaken as a part of the Cell 1 redesign:

- Review of beach slopes and storage capacity check for the updated crest elevation
- Slope stability review (1277.5 and final height)

- Cell 1 embankment re-design (perimeter, divider, TSF1 interface, decant access causeway)
- Update the Cell 1 RL 1275 m Drawings. Drawings shown in Appendix A are included as a standalone new set for Cell 1. Cell 2 Drawings are not included
- 3D Model update
- Addendum report covering aspects affected by the redesign

Following is excluded from the scope:

- Operations, Monitoring and Surveillance (OMS) manual. This will be undertaken as part of the commissioning stage and will be subject to a separate proposal
- Cost estimates
- All mechanical, electrical and instrumentation design engineering works for the project
- Detailed closure plan
- Undertaking the application for approval
- Geotechnical investigations
- Site wide water balance

1.6 Limitations

This report: has been prepared by GHD for Talison Lithium Pty Ltd and may only be used and relied on by Talison Lithium Pty Ltd for the purpose agreed between GHD and Talison Lithium Pty Ltd as set out in section 2 of this report.

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GHD has prepared the 12685165-GHD-01-77-MDL-CI-00001_Rev 01 ("Model") for, and for the benefit and sole use of, Talison Lithium Pty Ltd to support construction information only and must not be used for any other purpose or by any other person.

The Model is a representation only and does not reflect reality in every aspect. The Model contains simplified assumptions to derive a modelled outcome. The actual variables will inevitably be different to those used to prepare the Model. Accordingly, the outputs of the Model cannot be relied upon to represent actual conditions without due consideration of the inherent and expected inaccuracies. Such considerations are beyond GHD's scope.

The information, data and assumptions ("Inputs") used as inputs into the Model are from publicly available sources or provided by or on behalf of the Talison Lithium Pty Ltd, (including possibly through stakeholder engagements). GHD has not independently verified or checked Inputs beyond its agreed scope of work. GHD's scope of work does not include review or update of the Model as further Inputs becomes available.

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The Model is a customised model and not intended to be amended in any form or extracted to other software for amending. Any change made to the Model, other than by GHD, is undertaken on the express understanding that GHD is not responsible, and has no liability, for the changed Model including any outputs.

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2. Basis of design

2.1 General

The design objectives are annotated in Section 1.2 and the Scope of Work in Section 1.5.

The Basis of Design (BoD) noted in this section address the following Cell 1 redesign aspects:

- Geomembrane liner design – update to geometry
- Perimeter and divider embankment geometry
- Slope stability and seepage analysis
- Elevated drainage system
- Capacity review

There are no changes to the following aspects from the original RL 1275 m design (GHD, 2025a):

- Design principles, intent and philosophy
- Design standards and criteria
- Site background conditions (site location, climate, topography, surface water, geology, hydrogeology, previous land use, existing TSF landforms, powerlines arrangement, tailings retreatment plant positioning, site access)
- Tailings properties (no inconsistencies with previous assumptions were observed or reported to GHD)
- Underdrainage performance
- Seismic assessment
- Settlement and deformation assessment (was carried out for the ultimate TSF4 height)
- Water balance
- Tailings deposition strategy and tailings management principles
- Dambreak modelling (was carried out for the ultimate TSF4 height)
- Population at risk and ANCOLD consequence category (High B)
- Background geotechnical data, geotechnical parameters and liquefaction assessment
- Surface water management
- Closure design

Details for the above aspects are summarised in the original RL 1275 m design report (GHD, 2025a) and are not included in the addendum report.

2.2 Key assumptions

The RL 1277.5 m raise design is based on the following assumptions:

- The design parameters used in the RL 1275 m design (GHD, 2025a) is applicable except where indicated.
- The 7.5 m high TSF embankment raise will be constructed from mine waste rock and suitable subgrade material (for the installation of BGM liner) sources from Talison's mining operations
- A bituminous geomembrane (BGM) liner will be installed on the upstream perimeter embankment of the RL 1277.5 m raise and the Cell 2 side of the divider embankment. ES3 BGM will be used to line all the embankments
- The survey provided by Talison is and accurate reflection of the levels and conditions at TSF1. GHD has not independently verified the accuracy or correctness of the provided survey

- The completed construction of Cell 1 to RL 1270 m and Cell 2 to RL 1275 m has been undertaken in accordance with the design
- TSF1 stability is not in scope of this design, only interface with TSF4 was considered as needed
- TSF1 void at interface with TSF4 will be filled at least as noted in Section 5.2.2
- Beach for of the decant pond is assumed to be 0.5% – no bathymetry data is available at the time of the design
- Minimum of 10 m wide upstream sand platforms are required for construction of subsequent centreline raises of the RL1277.5 raise of the TSF4 Cell 1 northern embankment to the west of the causeway.
- Tailings required for constructing the compacted tailings sand platform will be harvested from deposited tailings material within TSF4
- Decant water is pumped to the Clear Water Pond (CWP). Water collected in the seepage collection sumps is pumped directly back into the TSF4 decant pond
- VWP data loggers are located outside of the RL 1295 m design footprint
- Tailings will be pumped from the Chemical Grade Plant 1 (CGP1), Chemical Grade Plant 2 (CGP2), Chemical Grade Plant 3 (CGP3), Technical Grade Plant (TGP) and Tailings Retreatment Plant (TRP) processing plants to the Central Tailings Pump Station (CTPS) and deposited in TSF4 via deposition pipelines and spigots.
- In some cases, tailings will be pumped directly to one of the cells in 'CTPS Bypass' and is deposited as a single point or spigoted discharge.

2.3 BoD Details

The Key BoD elements are noted in Table 2.1. Items not shown are assumed to remain unchanged from the RL 1275 m design (GHD, 2025a).

Table 2.1 Basis of design – Items affected or updated from the original design (GHD, 2025a) are shown.

Item	Value	Reference / basis
Existing crest heights (RL m)	Cell 1 perimeter – 1270 Divider and Cell 2 – 1275 (under construction)	Assumed constructed as per design (GHD, 2025a)
Cell 1 updated crest height (RL m)	1277.5	Crest height required to provide a minimum of 12 months storage
Cell 1 raise height (m) perimeter embankment	7.5	Raise from RL 1270 m to RL 1277.5 m
Raise height (m) divider embankment	2.5	Raise from RL 1275 m to RL 1277.5 m
Minimum required capacity of Cell 1	4.7 Mm ³ from January 2026	GHD redesign proposal "12685165-PRO-0_TSF4 Cell 1 Redesign to a Height of RL1277.5 m Density of 1.4 t/m ³ is assumed unchanged
Construction methodology for the RL 1277.5 raise of Cell 1 - eastern, western, and divider embankments –	Centreline	No change from the original design (GHD, 2025a)
Construction methodology for the RL 1277.5 raise of Cell 1 - northern embankment at TSF1 interface	Founded on existing TSF1 southern embankments and sand platform, not encroaching on the upstream side of TSF1 starter. TSF4 downstream toe located on TSF1 embankment crest. Infilling of TSF1 to be completed under Licence Amendment. For details of TSF1 interface refer to Section 5.2.1	As per email correspondence from Stefan Waldek to Don Blenkinsop on 12/12/2025

Item	Value	Reference / basis
Maximum tailings level (RL m)	1277.2	0.3 m operational freeboard from the new crest elevation
Freeboard level for pond water (RL m)	1276.6	0.9 m freeboard (GHD, 2025a) allowed from the updated crest elevation. Refer Section 5.6 for details
Maximum operating level	Pond to be maintained at least 200 m from embankments. RL 1272.4 m based on the base case beach slopes	Refer Section 5.6 for details
Perimeter embankment geometry (southern and eastern)	<ul style="list-style-type: none"> - Upstream and downstream slopes - 1(V):3(H) - Upstream lining - Bituminous Geomembrane (BGM) - Upstream BGM subgrade thickness 5 m - Crest width - 31.5 m - Sand platforms - minimum 10 m 	Unchanged from the original design (GHD, 2025a) Sand platforms extended to min 10 m to support constructability
Divider embankment width (m)	23	Standard divider embankment width (it had been reduced to facilitate deposition piping during cell 2 construction works)
Norther embankment geometry at TSF1 interface	<ul style="list-style-type: none"> - 1(V):3(H) slopes - Crest width 20m - Minimum BGM subgrade thickness 300 mm 	Refer to Section 5.2.1 for details
Beach slopes - subaerial	<ul style="list-style-type: none"> - 0-20 m: 5.5 % - 20-50 m: 4.5 % - 50-90 m: 2 % - 90 m onwards - 1.55 % 	Determined based on the November 2025 beach survey provided by Talison. Refer to Section 4.2 for details
Beach slopes - subaqueous	0.5 %	Assumed - there is no bathymetry data available for TSF4
Future raising sequence	<ul style="list-style-type: none"> Cell 1 raise to 1277.5 m (this design) Cell 2 raise to 1282.5 m Cell 1 raise to 1285 m Cell 2 raise to 1290 m 	Assumed 7.5 m raises to both cells

3. Current facility performance

3.1 General performance

The original RL 1275 m design report included a summary of facility performance based on the 6 months facility performance audit carried out in 2024 and 2024 groundwater monitoring results.

GHD reviewed monitoring data for Q1, Q2 and Q3 quarterly monitoring reports (Appendix C) and groundwater monitoring results for 2025. The review confirmed a satisfactory performance of TSF4 and no impacts on the redesign were identified.

3.2 Groundwater monitoring results

A total of 24 monitoring boreholes have been installed along the perimeter of TSF4 at 8 monitoring locations. The locations of the monitoring boreholes are shown in Figure 3.1.

The original RL 1275 m design report included a summary of facility performance based on the 2024 groundwater monitoring results

Talison provided additional groundwater monitoring data for 2025 (Appendix C). Available results for the TSF4 boreholes (Figure 3.1) are shown in Figure 3.2 to Figure 3.4 (combined, per metal) and Figure 3.5 to Figure 3.12 (per bore hole). The groundwater monitoring results indicate:

- Fluctuations of levels - line with the expected behaviour
- No increasing trend in Contaminants of Concern (COCs) is noted
- No impact on the Cell 1 redesign



Figure 3.1 Groundwater monitoring locations (GHD, 2025b)

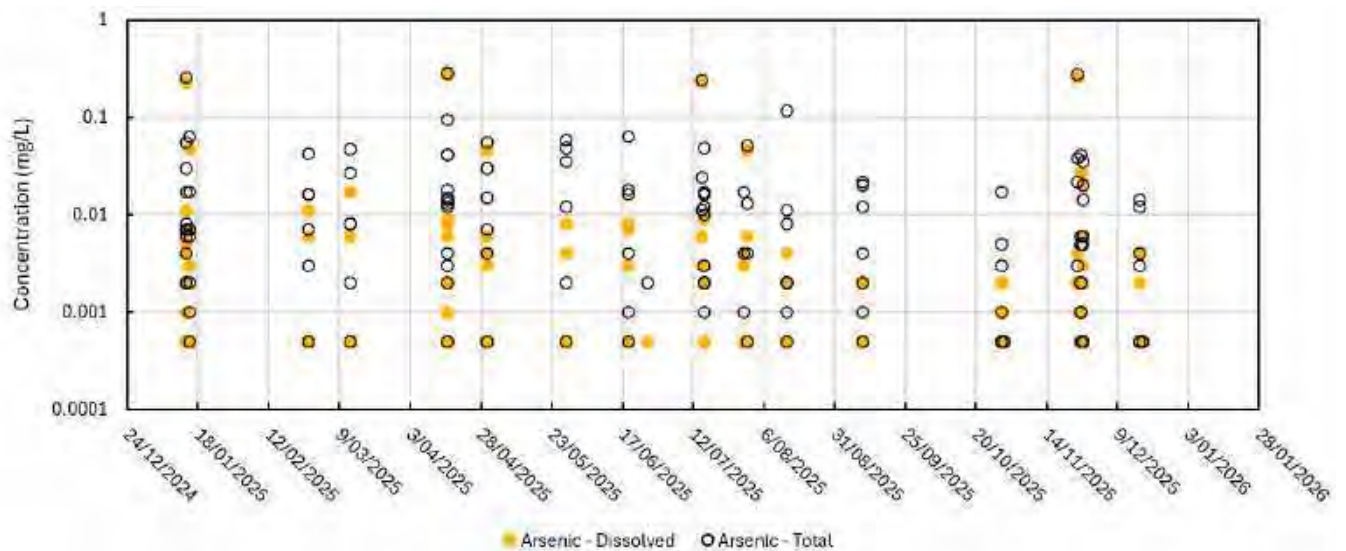


Figure 3.2 Arsenic concentration in TSF4 groundwater monitoring bores (Figure 3.1) based on provided data (Appendix C)

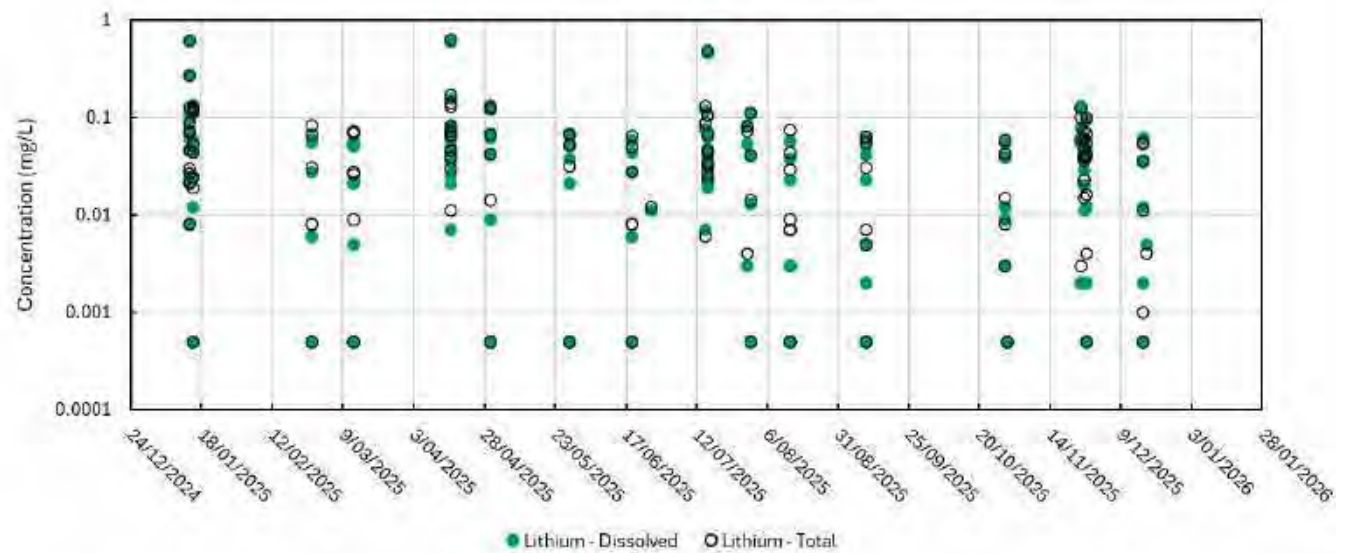


Figure 3.3 Lithium concentration in TSF4 groundwater monitoring bores (Figure 3.1) based on provided data (Appendix C)

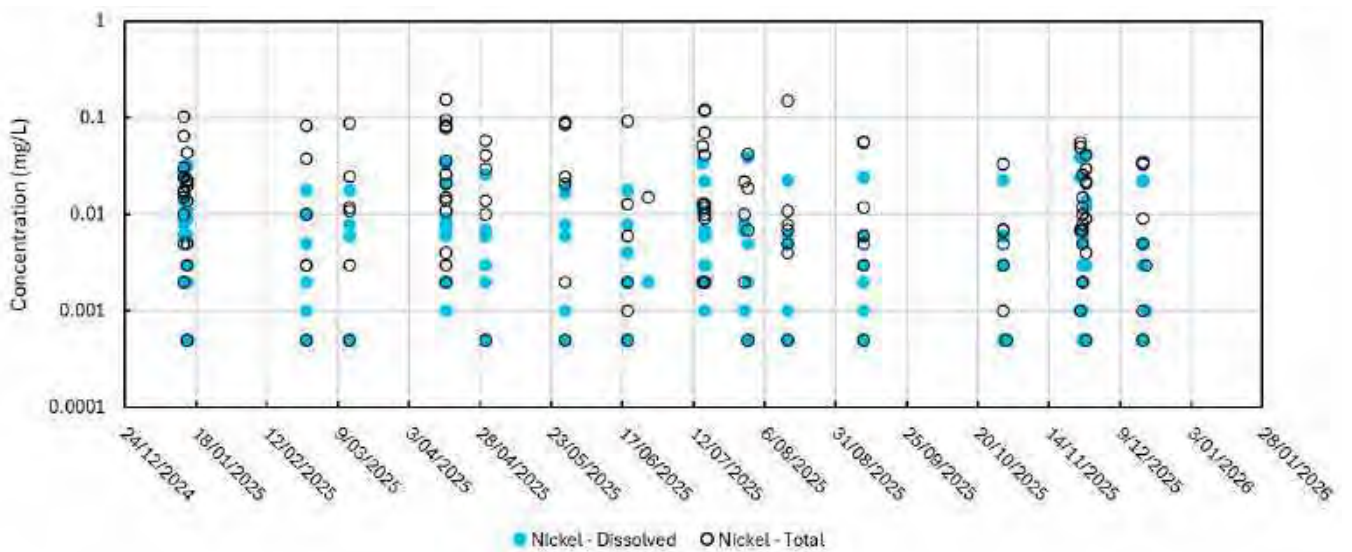


Figure 3.4 Nickel concentration in TSF4 groundwater monitoring bores (Figure 3.1) based on provided data (Appendix C)

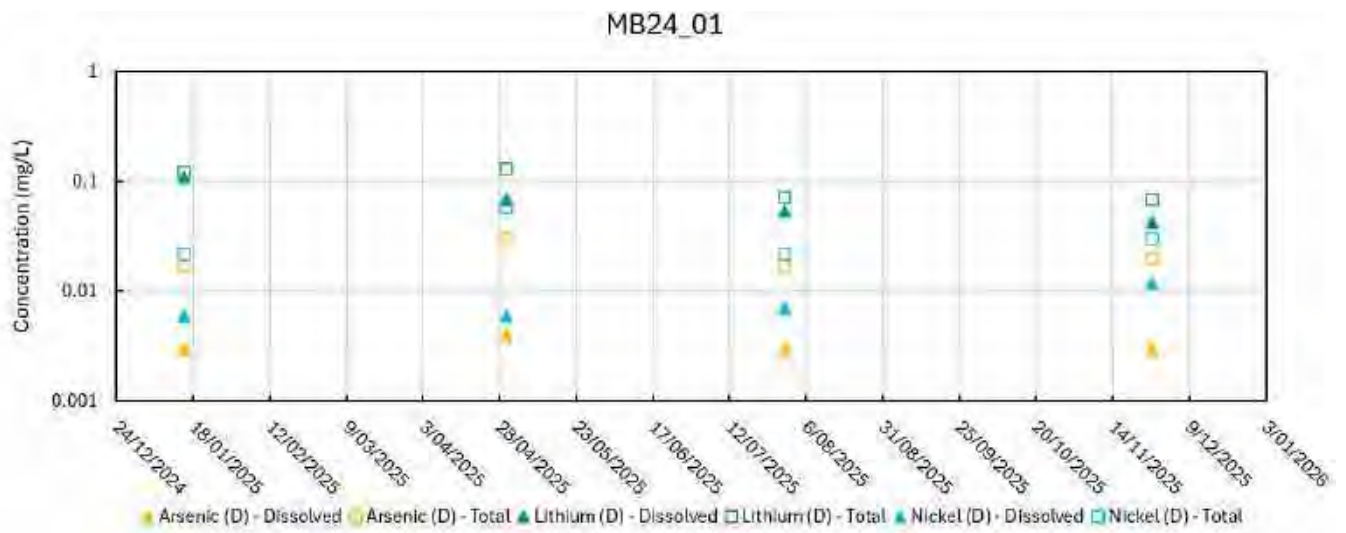


Figure 3.5 Arsenic, Lithium and Nickel concentration levels in MB24_01 (Figure 3.1)

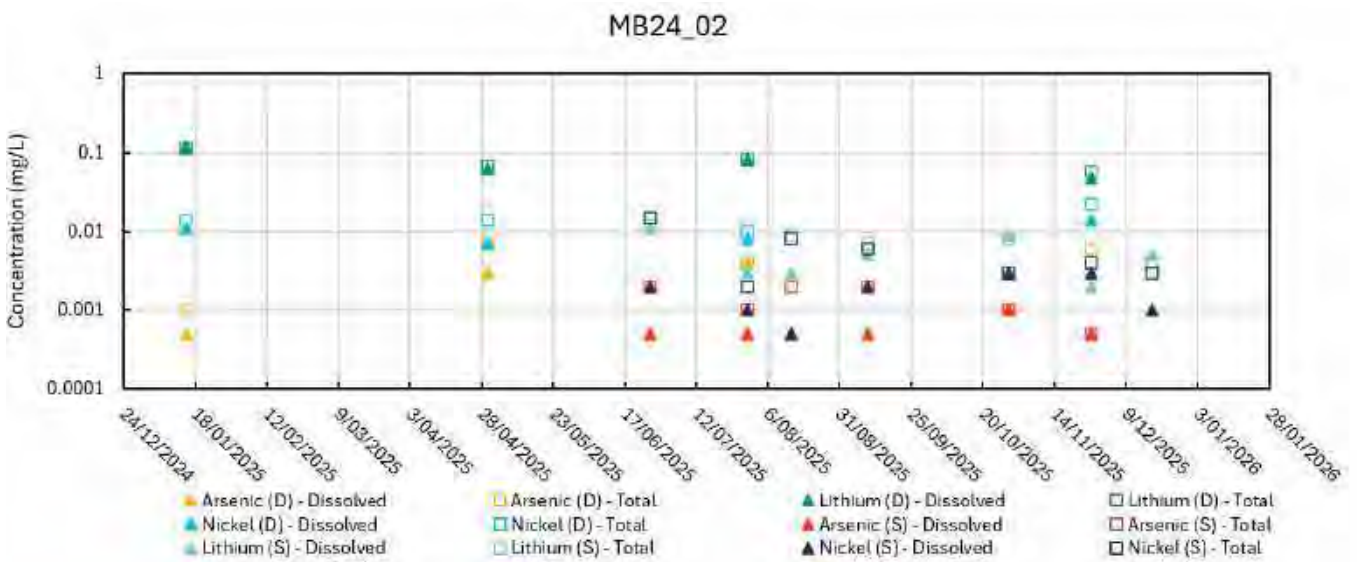


Figure 3.6 Arsenic, Lithium and Nickel concentration levels in MB24_02 (Figure 3.1)

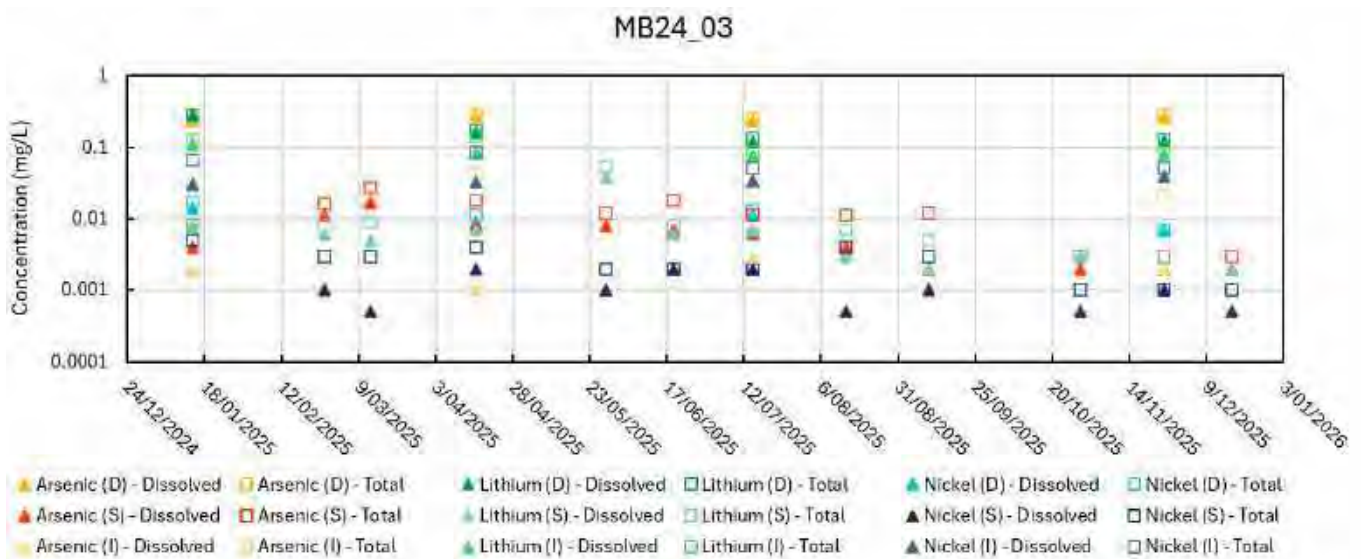


Figure 3.7 Arsenic, Lithium and Nickel concentration levels in MB24_03 (Figure 3.1)

MB24_04

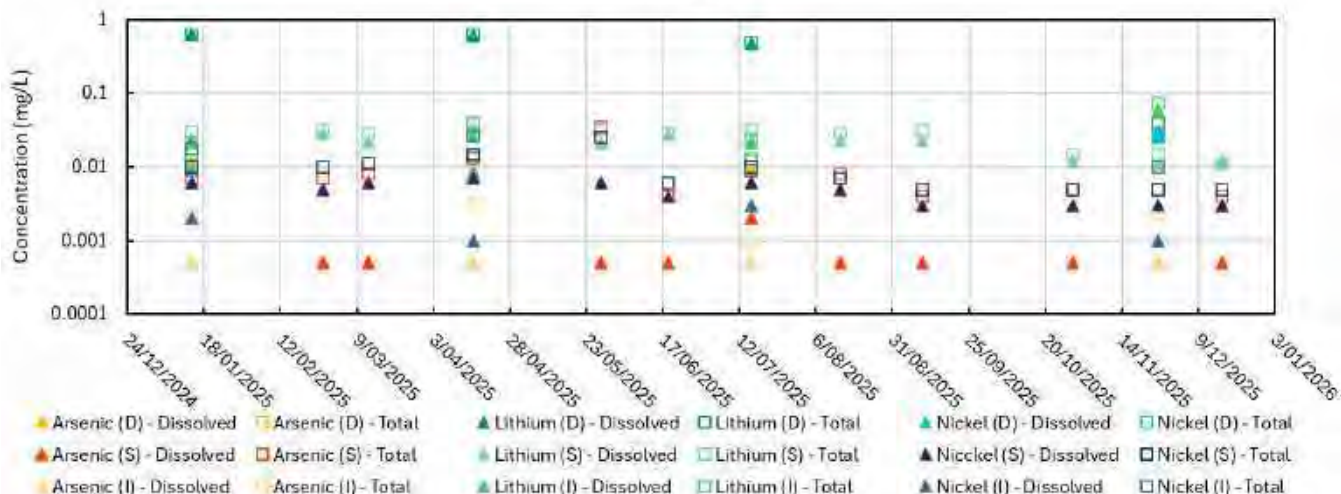


Figure 3.8 Arsenic, Lithium and Nickel concentration levels in MB24_04 (Figure 3.1)

MB24_05

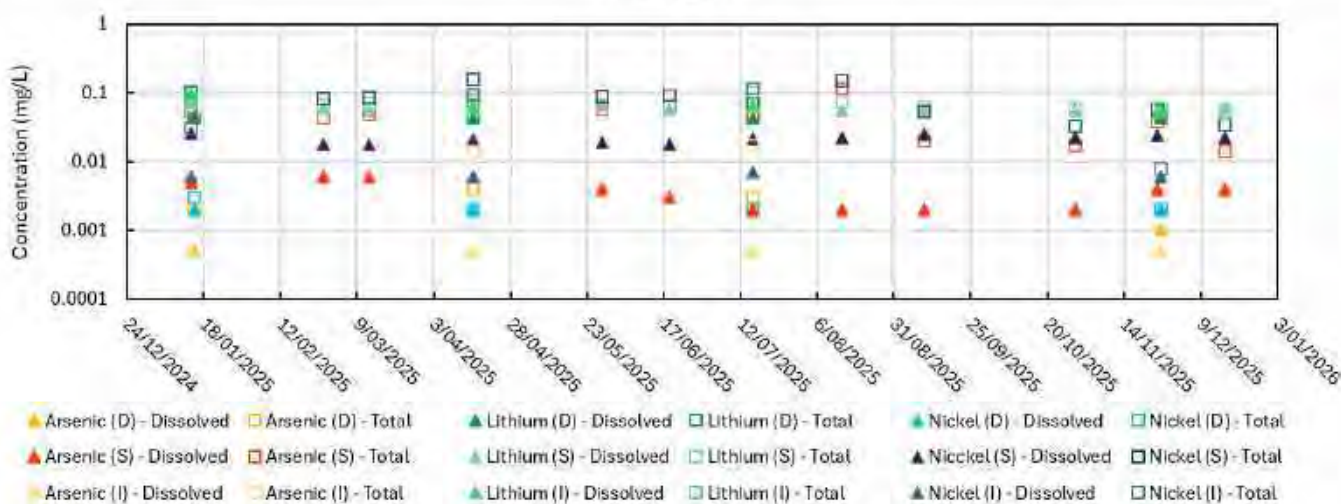


Figure 3.9 Arsenic, Lithium and Nickel concentration levels in MB24_05 (Figure 3.1)

MB24_06

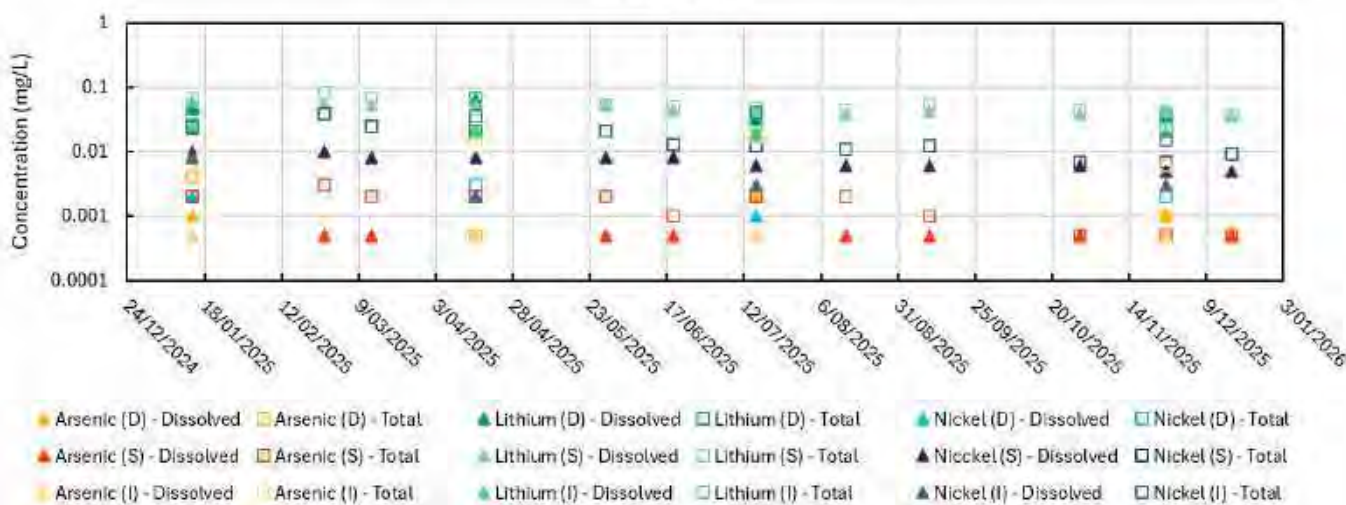


Figure 3.10 Arsenic, Lithium and Nickel concentration levels in MB24_06 (Figure 3.1)

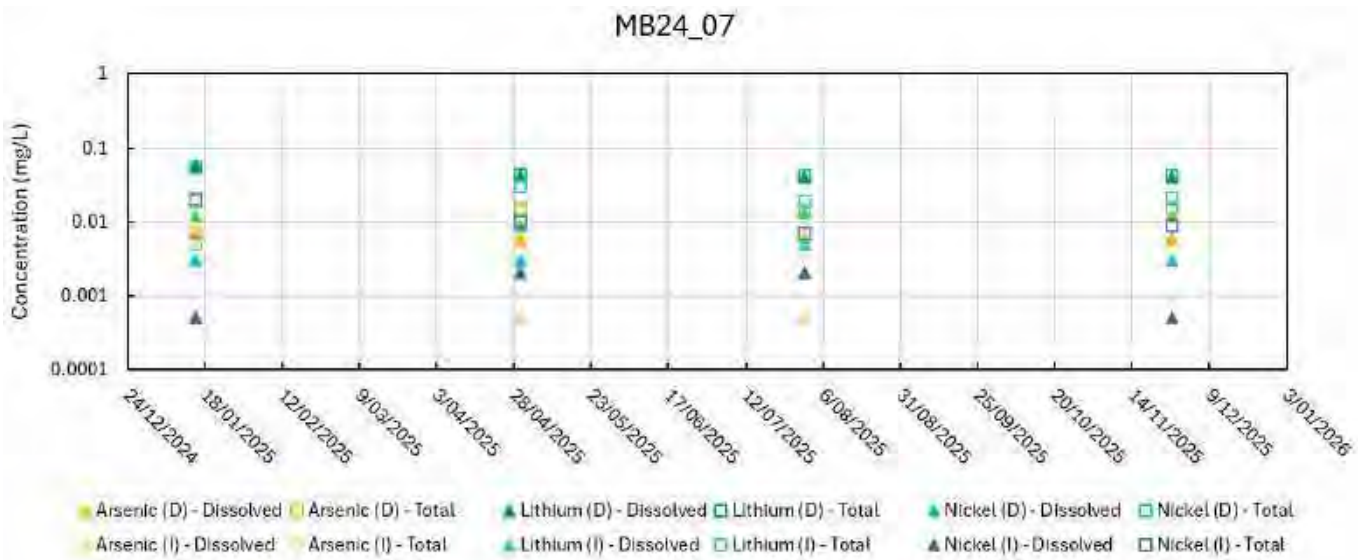


Figure 3.11 Arsenic, Lithium and Nickel concentration levels in MB24_07 (Figure 3.1)

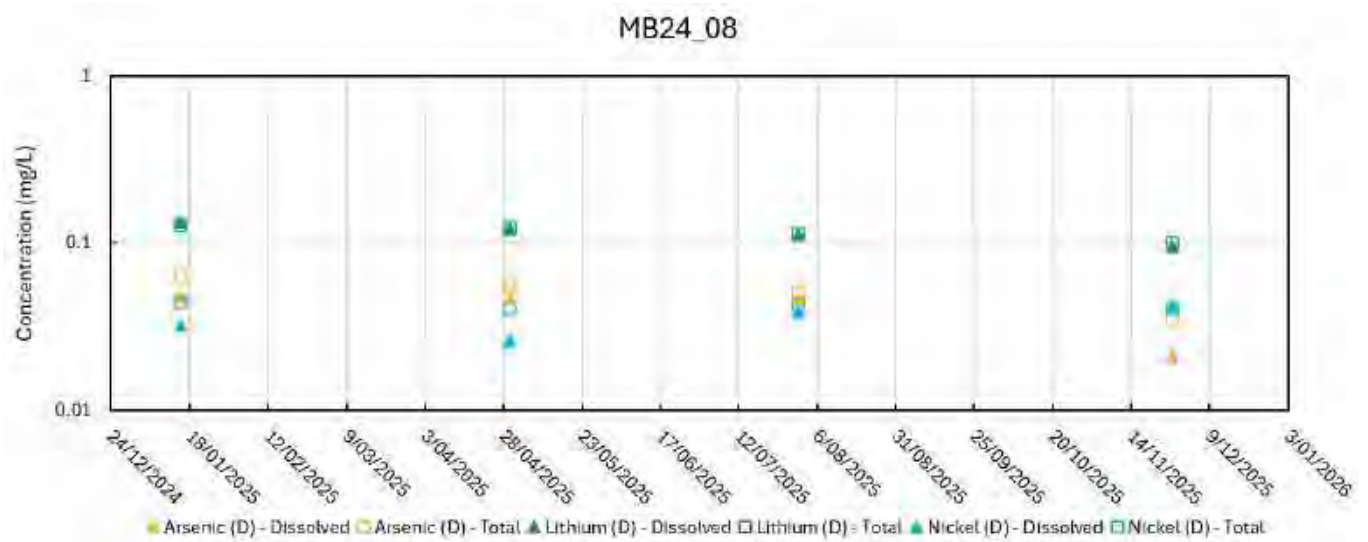


Figure 3.12 Arsenic, Lithium and Nickel concentration levels in MB24_08 (Figure 3.1)

4. Storage capacity review

4.1 General

The storage capacity review has been undertaken to support the redesign objective (Section 1.2). of providing at least 1 year of storage in Cell 1.

GHD utilised the Muk3D software to simulate the final tailings surface at Cell 1 and calculate the volume between the final tailings surface and the latest survey.

The storage capacity review comprised:

- Reviewing beach slopes from the provided survey data (Section 4.2)
- Modelling the embankment raise in 12D software and simulate the tailings beach at cell capacity in Muk3D software (Section 4.3)
- Sensitivity assessment to support the conservative approach (Section 4.4)

4.2 Beach slope review

Beach slope review was based on the November 2025 survey of Cell 1 provided by Talison. This survey was the latest available at the time of assessment and deemed more representative than earlier surveys more affected by the early stages of the TSF operation and initial beach establishment.

GHD analysed 15 sections shown in Figure 4.1 targeting top of the spigot location and direction perpendicular to the beach contours. Graphs are shown in Figure 4.2, with sections from west, east, north and south represented by different colour.

The beach slope review identified:

- Concave beach slope with relatively steep initial section
- Beach is generally well developed from south, west and north and trends can be observed
- Eastern beach slopes result in flatter grades; this is due to the topography involving current deposition from the hillside. Eastern beach is expected to unify when the hillside is covered with tailings as the deposition moves up
- Pond is positioned approximately 150 m from the embankment. There is no bathymetric data for TSF4
- The best fit and lower bound (steeper) beach slopes. The beach slope ranges are shown in Table 4.1 and Table 4.2
- GHD adopted best estimate beach slope for further assessment, with following assumptions:
 - Subaqueous beach slopes are assumed to be 0.5 % in line with the original design (originally based on historical observations)
 - Last grade observed in graphs is applicable up to the pond interface (relevant when pond is further away from embankments)
- Adopted beach shown in Figure 4.3 are:
 - 5.5 % for 20 m from the spigot
 - 4.5 % between 20 m and 50 from the spigot
 - 2 % between 50 m and 90 m from the spigot
 - 1.5 % between 90 m from the spigot and pond interface
 - 0.5 % - subaqueous

Table 4.1 Beach slope best estimate

Distance from spigot (m)	Beach slope (%)
20	5.50
50	4.50
90	2.00
150	1.55

Table 4.2 Beach slope lower bound (steeper)

Distance from spigot (m)	Beach slope (%)
35	6.86
50	4.67
75	3.40
150	1.65

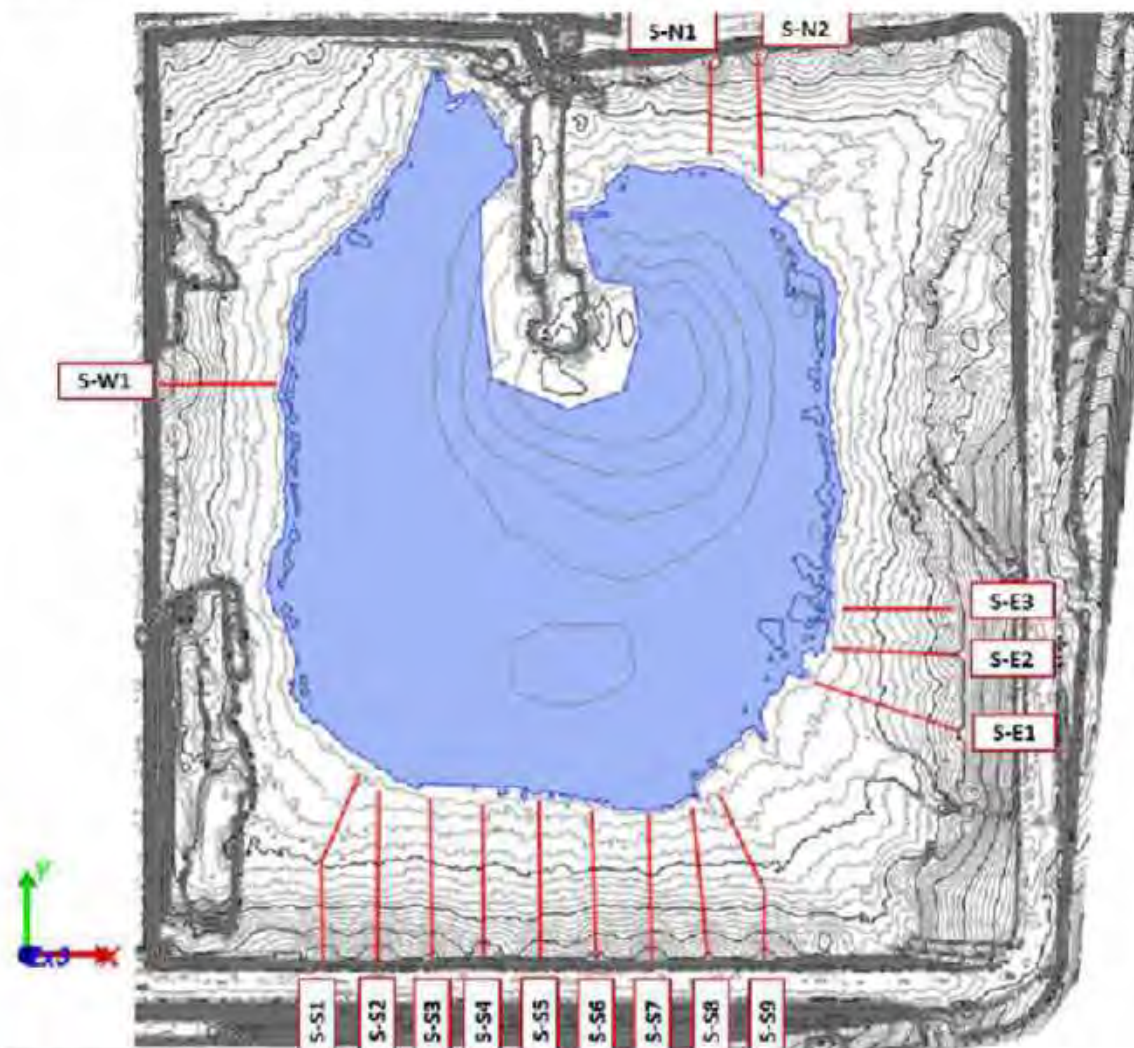


Figure 4.1 Sections reference for beach slope review (Nov 2025 survey)

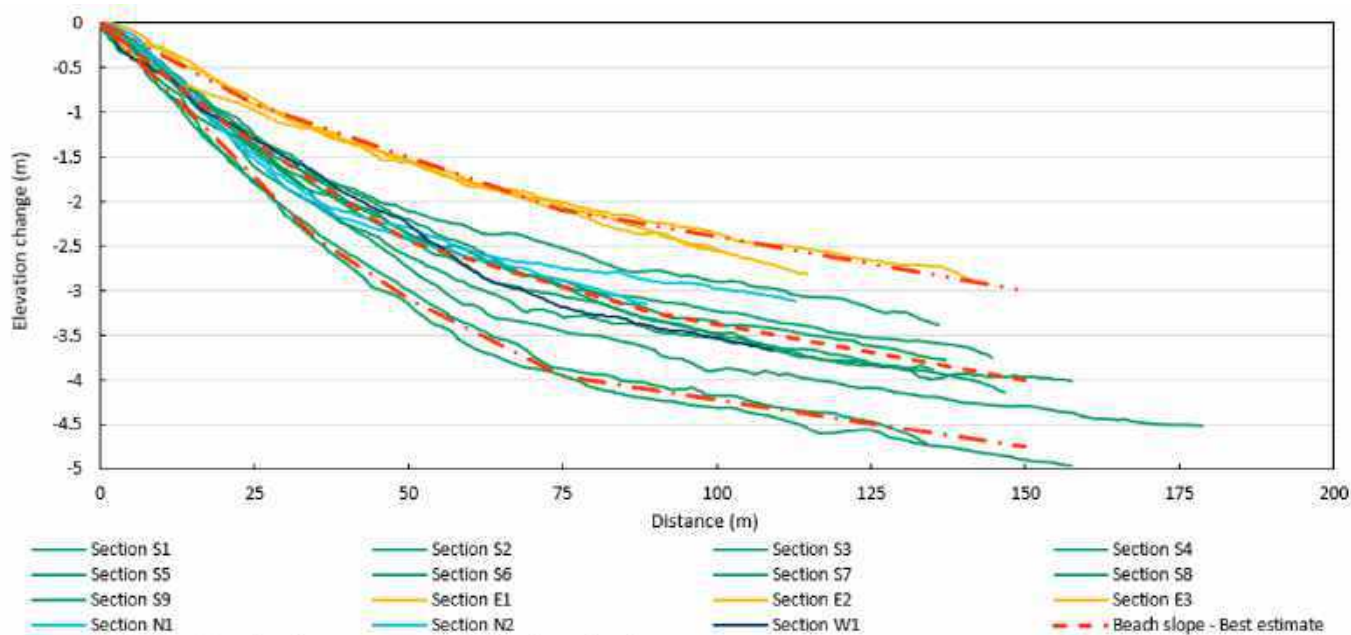


Figure 4.2 Beach slopes for locations shown in Figure 4.1

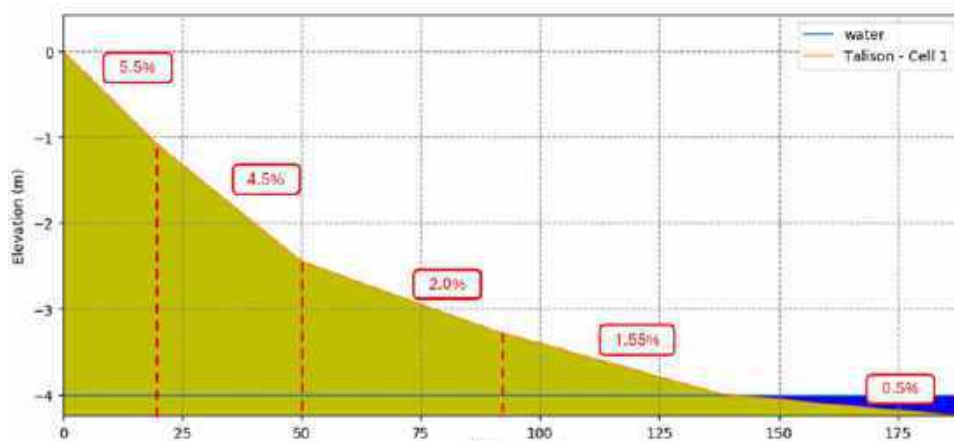


Figure 4.3 Adopted best estimate beach slope

4.3 Storage capacity

Storage capacity check has been carried out for the following case:

- Adopted best estimate beach slope (Figure 4.3)
- Tailings density of 1.4 t/m³ (settled dry density)
- Base surface - 05/01/2026 beach survey (latest available at the time of the capacity review)
- Maximum tailings elevation of 1277.2 m (0.3 m) below the crest in line with freeboard requirements (Section 5.6)
- Pond 200 m from embankments (0.5 % subaqueous beach slope has been manually added to the base surface)
- Spigot spaced at 50m centres (as observed)

Result indicates approximately **6 Mm³** (8.4 Mt) of available storage between January 2026 and Cell 1 capacity for tailings. An indicative tailings surface profile at capacity is shown in Figure 4.4

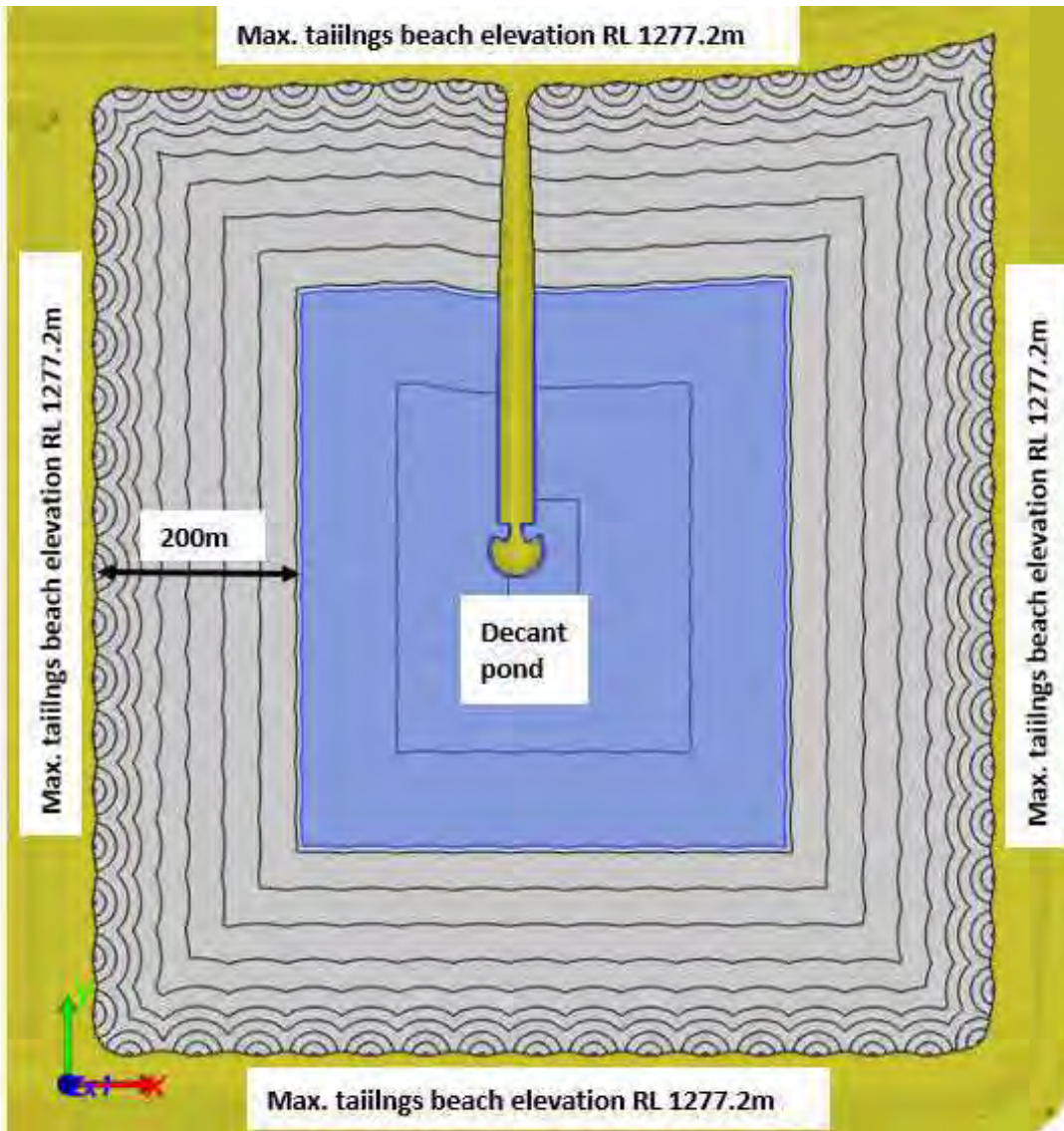


Figure 4.4 Tailings beach at capacity (result from Mik 3D software showing 0.5m contours)

4.4 Sensitivity

GHD carried out additional sensitivity analysis for the following scenarios:

- Fixed pond volume of 120,000 m³. This pond is closer to embankment than standard 200 m
- Steeper beach slope

Results shown in Table 4.3 indicate the target annual storage of 4.7 Mm³ is achieved for analysed cases and capacity of Cell 1 expected to be reached between February and April 2027. It is essential however to periodically verify the modelling input parameter and review the capacity.

Table 4.3 Sensitivity results summary

Item	Base case	Sensitivity 1 (shorter beach)	Sensitivity 2 (steeper beach slopes)
Tailings discharge elevation	RL 1277.2 m	RL 1277.2 m	RL 1277.2 m
Beach slopes (Section 4.2)	Best estimate "base case"	Best estimate "base case"	Lower bound - steeper
Decant pond assumption	200 m from embankments	Fixed volume of 120,000 m ³	200 m from embankments
Tailings beach length	200 m	153 m	200 m
Decant pond elevation	RL 1272.4 m	RL 1,273.0m	RL 1271.6 m
Decant pond volume	77,818 m ³	120,000 m ³	93,862 m ³
Decant pond area	207,496 m ²	279,773 m ²	235,140 m ²
Tailings storage capacity above January 2026 surface	5,996,381 m ³ / 8,394,933 t	6,088,145 m ³ / 8,523,403 t	5,432,113 m ³ / 7,604,958 t
Estimated time to capacity from Jan 2026 (based on 4.7 Mm ³ annual requirement)	15.4 months	15.6 months	13.9 months

5. Cell 1 raise re-design

5.1 Layout

The Cell 1 perimeter embankments will be raised via a centreline construction methodology from a crest level of RL 1270 m to RL 1277.5 m.

Cell 2 embankment is currently being raised to RL 1275 m as per the RL 1275 m design (GHD, 2025a).

Future centreline raises are planned to be undertaken in 7.5 m increments up to a final crest RL 1295 m, these raises are not included in the RL 1277.5 m design.

The RL1277.5 m layout is shown in Figure 5.1

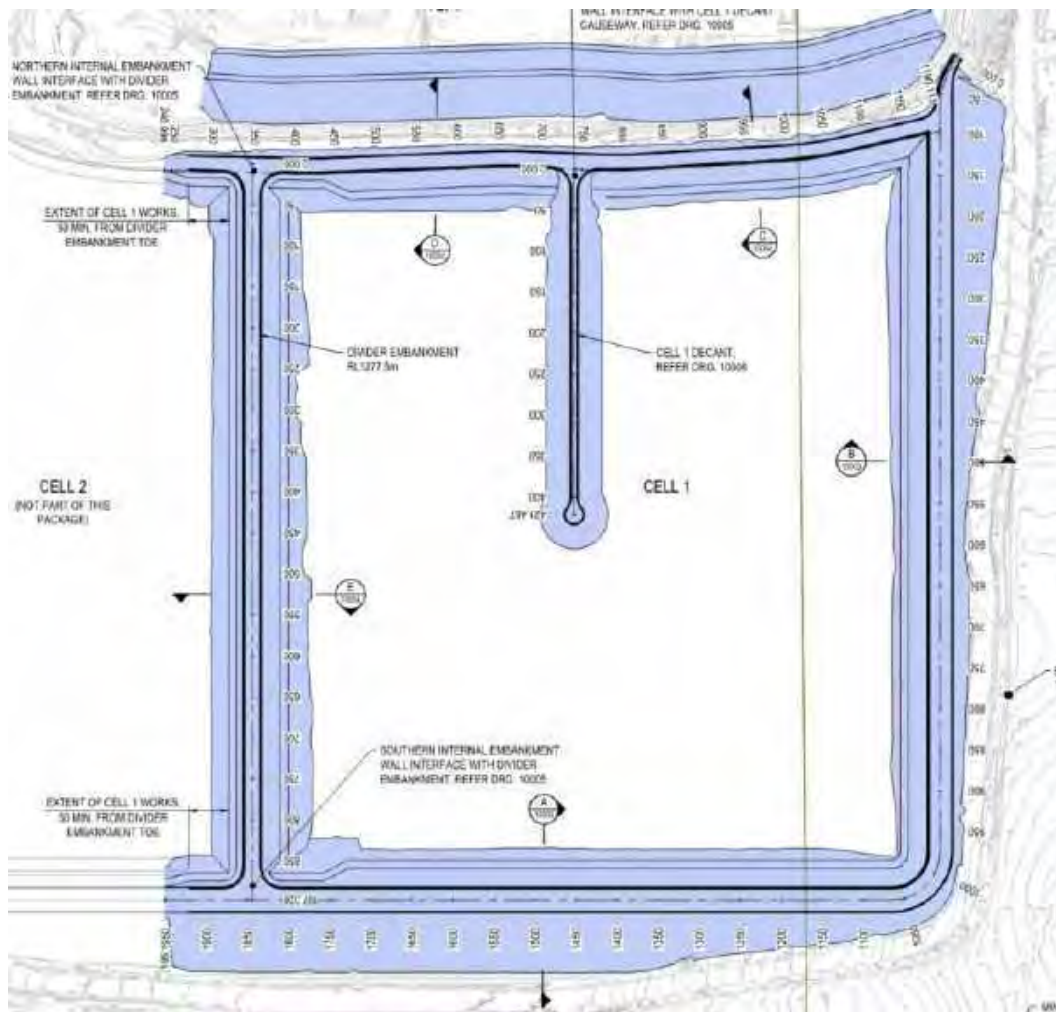


Figure 5.1 TSF4 Cell 1 layout

5.2 Embankment geometry

5.2.1 Perimeter embankment – south and east

The RL 1277.5 m perimeter embankment raise design uses a centreline construction methodology and crest width of 31.5 m matching previous raises. The embankment raise is primarily constructed from mine waste rock, and BGM is installed along the upstream face as a liner to provide containment of supernatant liquor.

The perimeter embankments' downstream slope will be constructed from mine waste rock at a slope of 3(H):1(V).

A clean tailings sand platform will be mechanically placed to RL 1270 m to provide a foundation to construct the RL 1277.5 m raise and allow for a working space to tie in the BGM liner.

The upstream slope will comprise an ES3 BGM ES3 liner placed over a nominal 5m thick layer of suitable subgrade material at a slope of 3(H):1(V). The ES3 BGM will be installed on top of the subgrade material from the top of the embankments (highest elevation) to the RL 1270 m embankment elevation. The ES3 BGM liner will be secured in an anchor trench at the crest of the RL 1277.5 m raise and welded to the ES1 BGM ES1 installed on the tailings sand platform. The ES1 BGM will be welded to the ES3 BGM installed as part of the RL 1270 m raise to provided containment.

A cushion geotextile and a protective clean tailings sand layer will be installed to protect this ES1 BGM liner from damage during the construction of the mine waste rock portion of the RL 1277.5 m raise.

A typical cross section is shown in Figure 5.2.

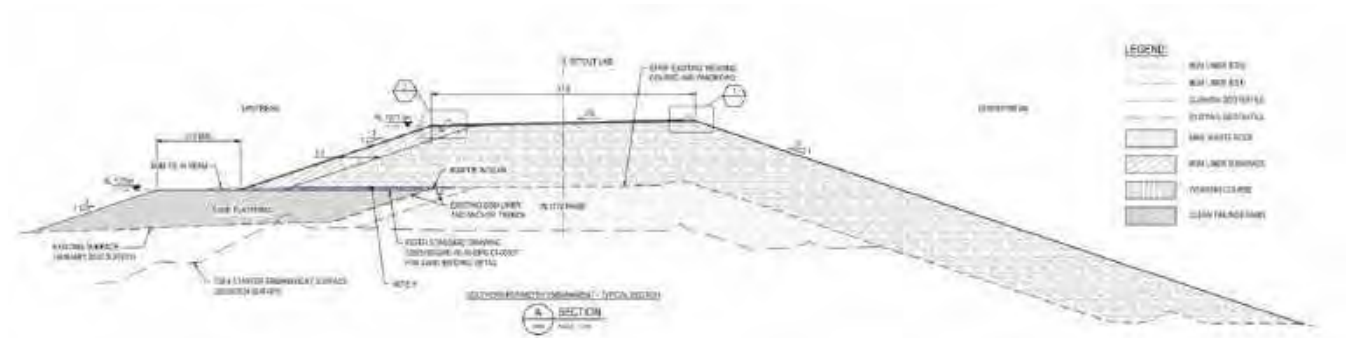


Figure 5.2 TSF4 Cell 1 raise perimeter embankment geometry

5.2.2 Northern embankment at TSF1 interface

The RL 1277.5 m northern embankment raise is designed to minimise encroaching of the TSF4 northern embankment on the upstream side of TSF1 starter embankment. The TSF1 starter embankment positioning is based on the interpretation of the internal geometry undertaken in the Recommissioning post remaining proof of concept report (GHD, 2020a). Distance of 20m has been applied from the current RL1270m embankment as shown in Figure 5.3.

The TSF4 RL 1277.5 m embankment is 20m wide, with 1(V):3(H) slopes, with BGM liner tied in to existing and extended to the face of the new raise. There are two different typical sections (Figure 5.1) identified along the TSF1 interface:

- Section C, typical for the eastern side of cell 1. Existing RL 1270m bench can accommodate the RL 1277.5 m raise of TSF4. Sand platform is added for constructability to provide enough construction room. Section C is shown Figure 5.4
- Section D, typical for the western side of cell 1, where sand platform partially supports the raise. Section D is shown Figure 5.5

TSF1 is currently being excavated (remining of tailings), with excavation expected to reach about RL 1264 m at the time of construction of the TSF4 Cell 1 raise to RL 1277.5 m. To support the stability of TSF1 embankment, infilling of the downstream side of the TSF4 Cell 1 northern embankment (upstream side of the TSF1 southern embankment) is required. Talison will Infill the void adjacent to the TSF4 Cell 1 northern embankment with mine waste rock to RL 1269.5 m. Approval for the infilling is currently being reviewed by the regulators under an application to amend the mining license. The TSF4 Cell 1 northern embankment will not be raised until the infilling has been completed.

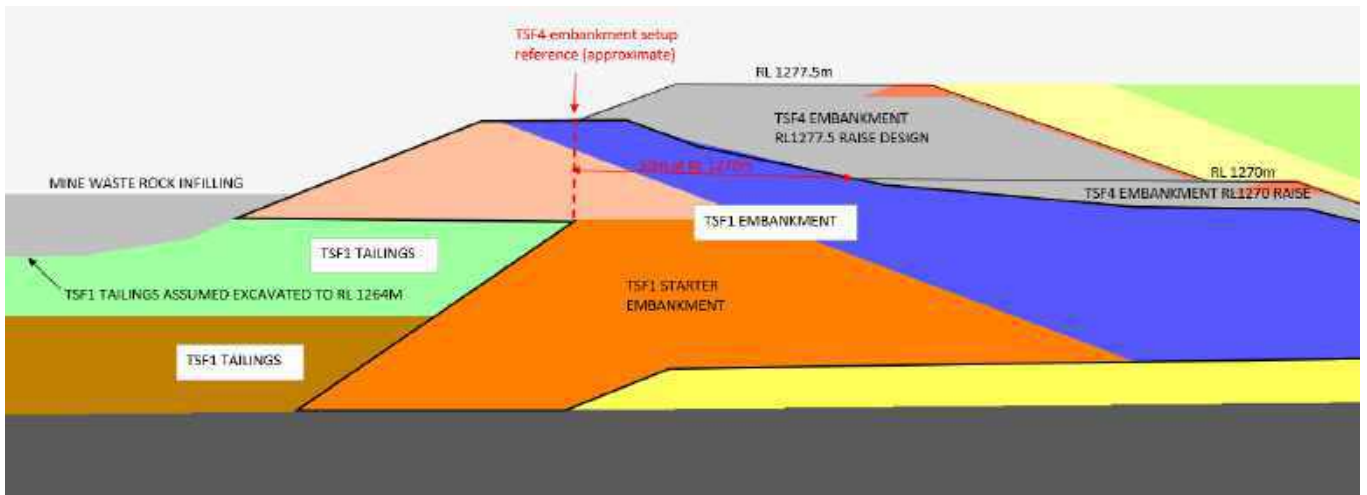


Figure 5.3 TSF4 northern embankment setup reference

The Infilling placement rate has been preliminarily assessed using Skempton's pore water pressure equation and the B-bar approach. The assessment concluded that the placement of the mine waste rock in **no greater than 0.5 m thick** lifts is required to avoid unacceptable pore water pressures developing. In addition, to allow for the dissipation of excess pore water pressure the subsequent lift can only be **placed 14 days after the preceding lift** had been placed.

Up to 6 vibrating wire piezometers (VWPs) will be installed in the footprint of the RL 1277.5 Infill to monitor the increase in pore water pressure in the underlying tailings. Continuous pore pressure measurements will be reviewed at the end of each shift and before the commencement of the next shift to ensure that increase in pore water pressure due to the loading is within acceptable limits. Before the placement of the subsequent lift the pore water pressure measurements will be reviewed to ensure that sufficient dissipation of the excess pore water pressures have occurred prior to placing the subsequent lift.

The Infilling placement rate has been assessed using the information available at the time of writing this report and will be reviewed and potentially optimised following the completion of a supplementary geotechnical investigation. In particular, residual excess pore water pressure in the tailings must be determined so that loading of TSF1 with future mine waste rock Infilling or ore storage can be undertaken safely and without causing static liquefaction of the underlying tailings.

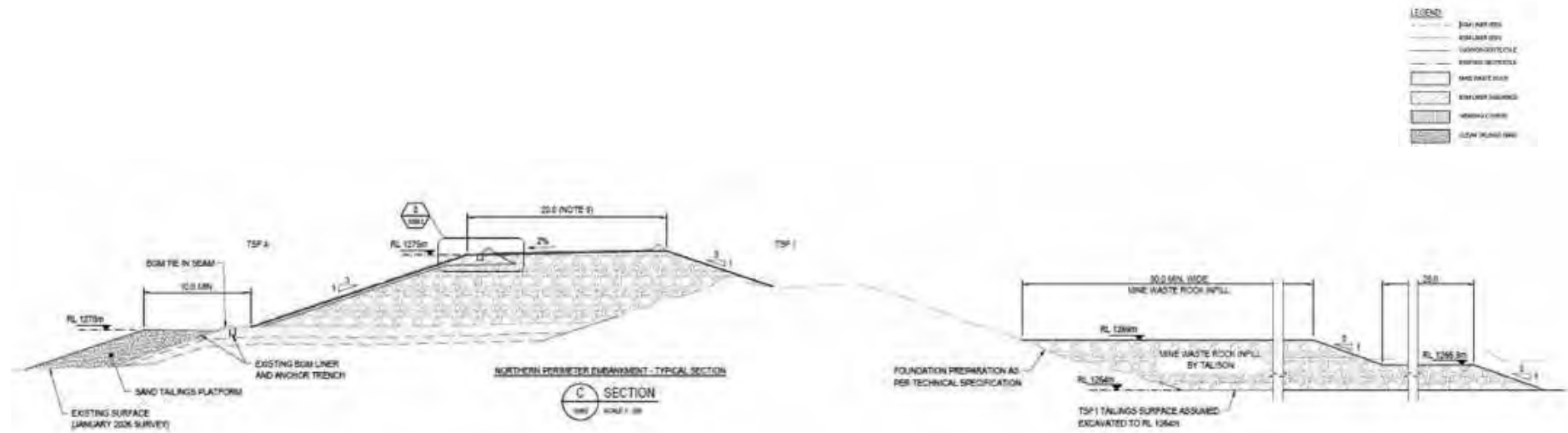


Figure 5.4 TSF4 Cell 1 raise, Section C

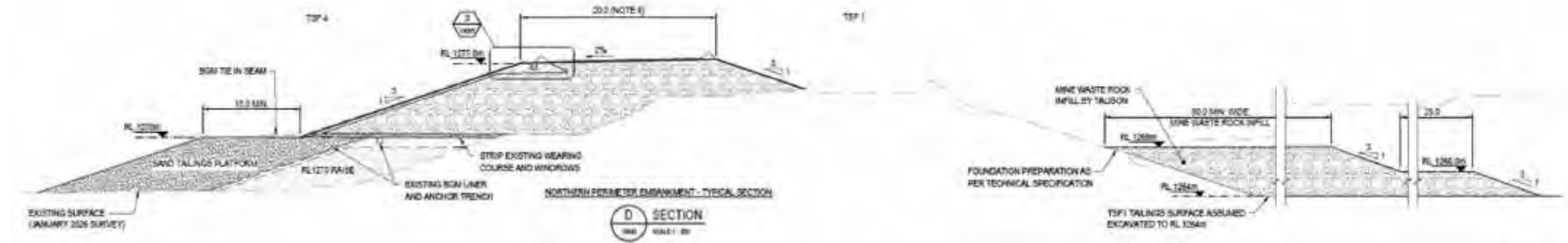


Figure 5.5 TSF4 Cell 1 raise, Section D

5.2.3 Divider embankment

The divider embankment is currently being raised to RL 1275 m in accordance with the RL 1275 m design (GHD, 2025a). Raise the divider embankment from RL 1275 m to RL 1277.5 m will comprise mine waste rock with a BGM liner on the Cell 2 face. A sand platform will be constructed on both sides of the divider to provide foundation for the raise. Typical section is shown in Figure 5.6

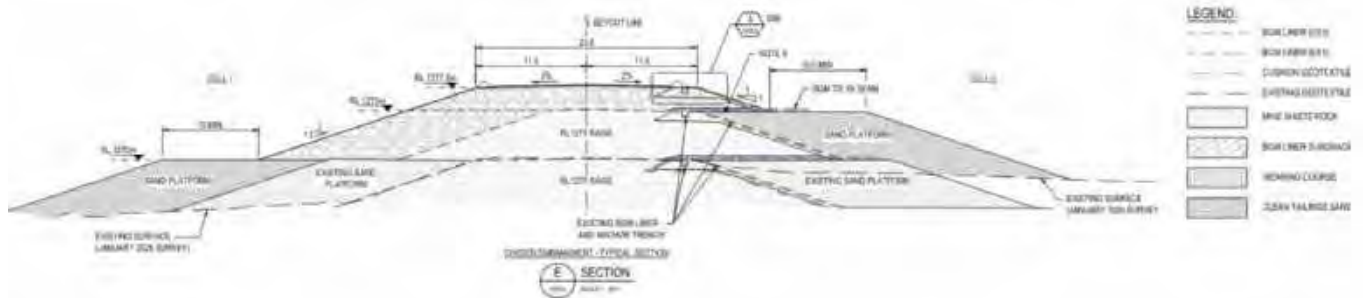


Figure 5.6 TSF4 Cell 1 raise, Section E – divider embankment

5.2.4 Decant causeway

The decant causeway is designed with a 10 m wide crest and 2 (H):1 (V) embankment slopes. The causeway shall be extended to reach middle of the pond and support decant recovery as shown in Figure 5.7

Tailings which are in, and adjacent to, the current decant pond are likely to be soft. The mine waste rock to be placed down to the original floor level of the TSF4 Cell 1. The construction methodology nominated by the contractor should consider safe construction methods, i.e. end tipping and placing mine waste rock over soft tailings areas.

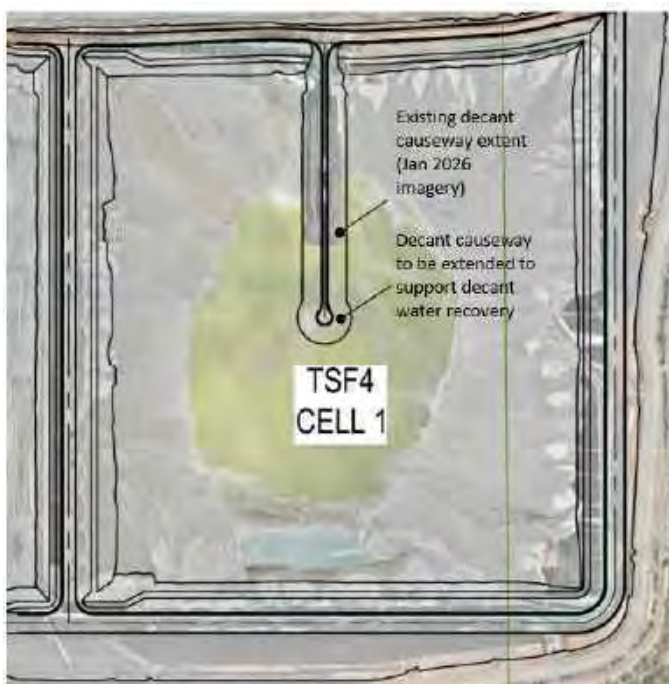


Figure 5.7 Decant causeway layout

5.3 Construction materials

There is no change to the construction material selection or placement methodology specified in the original RL 1275 m raise design (GHD, 2025a).

5.4 Seepage assessment

Seepage modelling was undertaken to estimate the phreatic surface through the embankment for used in the stability assessment (Section 5.5). Results are incorporated and shown in the stability assessment Figures (Appendix B).

There are no changes from the original RL 1275 m design (GHD, 2025a) to the following:

- Underdrainage – it is existing and sized for the RL 1295 m raise. No changes to the previously reported flow are expected due to the interim raise height change (RL 1277.5 m instead of RL 1275 m)
- Elevated underdrain objectives, elevation and details – as starting tailings elevation for this raise remains unchanged (RL 1270 m). Only the positioning of the drains has been shifted adjacent the northern wall, to accommodate northern embankment geometry changes.
- Material parameters or BGM permeability. Parameters have remained unchanged and are shown in Table 5.1 for reference

Changes were made to the TSF4 Cell 1 geometry (Section 5.2) and the maximum tailings/pond water level assumptions for the raise. The boundary conditions for the phreatic line development are annotated in Section 5.5 as needed.

Table 5.1 Parameters adopted in seepage analyses (GHD, 2025a)

Material description	Permeability (m/s)	Compressibility, Mv (/kPa)	Saturated Volumetric Water Content	Ky/Kx Ratio	Residual water content
Zone 1A – Clayey materials (BGM subgrade)	1×10^{-8}	3.33×10^{-5}	0.5	0.2	0.07
Mine waste rock	1×10^{-4}	1×10^{-5}	0.25	1	0
Tailings (inner sandy silt)	5×10^{-7}	5.41×10^{-5}	0.38	0.5	0.03
Tailings (outer beach sand)	1×10^{-5}	5×10^{-5}	0.38	0.5	0.03
Foundation layer 1 (laterite)	3×10^{-7}	4.72×10^{-5}	0.25	1	0.02
Foundation layer 2 (laterised saprolite)	1×10^{-7}	4×10^{-5}	0.3	1	0.03
Foundation layer 3 (pallid saprolite)	4.6×10^{-7}	2.86×10^{-5}	0.35	1	0.05
Foundation layer 4 (soft layer)	1×10^{-8}	2×10^{-4}	0.5	1	0.07
Foundation layer 5 (saprolite)	4.6×10^{-7}	2.86×10^{-5}	0.43	1	0.07
Foundation layer 6 (weathered basement rock)	2×10^{-6}	4.55×10^{-5}	0.24	1	0.01
Bedrock	5×10^{-8}	3×10^{-6}	-	1	-
BGM ⁽¹⁾	6×10^{-14}	-	-	-	-

(1) BGM liner was modelled as an impermeable boundary due to the high order of magnitude and material thickness

5.5 Stability analysis

5.5.1 General

Redesign to the new crest height affects the geometry of the cross sections (Section 5.2) and consequently the stability assessment has been updated. GHD has not identified any changes required to the following aspects of the RL 1275 m design (GHD, 2025a):

- Seismic assessment and loading
 - The Operational Basis Earthquake (OBE) is a 1 in 475 years' event with 0.07 g expected loading
 - The Safety Evaluation Earthquake (SEE) is a 1 in 5,000 years' (50th percentile) event with 0.406 g expected loading
- Liquefaction assessment:
 - Tailings are liquefiable under SEE loading
 - The compacted embankment is not considered liquefiable
 - Foundation materials are not susceptible to liquefaction

Slope stability analyses were conducted using GeoStudio Slope/W (Version 2024.2.1) software in conjunction with Seep/W. Limit equilibrium computer models were developed by adopting the Morgenstern-Price method of slices for all analyses. The figures showing the stability analysis results are included in Appendix B.

Process outline:

- Identify critical sections (Section 5.5.2)
- Apply new geometry (Section 5.2) and seepage boundary conditions (Section 5.5.3) for the RL 1277.5 m raise and the final RL 1295 m raise
- Select loading cases (Table 5.2)
- Apply geotechnical parameters as noted in Section 5.5.4
- Interpret results (Section 5.5.5)

The Factors of Safety (FoS) and shear strength parameters recommended by ANCOLD Guidelines on Tailings Dams (ANCOLD, 2019) were adopted and are shown in Table 5.2.

Table 5.2 Adopted FOS and shear strength parameters

Loading case	Recommended minimum FoS	Shear strength parameters	Additional comments – TSF4 specific
Long term drained	1.5	Effective strength	All material parameters are assumed to be drained.
Post seismic	1.0-1.2 (1.1 selected based on level of confidence in data used for analysis)	Post seismic shear strength	The low strength foundation material encountered is conservatively assumed to be undrained in southern embankment. For liquified tailings, the shear strength ratio was applied Sand platform and outer beach sand tailings are assumed not liquefiable. The elevated underdrainage, effective pond management and pore water pressure monitoring is required to reduce the likelihood of liquefaction. Where not considered potentially liquefiable, 20% strengths reduction was applied (foundation materials and outer beach sand tailings/ sand platform)
Post static	1.0-1.2 (1.1 selected based on level of confidence in data used for analysis)	Post static shear strength for liquified tailings	Post static liquefaction case The low strength foundation material where encountered are assumed to behave as undrained Inner sandy silt tailings are assumed to have liquified with shear strength ratio applied No 20% strength reduction applied

5.5.2 Section selection and geometry updates

GHD analysed five (5) critical sections shown in Figure 5.8 and summarised in Table 5.3. For geometry details refer to Section 5.2.

Table 5.3 Critical section selection and considerations

Section location (Figure 5.8)	Reason for selection	Additional considerations
A	Representative for southern embankment; includes highest Cell 1 section and soft foundation soils	Additional sensitivity case was analysed for the pond at freeboard level (Section 5.6) and 50m from the embankment
B	Representative for eastern embankment. The section was selected to correspond with the existing CPT locations.	Additional sensitivity case was analysed for the pond at freeboard level (Section 5.6) and 50m from the embankment
C	Representative for eastern side of northern embankment at TSF1 Interface	Downstream stability is critical at TSF1/TSF4 interface (downstream to TSF4, upstream to TSF1). TSF1 tailings are currently being excavated, with elevations of around RL 1264 m observed at interface (locally). Further excavation is anticipated to TSF1 tailings on downstream side of the embankment (remaining operations). The TSF4 tailings currently exceed RL 1266 m elevation within TSF1. Stability models are assuming tailings at TSF1 excavated to RL 1264 m with phreatic line just below the excavation (simulating ongoing dewatering). TSF1 comprises two tailings' layers – liquefiable top layer and cohesive bottom layer (GHD, 2020a). Minimum Infilling has been sized based on the stability assessment, to support the interface wall in case of the earthquake triggered tailings liquefaction. It shall be noted, that loading of the Infill must be undertaken to allow pore pressure dissipation in the bottom cohesive tailings layer.
D	Representative for western side of northern embankment at TSF1 Interface	
E	Representative for divider embankment	Analysed cases: <ul style="list-style-type: none"> – Construction case – Divider at RL 1275 m, Cell 2 tailings at RL 1275 m; Cell 1 tailings at RL 1270 m – Divider at RL 1277.5 m, Cell 2 tailings at RL 1275 m; Cell 1 tailings at RL 1277.5 m – Divider at RL 1295 m, Cell 2 tailings at RL 1295 m; Cell 1 tailings at RL 1292.5 m

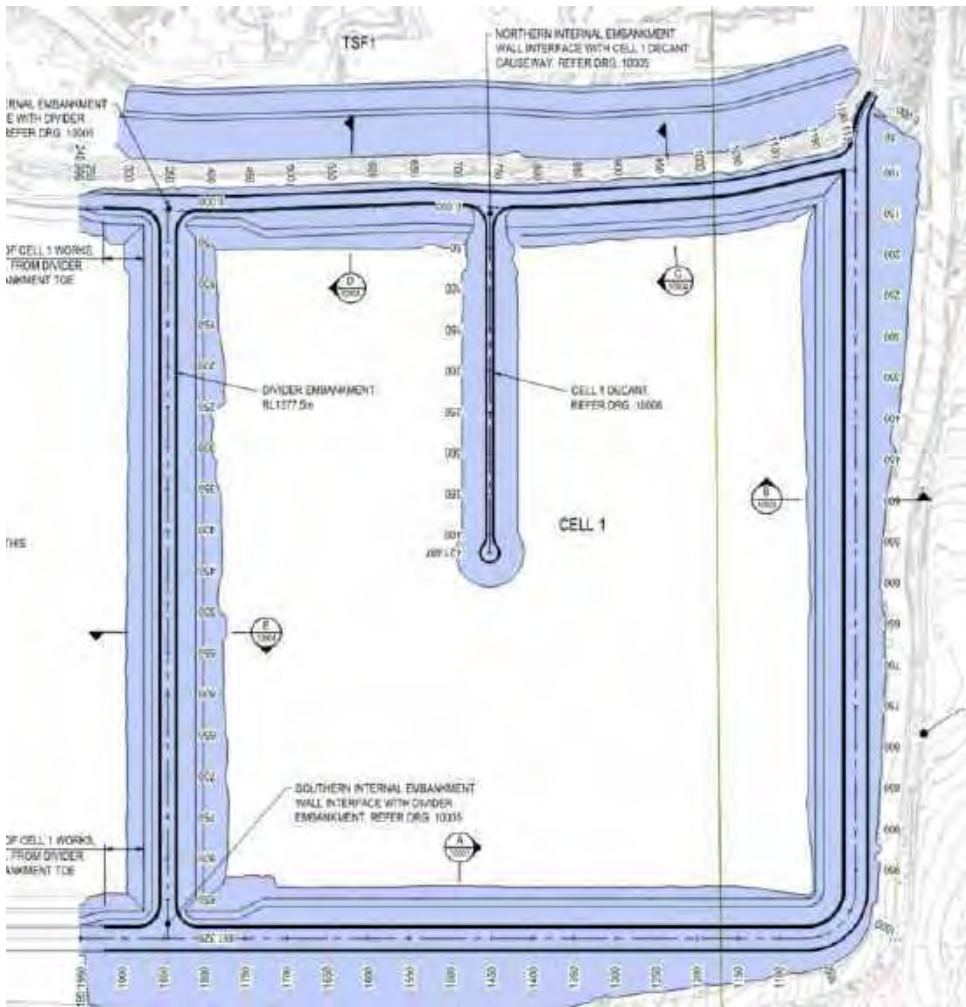


Figure 5.8 Stability sections location

5.5.3 Phreatic surface

GHD carried out seepage assessment (Section 5.4) to determine the phreatic surface for stability modelling, using following assumptions/inputs:

- Pond level at TSF4 assumed 1.5 m below the crest and 200 m from the embankment. This is considered conservative. Due to the observed beach slopes (Section 4.2), pond level at 200 m from the embankment is expected to be nearly 5 m below the crest.
- Additional sensitivity based on the current beach profile and observed pond distance (Section 4.2). The sensitivity check included:
 - Pond level at freeboard – 0.9 m below the crest (Section 5.6)
 - Pond 50 m from the embankment with working elevated drainage at RL 1270 m (included in this design)
- Groundwater table RL 1243 m in Section A and RL1262 m in Section B
- Effective underdrainage at the upstream toe of the starter embankment of TSF4.
- Pond level at TSF1 assumed reduced from original RL 1266.8 m to RL 1263.9 m (just below the observed excavations) due to dewatering during remaining operations

5.5.4 Material parameters

Parameters used in the original design (GHD, 2025a) have been applied to this assessment. Parameters are shown in Table 5.4 for reference.

Table 5.4 Material strength parameters adopted for stability assessment (GHD, 2025a)

Material description	Long-term			Post static			Post seismic		
	Unit weight (kN/m ³)	Friction angle, ϕ (°)	Undrained shear strength, SU (kPa)	Friction angle, ϕ (°)	Undrained shear strength, Su (kPa)	Su/σ ratio	Friction angle, ϕ (°)	Undrained shear strength, Su (kPa)	Su/σ ratio
Zone 1A – Clayey materials (BGM subgrade)	19	28	-	-	10	0.55	-	10	0.44
Mine waste rock	23	40	-	40	-	-	40	-	-
Tailings (inner sandy silt)	14.1	26	-	-	5	0.08	-	10	0.08
Tailings (outer beach sand)	16	28	-	28	-	-	22.4	-	-
Foundation layer 1 (laterite)	21	32	-	32	-	-	26	-	-
Foundation layer 2 (laterised saprolite)	20	28	-	28	-	-	22.4	-	-
Foundation layer 3 (pallid saprolite)	17	22	-	-	10	0.33	-	10	0.264
Foundation layer 4 (soft layer)	17	24	-	-	-	0.41	-	-	0.328
Foundation layer 5 (saprolite)	18	24	-	-	10	0.43	-	10	0.328
Foundation layer 6 (weathered basement rock)	21	27	-	27	-	-	22.4	-	-
Bedrock	24	-	500	-	500	-	-	500	-

5.5.5 Stability results and discussion

Results shown in Table 5.5 and Appendix B indicate:

- Proposed geometry (Section 5.2) results in satisfactory factors of safety within specified assumptions and criteria
- TSF1 interface requires minimum Infilling as shown in Section 5.2.2. Loading undertaken in 0.5m lifts with a minimum of 14 days between subsequent lifts will provide sufficient time for increased pore pressure to dissipate before the next lift is placed. The placement rate and thickness of the mine waste rock will be optimised once the supplementary geotechnical investigation and associated geotechnical numerical modelling has been completed.

Critical conditions such as pore pressures, underdrain performance, and beach drying should be confirmed during the TSF4 operation. The assumptions should be reviewed before the design of each lift.

Table 5.5 Stability analysis results

Section Number	TSF4 crest RL (m)	Loading Case	TSF4 Cell 1 Slope failure direction (US/DS)	Calculated minimum FoS	Minimum FoS (Table 5.2)	Minimum achieved (Y/N)	Figure reference (Appendix B)
A	1277.5	Long Term	DS	1.9	1.5	Y	Figure 1
A	1277.5	Post Static	DS	1.8	1.1	Y	Figure 2
A	1277.5	Post Seismic	DS	1.5	1.1	Y	Figure 3
A	1295	Long Term	DS	1.7	1.5	Y	Figure 4
A	1295	Post Static	DS	1.5	1.1	Y	Figure 5
A	1295	Post Seismic	DS	1.3	1.1	Y	Figure 6
A Sensitivity	1277.5	Long Term	DS	1.6	1.5	Y	Figure 7
A Sensitivity	1277.5	Post Static	DS	1.5	1.1	Y	Figure 8
A Sensitivity	1277.5	Post Seismic	DS	1.3	1.1	Y	Figure 9
B	1277.5	Long Term	DS	2.0	1.5	Y	Figure 10
B	1277.5	Post Static	DS	1.8	1.1	Y	Figure 11
B	1277.5	Post Seismic	DS	1.6	1.1	Y	Figure 12
B	1295	Long Term	DS	1.8	1.5	Y	Figure 13
B	1295	Post Static	DS	1.6	1.1	Y	Figure 14
B	1295	Post Seismic	DS	1.4	1.1	Y	Figure 15
B Sensitivity	1277.5	Long Term	DS	2.0	1.5	Y	Figure 16
B Sensitivity	1277.5	Post Static	DS	1.8	1.1	Y	Figure 17
B Sensitivity	1277.5	Post Seismic	DS	1.6	1.1	Y	Figure 18
C	1277.5	Long Term	DS (US to TSF1)	2.1	1.5	Y	Figure 19
C	1277.5	Post Static	DS (US to TSF1)	1.1	1.1	Y	Figure 20
C	1277.5	Post Seismic	DS (US to TSF1)	1.1	1.1	Y	Figure 21
C	1295	Long Term	DS (US to TSF1)	2.2	1.5	Y	Figure 22
C	1295	Post Static	DS (US to TSF1)	2.0	1.1	Y	Figure 23
C	1295	Post Seismic	DS (US to TSF1)	1.1	1.1	Y	Figure 24
C	1270 – existing – construction case	Long Term	US	3.9	1.5	Y	Figure 25
C	1270 – existing – construction case	Post Static	US	3.8	1.1	Y	Figure 26
C	1270 – existing – construction case	Post Seismic	US	3.8	1.1	Y	Figure 27
D	1277.5	Long Term	DS	2.1	1.5	Y	Figure 28
D	1277.5	Post Static	DS	1.3	1.1	Y	Figure 29
D	1277.5	Post Seismic	DS	1.3	1.1	Y	Figure 30
D	1295	Long Term	DS	2.5	1.5	Y	Figure 31
D	1295	Post Static	DS	2.4	1.1	Y	Figure 32
D	1295	Post Seismic	DS	1.2	1.1	Y	Figure 33
D	1270 – existing – construction case	Long Term	US	2.4	1.5	Y	Figure 34
D	1270 – existing – construction case	Post Static	US	2.4	1.1	Y	Figure 35
D	1270 – existing – construction case	Post Seismic	US	2.4	1.1	Y	Figure 36
E	Divider at RL 1275, Cell 1 tailings RL 1270	Long Term	US	2.1	1.5	Y	Figure 37
E	Divider at RL 1275, Cell 1 tailings RL 1270	Post Static	US	2.1	1.1	Y	Figure 38
E	Divider at RL 1275, Cell 1 tailings RL 1270	Post Seismic	US	1.8	1.1	Y	Figure 39
E	Divider at RL 1277.5, Cell 2 tailings RL 1275	Long Term	DS	2.3	1.5	Y	Figure 40
E	Divider at RL 1277.5, Cell 2 tailings RL 1275	Post Static	DS	2.2	1.1	Y	Figure 41
E	Divider at RL 1277.5, Cell 2 tailings RL 1275	Post Seismic	DS	1.8	1.1	Y	Figure 42
E	Divider at RL 1295, Cell 1 tailings RL 1292.5	Long Term	US	2.5	1.5	Y	Figure 43
E	Divider at RL 1295, Cell 1 tailings RL 1292.5	Post Static	US	2.4	1.1	Y	Figure 44
E	Divider at RL 1295, Cell 1 tailings RL 1292.5	Post Seismic	US	2.1	1.1	Y	Figure 45

5.6 Freeboard

5.6.1 Freeboard criteria

The decant pond is designed to be centrally located in Cell 1. There is no change to the freeboard criteria summarised in the original design (GHD, 2025a). The freeboard criteria for TSF4 Cell 1 and Cell 2 were informed by:

- ANCOLD Guidelines on Tailings Dam (ANCOLD, 2019). ANCOLD Guidelines recommend that the freeboard for a TSF be defined based on the facility’s consequence category. ANCOLD’s freeboard definitions are shown in Figure 5.9
- The DMIRS Guidelines (Department of Mines and Petroleum, 2015) also define the required freeboard for TSF in addition to the storm surge. DMIRS define an “operational freeboard” and a “beach freeboard” which together form the total freeboard as shown in Figure 5.10

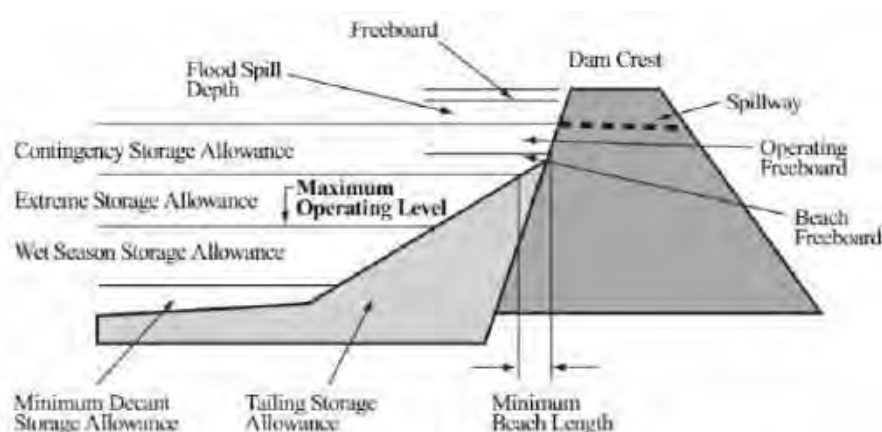


Figure 5.9 Freeboard definitions (ANCOLD, 2019)

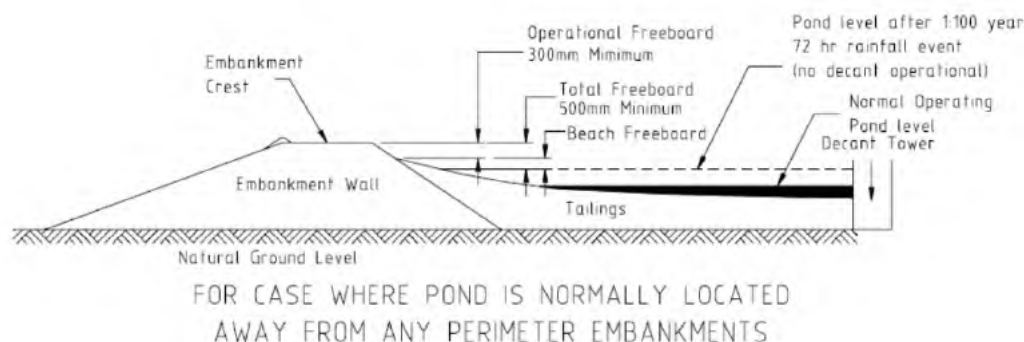


Figure 5.10 Definition of freeboard for TSF (Department of Mines and Petroleum, 2015)

5.6.2 Required freeboard

To comply with the DMIRS Guideline’s requirements the rainfall depth for a 1 in 100 year 72-hour rainfall event was estimated using Bureau of Meteorology (BoM) IFD data shown in Figure 5.11. The freeboard needed to meet the DMIRS Guideline’s requirements is 159 mm ($72 \times 2.21 \text{ mm/h} = 159 \text{ mm}$).

The ANCOLD Guidelines (ANCOLD, 2019) requires a High B consequence category TSF to safely contain rainfall and runoff of 1 in 1,000 year, 72-hour storm event. The 1 in 1,000 year 72-hour design rainfall depth was calculated to be 218 mm (refer to Figure 5.11). The 218 mm rainfall depth will create a 300 mm rise in decant pond water level.

The ANCOLD freeboard criteria is a more stringent requirement and was adopted for TSF4.

Additional allowance of 0.1 m was added for settlement based on the original design (GHD, 2025a). Settlement was estimated for the final embankment and can be applicable to the interim raises. It is suggested, however, that a numerical analysis be completed under the available Probabilistic Seismic Hazard Analysis (PSHA) to confirm the deformation settlement due to the differences in the results from Swaisgood and Seed & Makdisi, as these are both simplified methods. It is also recommended to verify this using the Finite Element Method, as these methods illustrate the deformations of the embankment rather than the foundations under seismic loads.

To comply with the freeboard requirements, it is necessary to maintain the normal operating pond level not less than 0.9 m below the embankment crest. The minimum freeboard components are summarised in Table 5.6.

It is important to note that beach slopes are anticipated to be concaved and steeper than in the original design. For details refer to Section 4.2. The 0.9 m freeboard level is illustrated in Figure 5.12. Due to the steep initial beach slopes, pond is expected to be against the wall when at freeboard. Decant pond shall be maintained at minimum 200 m away from embankments in line with the stability assessment (Section 5.5.5). Although sensitivity analysis indicates satisfactory FoS for pond 50 m from the embankment, it is important to avoid saturation to the sand platform and outer beach sand to reduce the liquefaction risk. It is also important to allow distance from the wall for the elevated drains to intercept the seepage. Therefore, pond level shall be managed in conjunction with the distance from embankments.

The key freeboard levels for the TSF4 are provided in Table 5.7.

Duration	Annual Exceedance Probability (1 in x)				
	1 in 100	1 in 200	1 in 500	1 in 1000	1 in 2000
24 hour	5.23	5.90	6.89	7.71	8.63
30 hour	4.45	5.06	5.88	6.55	7.22
36 hour	3.87	4.40	5.09	5.66	6.20
48 hour	3.09	3.47	3.98	4.43	4.81
72 hour	2.21	2.43	2.76	3.02	3.31
96 hour	1.73	1.89	2.12	2.30	2.52
120 hour	1.44	1.56	1.74	1.87	2.04
144 hour	1.25	1.35	1.48	1.59	1.72
168 hour	1.12	1.20	1.31	1.39	1.49

Figure 5.11 IFD Data for Greenbushes WA – mm/hour (BoM) as per original design (GHD, 2025a)

Table 5.6 Freeboard components summary

Operational freeboard (m)	Beach freeboard (m)	Additional allowance for settlement (m)	1,000-year storm storage allowance (m)	Total freeboard required (m)
0.3	0.2	0.1	0.3	0.9

Table 5.7 Key TSF4 Cell 1 levels for RL 1277.5 m raise

Item	Value (RL m)
Crest elevation	1277.5
Maximum tailings beach level	1277.2
Freeboard level for decant pond	1276.6
Maximum Operating Level (MOL)	Whichever is lower: <ul style="list-style-type: none"> - Freeboard (1276.6) - Pond level 200 m from embankments at tailings capacity (estimated approximately 1272.4 based on Table 4.3). This will be reviewed as the beach profile flattens

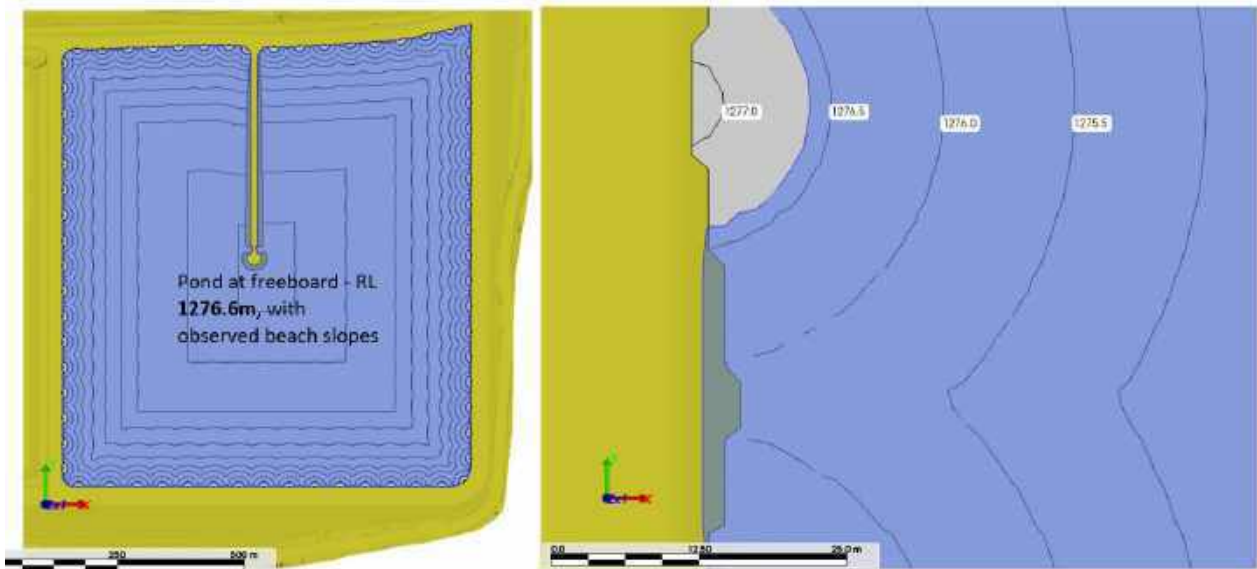


Figure 5.12 Pond at freeboard

5.7 Monitoring instrumentation

Vibrating wire piezometers (VWPs), survey monitoring points, inclinometers and monitoring bores were installed as part of the previous raises. One VWP is yet to be installed as shown in Drawings (Appendix A).

The RL 1277.5 m design includes provision for the following modifications to the existing monitoring infrastructure:

- Protection of the existing infrastructure
- Raising the settlement survey markers
- Raising the inclinometers

Data logger relocation was specified in the RL 1275 m design (GHD, 2025a). This work has already been completed.

The settlement survey markers are designed to be raised by adding threaded rods to extend the survey rod above the embankment raise. The survey rod is housed within a PVC pipe and the PVC pipe will also be extended as part of subsequent embankment lifts. Each marker is protected by a compacted earthen bund. The extended rod will be supported by the placement of cracker dust around the rod within the PVC pipe sleeve.

Due to the disturbance of the survey marker at each raise, there will be a period of re-calibration after each raise. The survey marker should be resurveyed after it has been raised to provide a new baseline for the ongoing dam safety monitoring.

The design includes raising the inclinometer by extending the casing to 1.5 m above crest level and constructing a protective windrow around the casing. To stabilise the inclinometer casing the area within the window was designed to be backfilled with compacted crusher dust.

6. Surface water management

No modifications to the surface water management system in the RL 1275 m design (GHD, 2025a) is required.

The design of the drain and connection chamber/pump chamber and the installation and commissioning of the extension drain is excluded from the RL 1277.5 m raise design and will be undertaken by others.

The change is not affecting the limits of this design assuming that the extension will maintain or exceed the existing flow capacity, and acknowledge the loads planned above the pipe (waste rock dump).

7. Construction

No modifications to the foundation preparation, access roads, materials, construction methodology or technical specifications are required to the original design (GHD, 2025a). Changes are generally related to the crest elevation and zone geometry.

It shall be noted however that:

- The divider embankment starting elevation changes to RL 1275 m (being constructed to this elevation as part of Cell 2 works,
- TSF1 interface will require supporting TSF1 stability and Infilling. Construction methodology for the Infilling is to be determined as noted in Section 5.5 and Section 9 to support the pore pressure dissipation in TSF1 tailings
- The RL 1275 m embankments of Cell 2 and RL 1277.5 embankments of Cell 1 need to be constructed and BGM lined in a sequence that provides continuous impermeable boundary. The construction contractor's detailed methodology must demonstrate that this can be achieved

8. Safety in design

GHD has reviewed the Safety in Design (SiD) prepared for the original design (GHD, 2025a). Safety measures are considered valid for the updated crest elevation. Following additional safety measures have been identified and included in the design:

- Field trials and assessment of the loading to TSF1 tailings and associated pore pressure dissipation rate is recommended to reduce the risk of TSF1/TSF4 slope failure and further cascade failure
- The raise of the northern wall is designed to reduce the risk of encroachment on the TSF1 tailings (as detailed in Section 5.2.2)
- Further excavations in the adjacent TSF1 can result in instability of the interface wall. Additional stability assessment to be carried out if the excavations continue beyond the design assumptions
- Additional requirement has been added to the MOL to consider the pond distance from the embankments

The updated SiD Register is included in Appendix E. Updates are relevant to Cell 1 redesign only.

GHD formally hands responsibility for the residual risks to Talison for further mitigation and trusts that Talison will complete the safety in design review process for the phases of this project within your responsibility. The combined safety in design documents should be provided to each of the parties who may be identified as being able to influence design. The safety in design risk assessment should be continually updated to reflect the current risks associated with all current activities associated with the asset.

9. Conclusions and Recommendations

Following conclusions and recommendations are noted:

- Cell 1 at new height (crest RL 1277.5 m) is expected to accommodate at least one (1) year of deposition as noted in Section 4, therefore the redesign meets the objective noted in Section 1.2. This is based on assumption that production rates and tailings properties remain unchanged. If tailings properties change, this may be subject to verification.
- TSF1 Infilling with mine waste rock is required at TSF4 Cell 1/TSF1 interface, to support TSF1 stability (and therefore northern wall of TSF4 Cell 1).
- If at the time of construction TSF1 condition varies from assumed in this design, additional verification of the geometry may be required.

10. References

- ANCOLD. (2019). *Guidelines on Tailings Dams - Planning design construction operation and closure*.
- Department of Mines and Petroleum. (2015). *Guide to preparation of design for TSFs*.
- GHD. (2020a). *TSF1 Recommissioning Post Remining; Proof of Concept Report*.
- GHD. (2025a). *TSF4 RL 1275m Raise; Detailed Engineering Design Report*.
- GHD. (2025b). *Quarterly Monitoring Report (July to September 2025)*.

Appendices

Appendix A

Drawings



PLAN
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FOR INFORMATION ONLY
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- NOTES:**
- ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
 - ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
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- LEGEND:**
- BOREHOLES
 - GROUND WATER BOREHOLES
 - TEST PITS
 - CPT LOCATIONS

BOREHOLES & GROUNDWATER BOREHOLES CO-ORDINATES TABLE

POINT ID	EASTING	NORTHING	RL
BH 01	413724.5	6249677.5	1285.7
BH 02	413379.5	6249580.5	1247.2
BH 03	413228.9	6249641.5	1249.7
BH 04	412860.5	6249642.5	1258.8
BH 05	412343.7	6250684.5	1250.8
BH 06	412067.7	6250660.5	1284.9
BH 07	412634.3	6250610.5	1257.0
BH 08	412627.7	6250486.5	1260.2
BH 09	412089.7	6250049.5	1287.2
BH 10	412086.5	6249744.5	1274.8
BH 11	412546.9	6250174.5	-
BH 12	412861.8	6249646.5	1260.9
BH 13	412528.8	6249653.5	1265.0
BH 14	413278.4	6249666.5	1251.5
BH 15	413892.4	6250488.5	1262.0
BH 16	413828.9	6250126.5	1270.2
GW 01D	412111.1	6250106.5	1269.5
GW 01M	412110.4	6250108.5	1269.4
GW 01S	412109.5	6250107.5	1269.4
GWD 02D	412865.7	6250058.5	1262.6
GWD 02M	412865.1	6250057.5	1262.5
GWD 02S	412866.7	6250056.5	1262.6
GWD 03D	413458.0	6249913.5	1251.1
GWD 03M	413456.7	6249912.5	1251.1
GWD 03S	413455.0	6249911.5	1250.9

TEST PIT CO-ORDINATES TABLE

POINT ID	EASTING	NORTHING
TP 01	412601.0	6249779.0
TP 02	412601.0	6250089.7
TP 03	412601.0	6250294.6
TP 04	412351.0	6249914.6
TP 05	412351.0	6250121.2
TP 06	412351.0	6250331.2
TP 07	412609.2	6250644.3
TP 08	412270.4	6250644.3
TP 09	412101.0	6250518.7
TP 10	412101.0	6250261.0
TP 11	412101.0	6250051.7
TP 12	412101.0	6249773.2
TP 13	412226.0	6249649.2
TP 14	412727.1	6249649.2
TP 15	412973.7	6249649.2
TP 16	412853.2	6249744.4
TP 17	413033.9	6249795.4
TP 18	413033.9	6250002.5
TP 19	412853.2	6250201.7
TP 20	413033.9	6250206.2
TP 21	412853.2	6250424.2
TP 22	413033.9	6250417.7
TP 23	413251.8	6249649.2
TP 24	413255.9	6249752.7
TP 25	413255.9	6249851.1
TP 26	413255.9	6250064.8
TP 27	413255.9	6250269.8
TP 28	413455.7	6249649.2
TP 29	413626.9	6249727.4
TP 30	413526.9	6250036.8
TP 31	413626.9	6250140.0
TP 32	413526.9	6250348.5
TP 33	413727.4	6249649.2
TP 34	413825.3	6249762.5
TP 35	413838.8	6250020.1
TP 36	413851.9	6250231.3
TP 37	413867.2	6250478.1
TP 38	413734.1	6250475.2

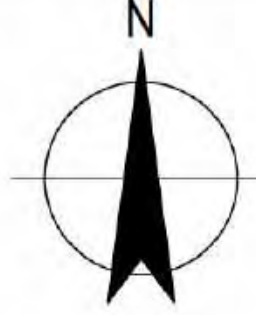
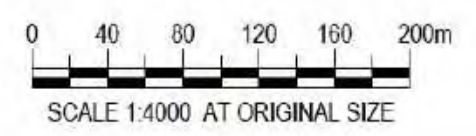
CPT CO-ORDINATES TABLE

POINT ID	EASTING	NORTHING	RL
CPT 01	412860.9	6249923.6	1260.7
CPT 02	413280.2	6249946.5	1251.2
CPT 03	412857.3	6249618.8	1255.8
CPT 04	412920.7	6249986.6	1266.1
CPT 05	412953.3	6249551.5	1250.8
CPT 06	413054.7	6249624.4	1247.7
CPT 07	413132.7	6249627.7	1249.4
CPT 08	413150.0	6249533.8	1248.2
CPT 09	413230.1	6249621.2	1248.9
CPT 10	413326.0	6249684.0	1248.1
CPT 11	413368.6	6249586.4	1247.2
CPT 12	413563.7	6249640.7	1260.7
CPT 13	412728.1	6249668.2	1261.0

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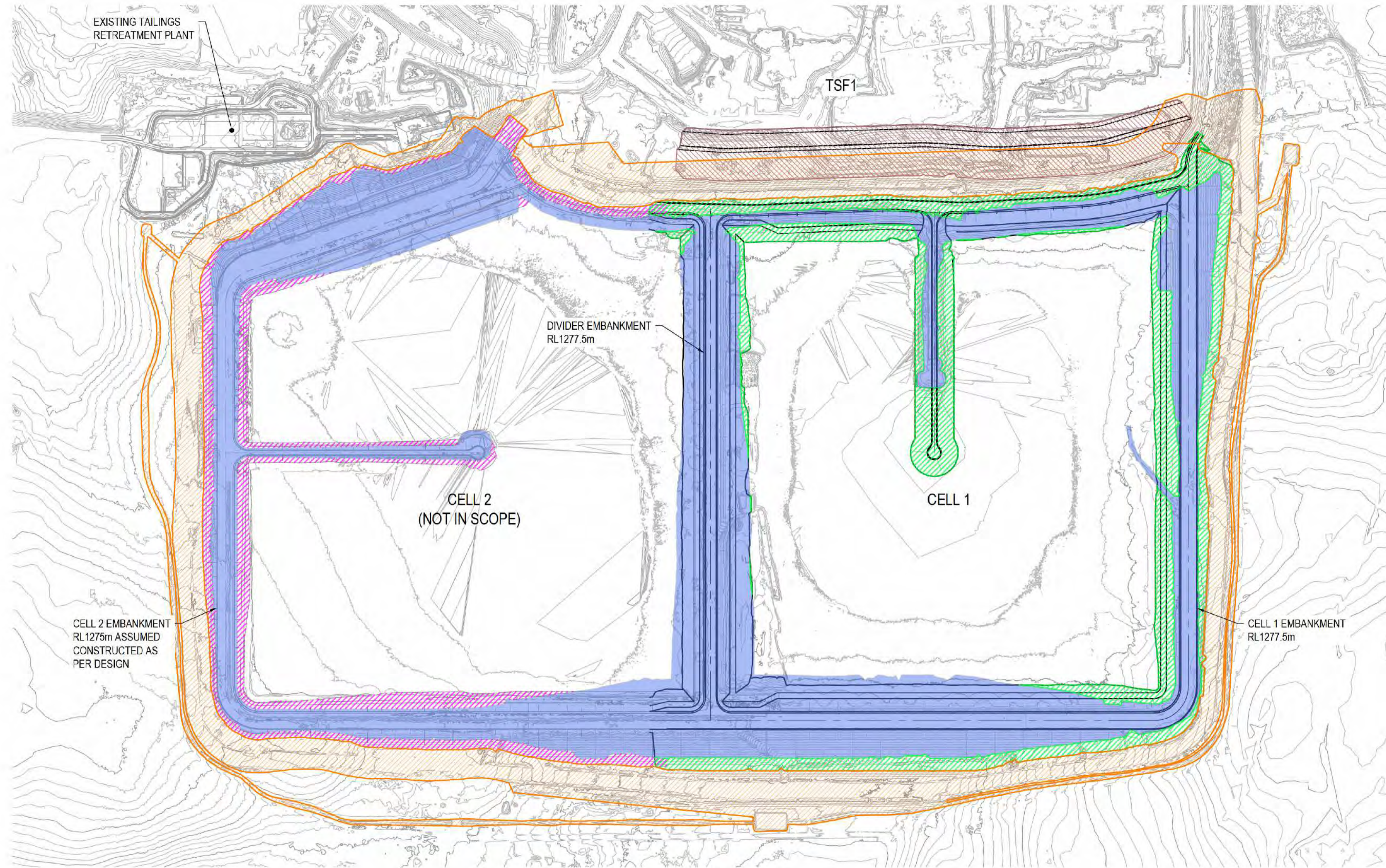
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4. OFF SPECIFICATION MATERIAL TO BE REMOVED PRIOR TO THE CONSTRUCTION OF THE RL1277.5m EMBANKMENT.

LEGEND:

-  RL1270m EMBANKMENT EXTENT (EXISTING)
-  RL1277.5m CELL 1 EMBANKMENT EXTENT
-  MINE WASTE ROCK INFILL
-  RL1275m CELL 2 EMBANKMENT EXTENT (NOT IN SCOPE, FOR INFORMATION ONLY)
-  RL1295m CLEARING EXTENT (NOT IN SCOPE, FOR INFORMATION ONLY)

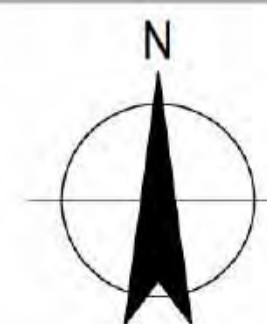
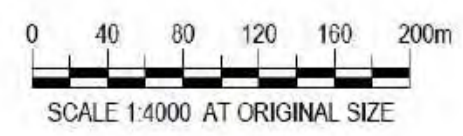


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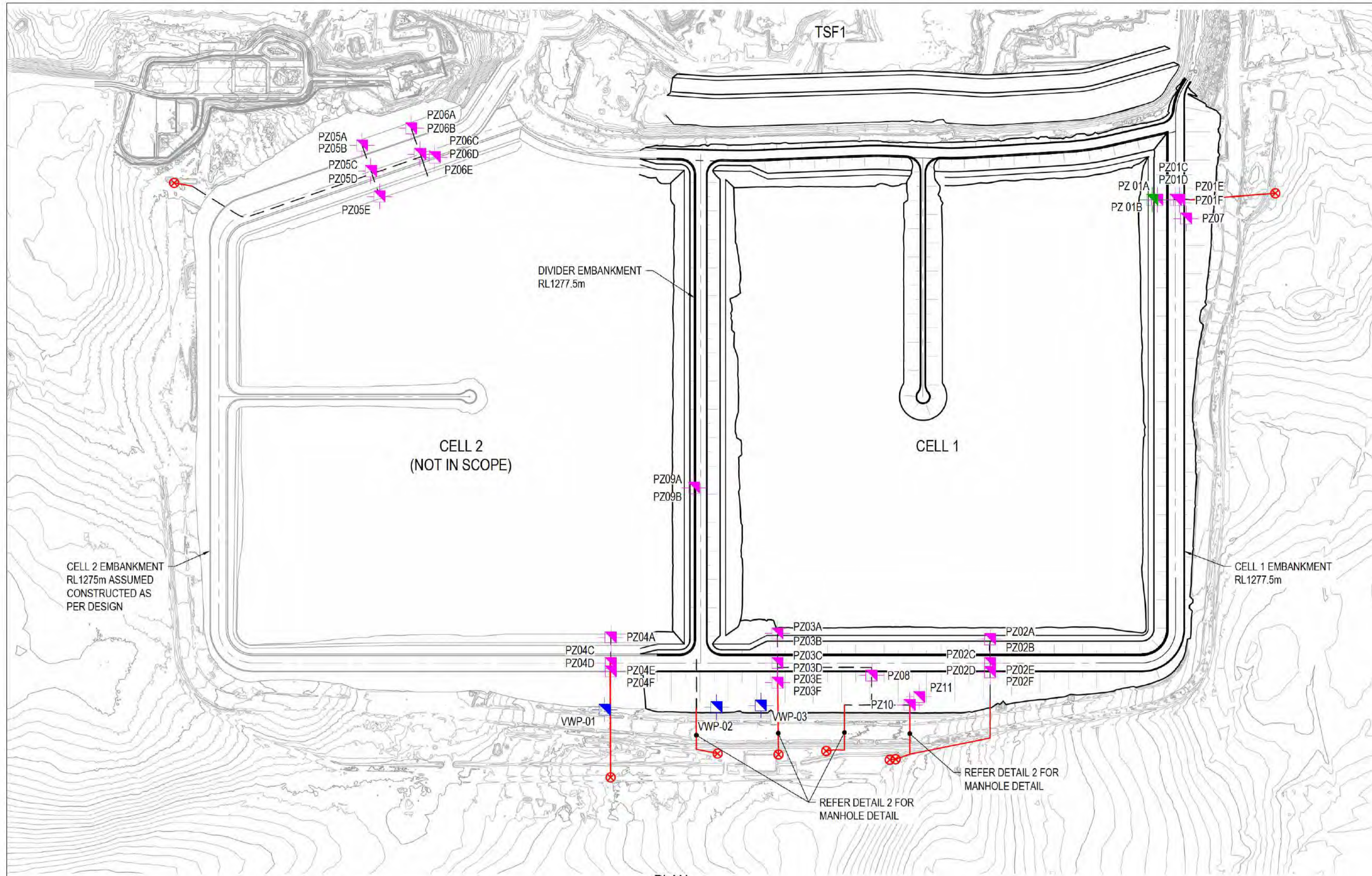
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5. FOR INSTRUMENTATION DETAILS REFER TO DRAWING 00005.

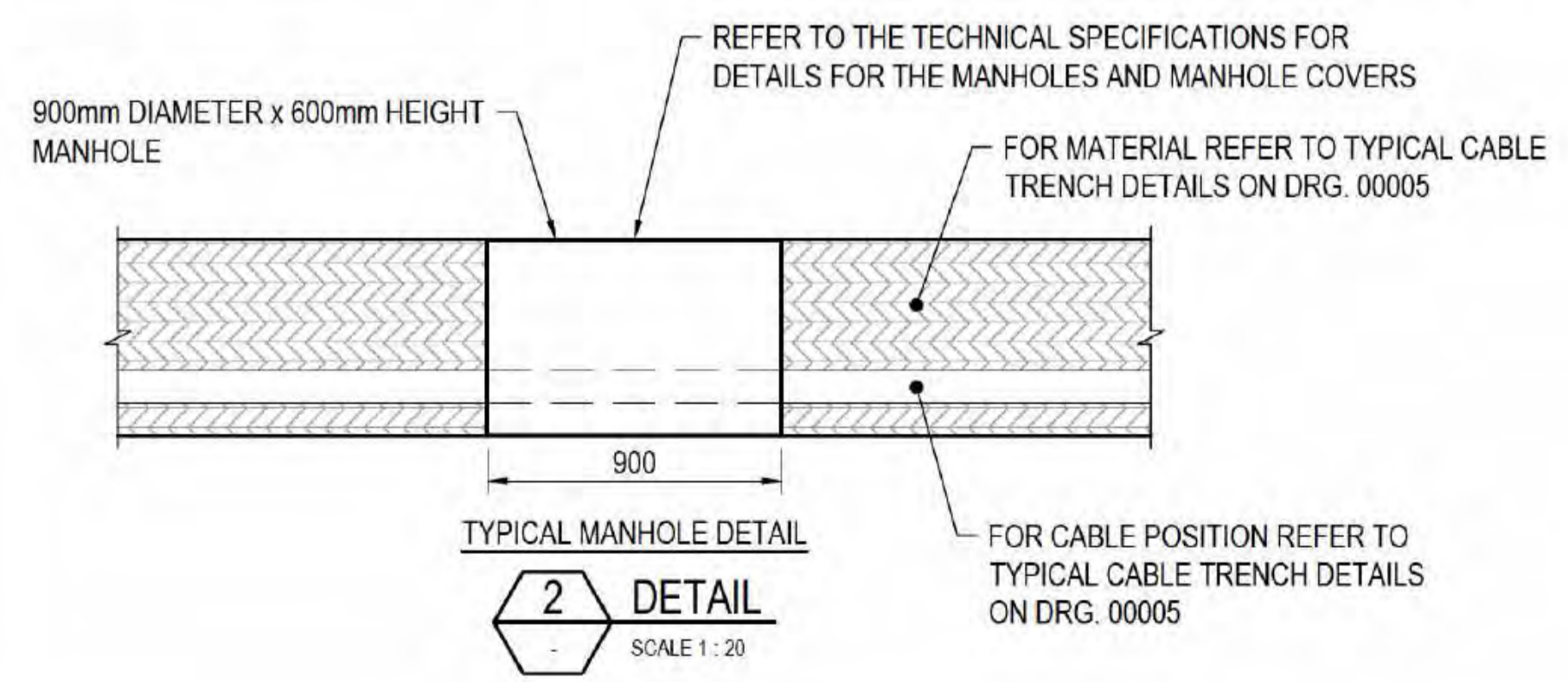
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- EXISTING VWP LOCATIONS
- EXISTING VWP LOCATIONS TO MEASURE PORE PRESSURES DURING CONSTRUCTION
- PROPOSED VWP LOCATION
- EXISTING DATA LOGGER LOCATION
- EXISTING PIEZOMETER CONDUITS
- ROCK LINING
- NATURAL GROUND

PROPOSED PIEZOMETER SETOUT TABLE			
POINT ID	EASTING	NORTHING	TIP RL (m)
PZ 01B	413823.2	6250519.5	1260.3

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POINT ID	EASTING	NORTHING	TIP RL (m)
PZ 01A	413832	6250519.5	1258.8
PZ 01C	413864	6250519.5	1263.2
PZ 01D	413864	6250519.5	1254.6
PZ 01E	413871	6250519.5	1263.4
PZ 01F			1257.7
PZ 02A	413537	6249744.5	1253.9
PZ 02B			1255.9
PZ 02C	413537	6249702.5	1260.3
PZ 02D			1250.3
PZ 02E	413537.1	6249686.5	1260.8
PZ 02F			1253.8
PZ 03A	413162	6249754.5	1249.4
PZ 03B			1251.4
PZ 03C	413161.9	6249702.5	1251.0
PZ 03D			1241.0
PZ 03E	413162.2	6249667.5	1250.7
PZ 03F			1243.7
PZ 04A	412868	6249747.8	1255.5
PZ 04C	412868	6249702.5	1261.2
PZ 04D			1251.2
PZ 04E	412868	6249687.5	1261.1
PZ 04F			1254.1
PZ 05A	412429.4	6250615.5	1256.7
PZ 05B			1246.7
PZ 05C	412445.2	6250507.0	1257.6
PZ 05D			1247.6
PZ 05E	412460.6	6250525.4	1257.1
PZ 06A	412516.2	6250645.6	1255.7
PZ 06B			1245.7
PZ 06C	412532	6250600.1	1258.5
PZ 06D			1248.5
PZ 06E	412558	6250594.5	1254.1
PZ 07	413883	6250486.5	1250.0
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PZ 09A	413015	6250011.5	1249.5
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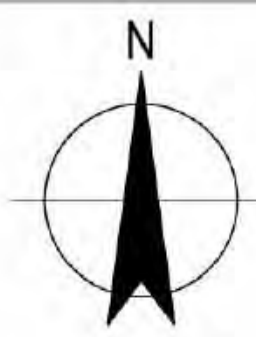
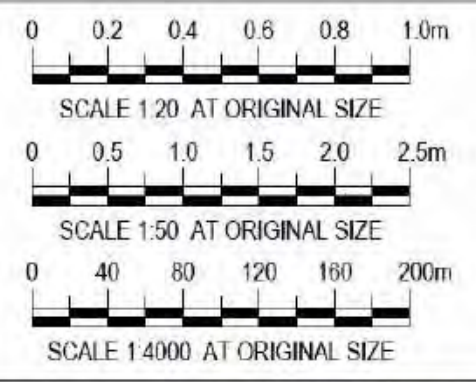
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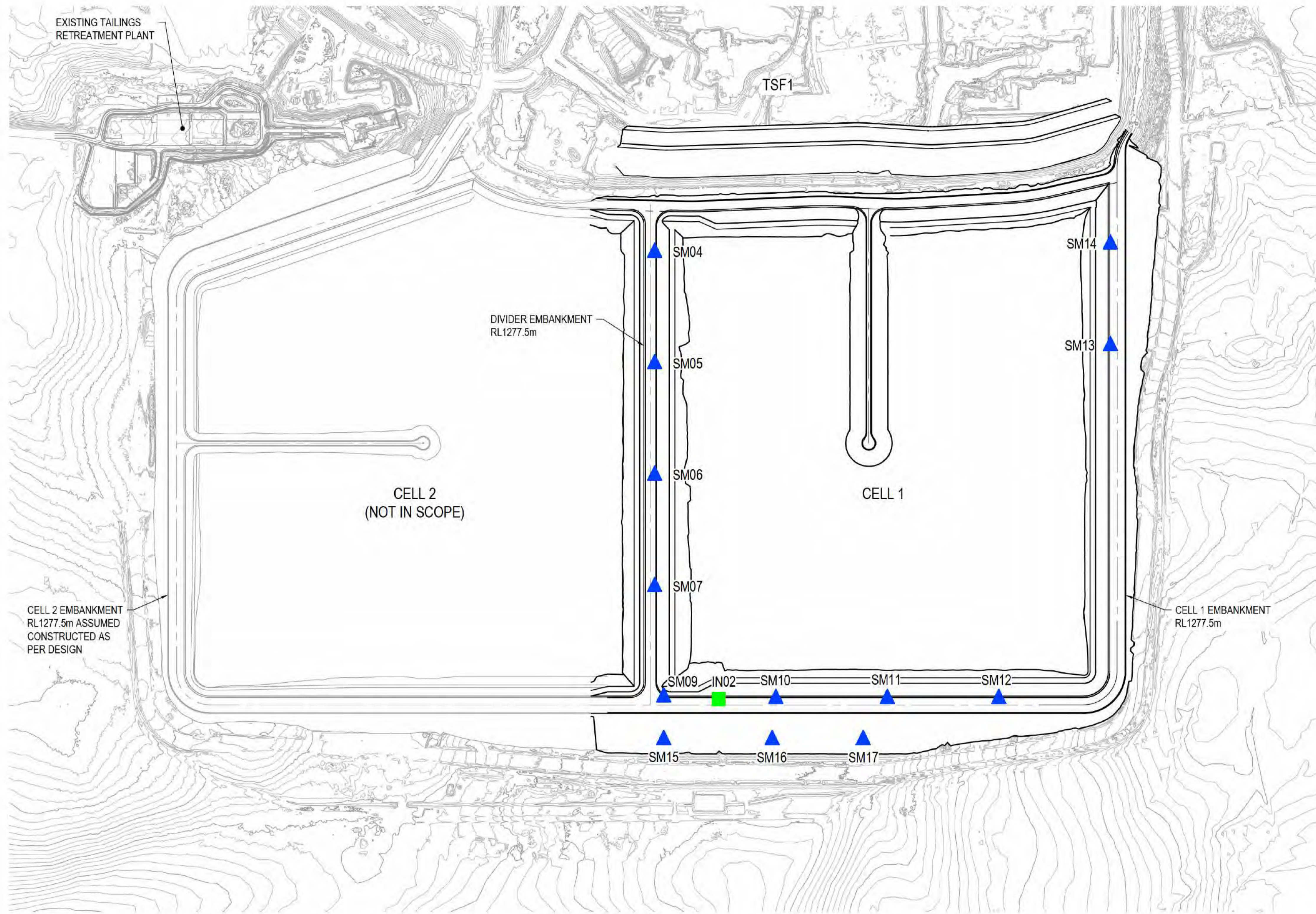
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Drawing No.	
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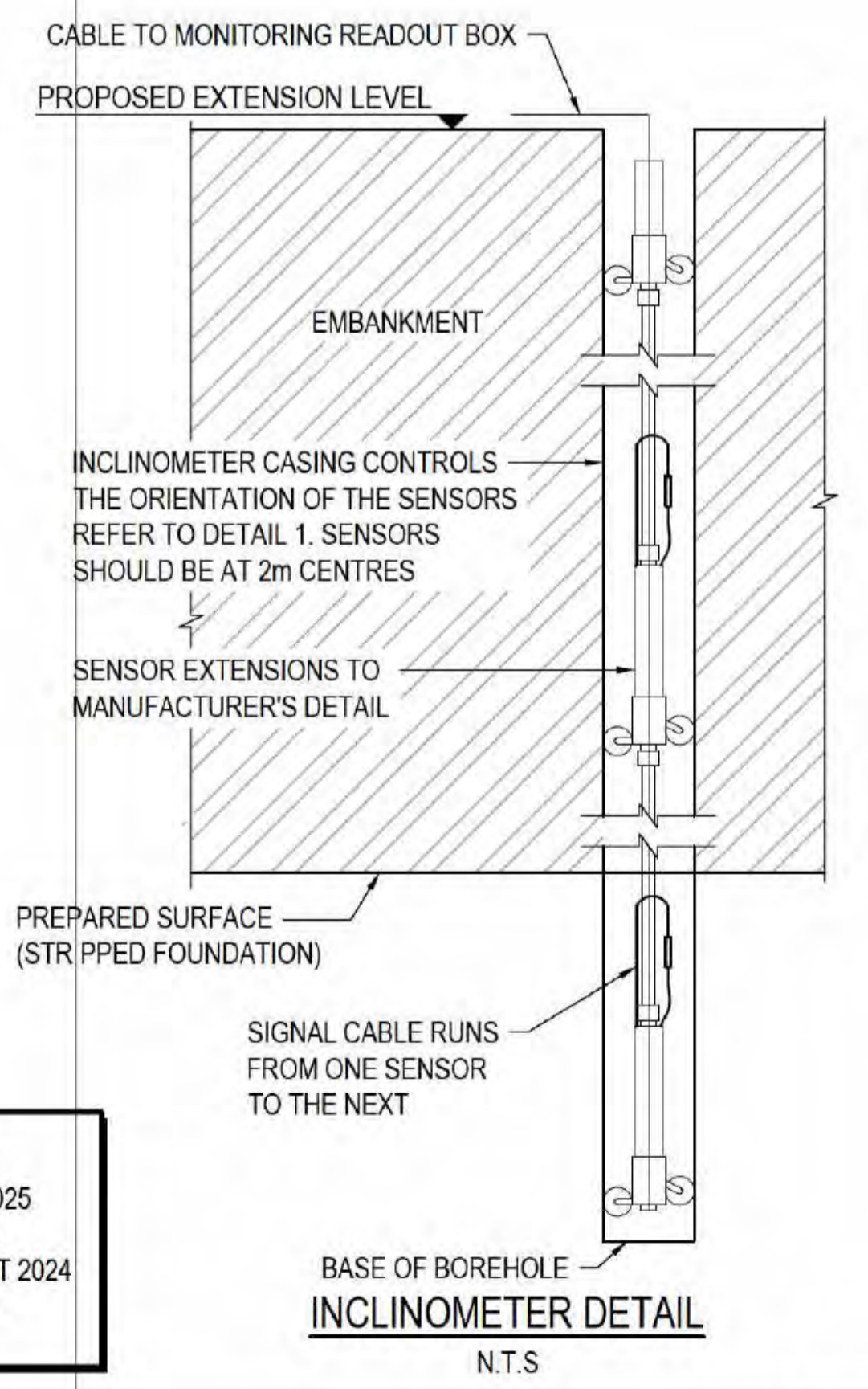
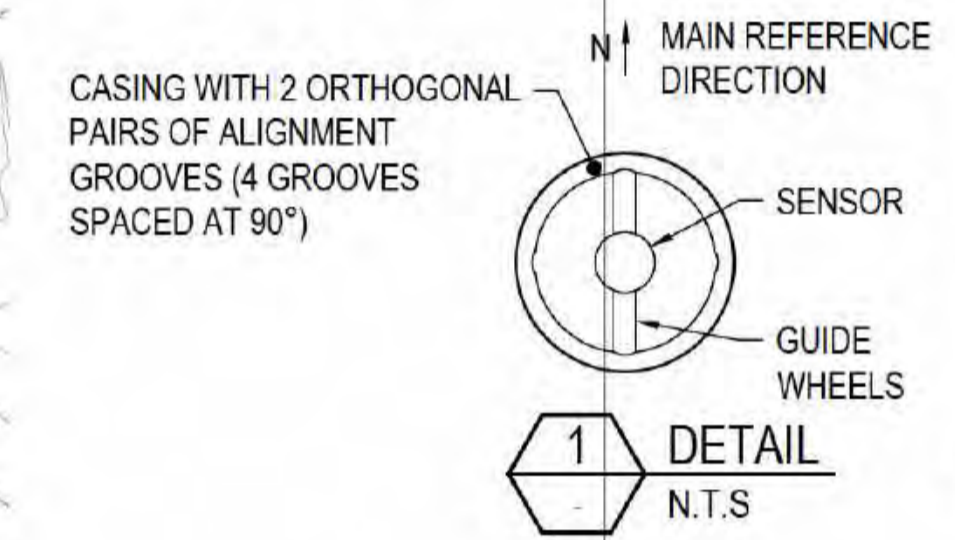
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4. FOR INSTRUMENTATION DETAILS REFER TO STANDARD DRAWING 00005.
5. FOR INCLINOMETER STAGING PLAN REFER TO DRAWING 00006.
6. THE CASING EXTENSION IS TO BE INSTALLED IN ACCORDANCE WITH THE PREVIOUS INSTALLED SET OF ALIGNMENT GROOVES AND SUPERVISED BY AN EXPERIENCED PERSON. FOR MORE DETAILS REFER TO TECHNICAL SPECIFICATION.
7. CELL 2 INSTRUMENTS ARE NOT SHOWN - NOT IN SCOPE.

LEGEND:

- ▲ EXISTING SURVEY MARKERS TO BE RAISED
- EXISTING INCLINOMETER / SETTLEMENT GAUGE TO BE RAISED

EXISTING CELL 1 INCLINOMETER SETOUT TABLE (NOTE 7)		
POINT ID	EASTING	NORTHING
IN02	413149.3	6249712.9

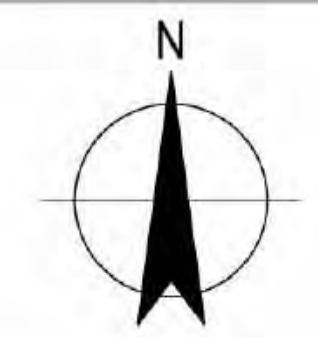
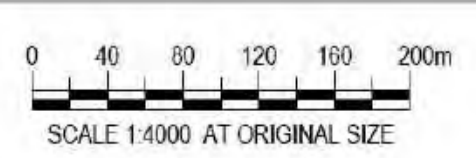
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POINT ID	EASTING	NORTHING
SM04	413034.4	6250615.2
SM05	413034.4	6250315.2
SM06	413034.4	6250115.2
SM07	413034.4	6249915.2
SM09	413050.7	6249717.4
SM10	413251.9	6249714.5
SM11	413451.9	6249714.5
SM12	413651.9	6249714.5
SM13	413852.2	6250347.7
SM14	413852.2	6250530.2
SM15	413050.7	6249840.8
SM16	413245.2	6249840.8
SM17	413408.6	6249840.8



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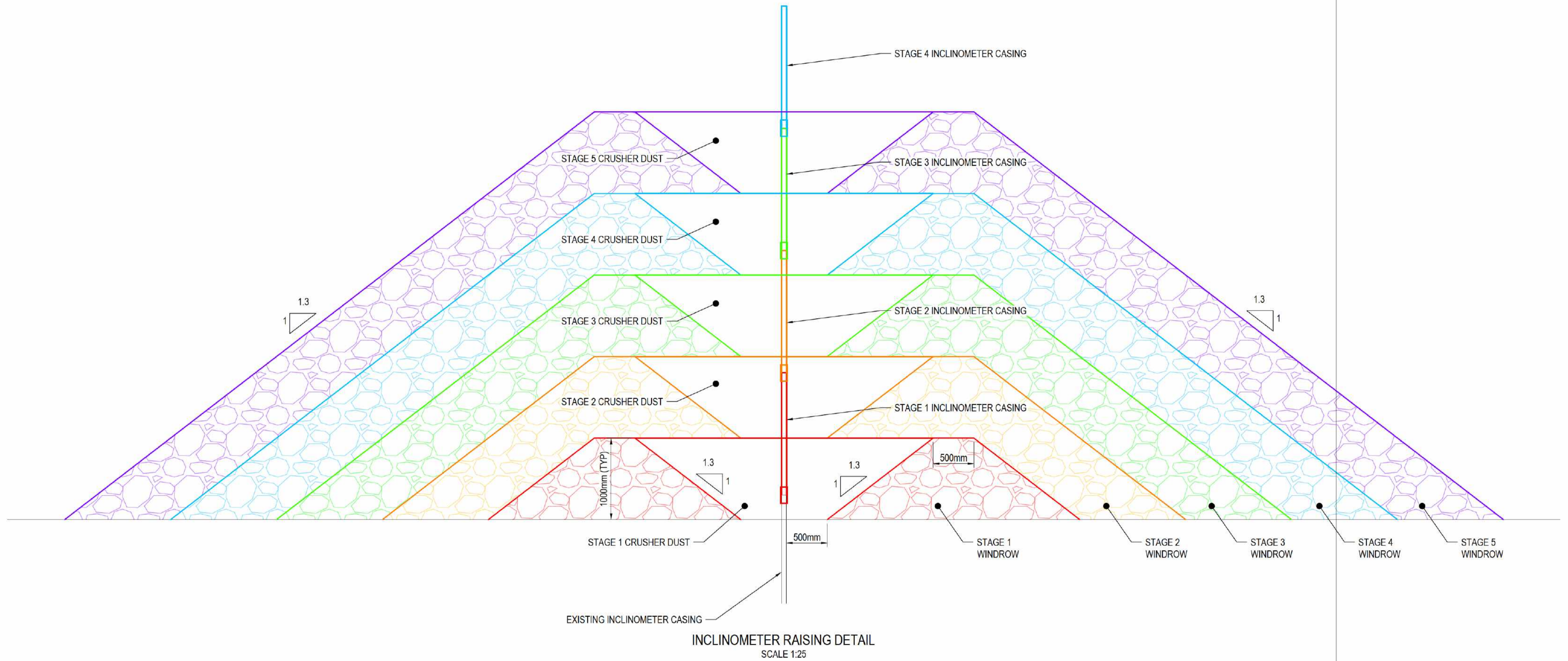


Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
Status FOR CONSTRUCTION

Drawing Title
Drawing No. Rev
Size A1

NOTES:

1. ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
2. ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-00-00-DRG-CI-".
4. REFER DRAWING 00004 FOR INCLINOMETER LOCATIONS.



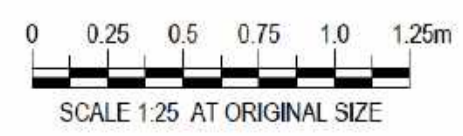
STAGING

- STAGE 1: CONSTRUCT 1M HIGH WINDROW; INSTALL 1.5M LONG INCLINOMETER CASING; GLUE, POP RIVET AND TAPE INCLINOMETER CASING JOINT; MANUAL BACKFILL ANNULUS BETWEEN CASING AND WINDROW WITH CRUSHER DUST; COMPACT CRUSHER DUST WITH HAND COMPACTOR
- STAGE 2: RAISE WINDROW BY 1M; INSTALL 1.5M LONG INCLINOMETER CASING; GLUE, POP RIVET AND TAPE INCLINOMETER CASING JOINT; MANUAL BACKFILL ANNULUS BETWEEN CASING AND WINDROW WITH CRUSHER DUST; COMPACT CRUSHER DUST WITH HAND COMPACTOR
- STAGE 3: RAISE WINDROW BY 1M; INSTALL 1.5M LONG INCLINOMETER CASING; GLUE, POP RIVET AND TAPE INCLINOMETER CASING JOINT; MANUAL BACKFILL ANNULUS BETWEEN CASING AND WINDROW WITH CRUSHER DUST; COMPACT CRUSHER DUST WITH HAND COMPACTOR
- STAGE 4: RAISE WINDROW BY 1M; INSTALL 1.5M LONG INCLINOMETER CASING; GLUE, POP RIVET AND TAPE INCLINOMETER CASING JOINT; MANUAL BACKFILL ANNULUS BETWEEN CASING AND WINDROW WITH CRUSHER DUST; COMPACT CRUSHER DUST WITH HAND COMPACTOR
- STAGE 5: RAISE WINDROW BY 1M; MANUAL BACKFILL ANNULUS BETWEEN CASING AND WINDROW WITH CRUSHER DUST; COMPACT CRUSHER DUST WITH HAND COMPACTOR

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TALISON LITHIUM PTY LTD
RECEIVED ON: TSF4, 21 DECEMBER 2025
TSF1, 5 JANUARY 2026
EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM



001 ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
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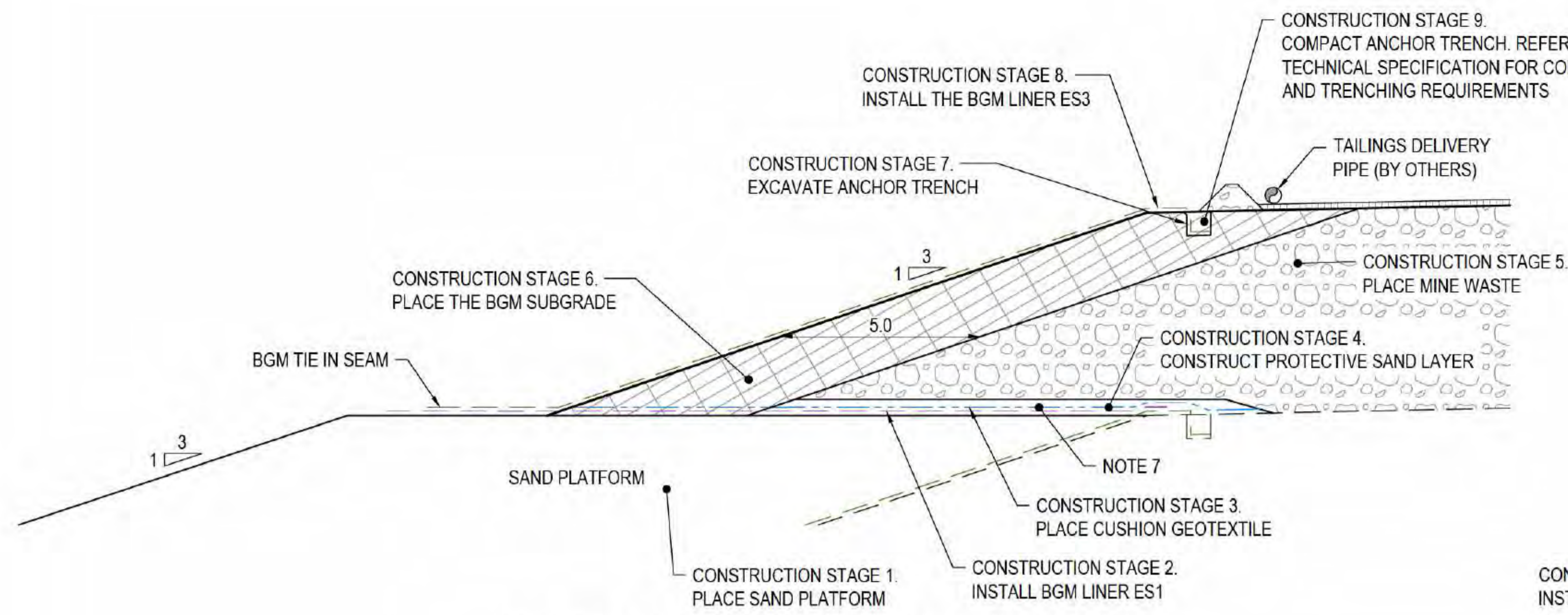
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12685165

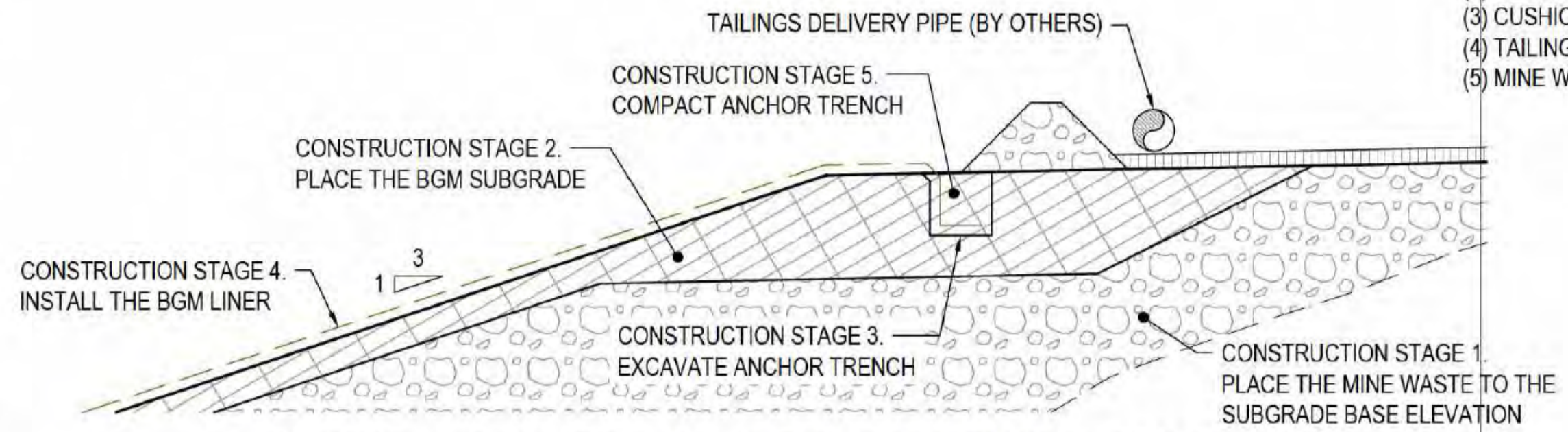
Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
Status FOR CONSTRUCTION

Drawing Title	Size A1
Drawing No.	Rev



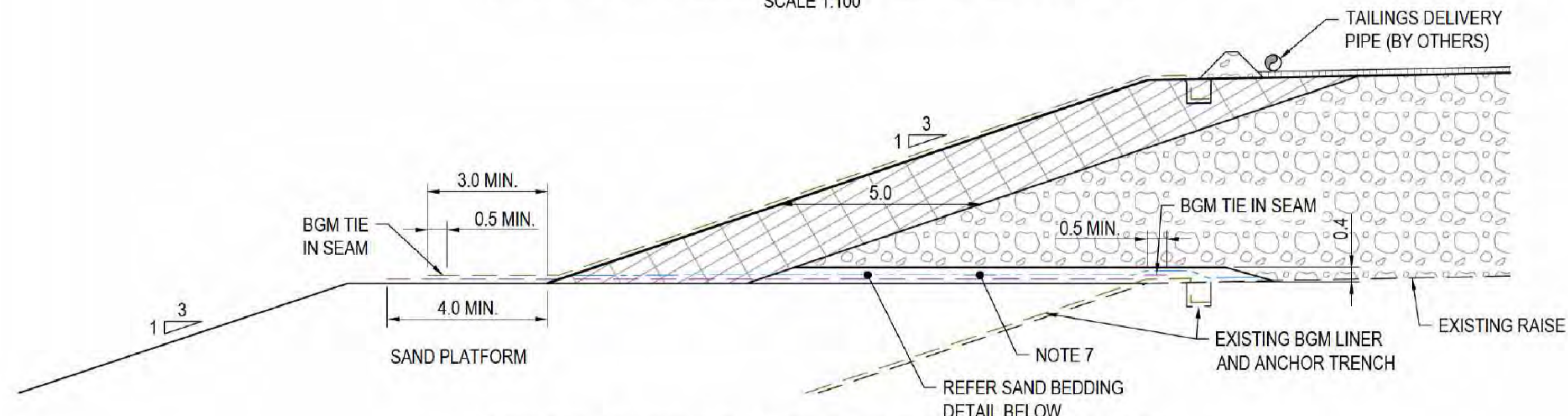
PERIMETER EMBANKMENT - CONSTRUCTION SEQUENCE

SCALE 1:100



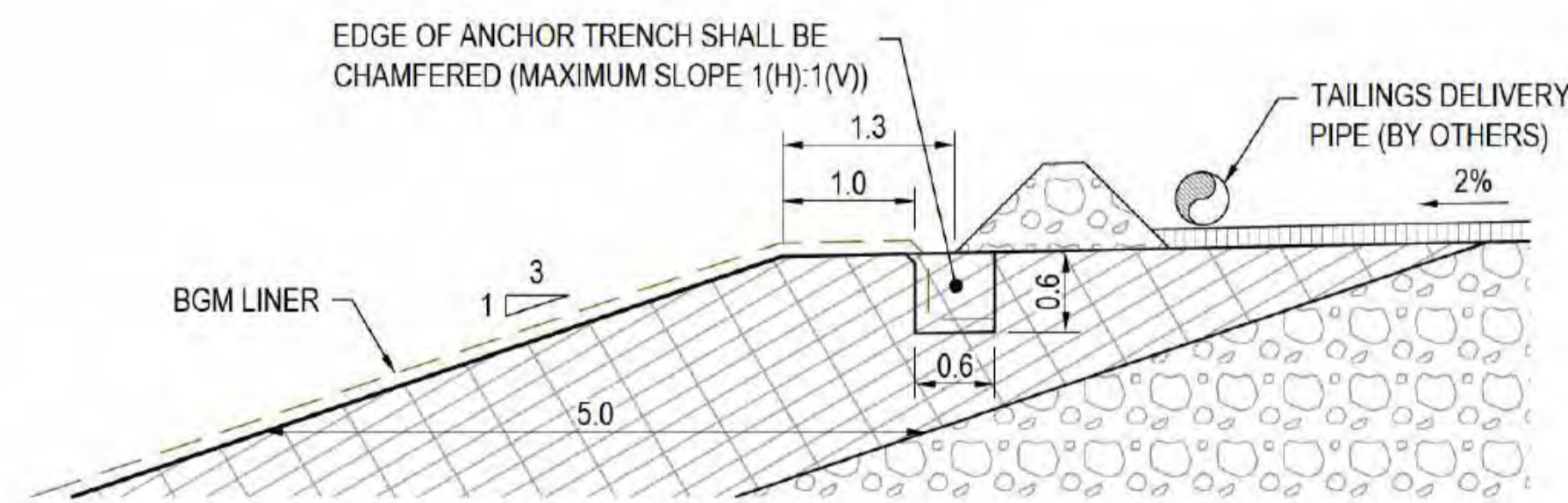
TSF4/TSF1 AND DIVIDER EMBANKMENT - CONSTRUCTION SEQUENCE

SCALE 1:50



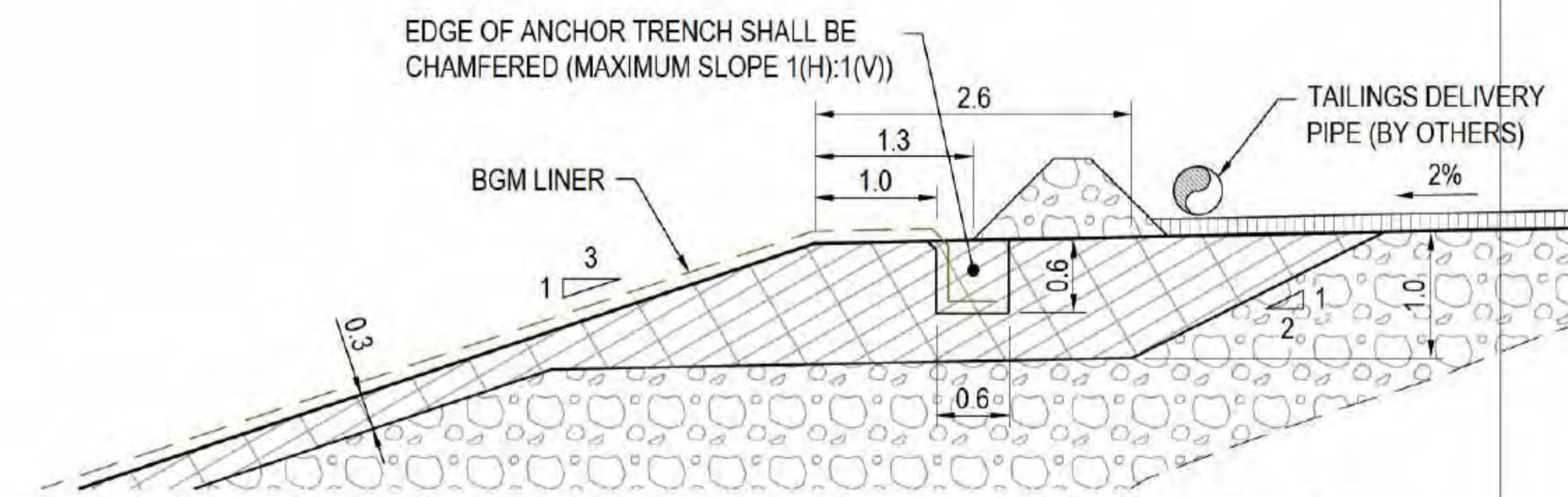
PERIMETER EMBANKMENT - TYING INTO EXISTING BGM LINER

SCALE 1:100



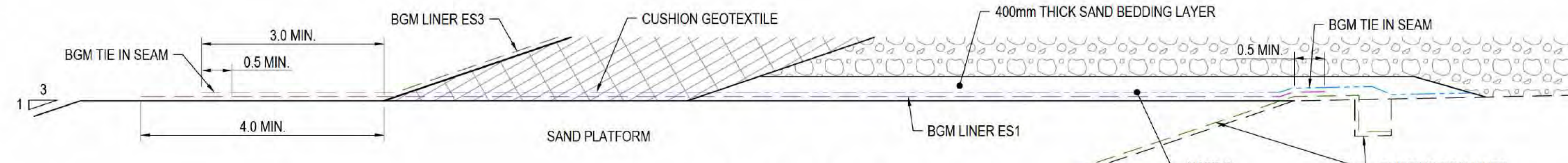
PERIMETER EMBANKMENT - ANCHOR TRENCH DETAIL FOR BGM

SCALE 1:50



TSF4/TSF1 AND DIVIDER EMBANKMENT - ANCHOR TRENCH DETAIL FOR BGM

SCALE 1:50



SAND BEDDING DETAIL

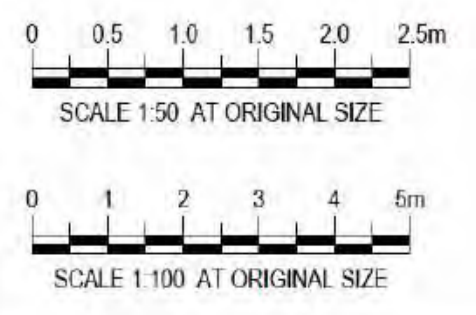
SCALE 1:50

- NOTES:**
1. ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
 2. ALL RL's ARE IN METRES TO LOCAL MINE DATUM (m).
 3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-00-00-DRG-C1".
 4. WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE.
 5. REFER TO TECHNICAL SPECIFICATIONS FOR BGM MATERIAL REQUIREMENTS ES3 TO BE INSTALLED ON EMBANKMENT SLOPES, AND ES1 TO BE INSTALLED ON THE TAILINGS PLATFORM FLOOR.
 6. REFER TO TECHNICAL SPECIFICATIONS FOR ANCHOR TRENCH COMPACTION REQUIREMENTS, BGM MATERIAL REQUIREMENTS AND GEOTEXTILE REQUIREMENTS.
 7. THE ORDER OF THE BGM PROTECTION LAYER IS:
 (1) TAILINGS SAND PLATFORM, OVERLAIN BY
 (2) ES1 BGM, OVERLAIN BY
 (3) CUSHION GEOTEXTILE, OVERLAIN BY
 (4) TAILINGS SAND PROTECTION, OVERLAIN BY
 (5) MINE WASTE ROCK.

- LEGEND:**
- CUSHION GEOTEXTILE
 - BGM LINER ES3
 - BGM LINER ES1
 - MINE WASTE ROCK
 - BGM LINER SUBGRADE
 - WEARING COURSE
 - CLEAN TAILINGS SAND

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001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
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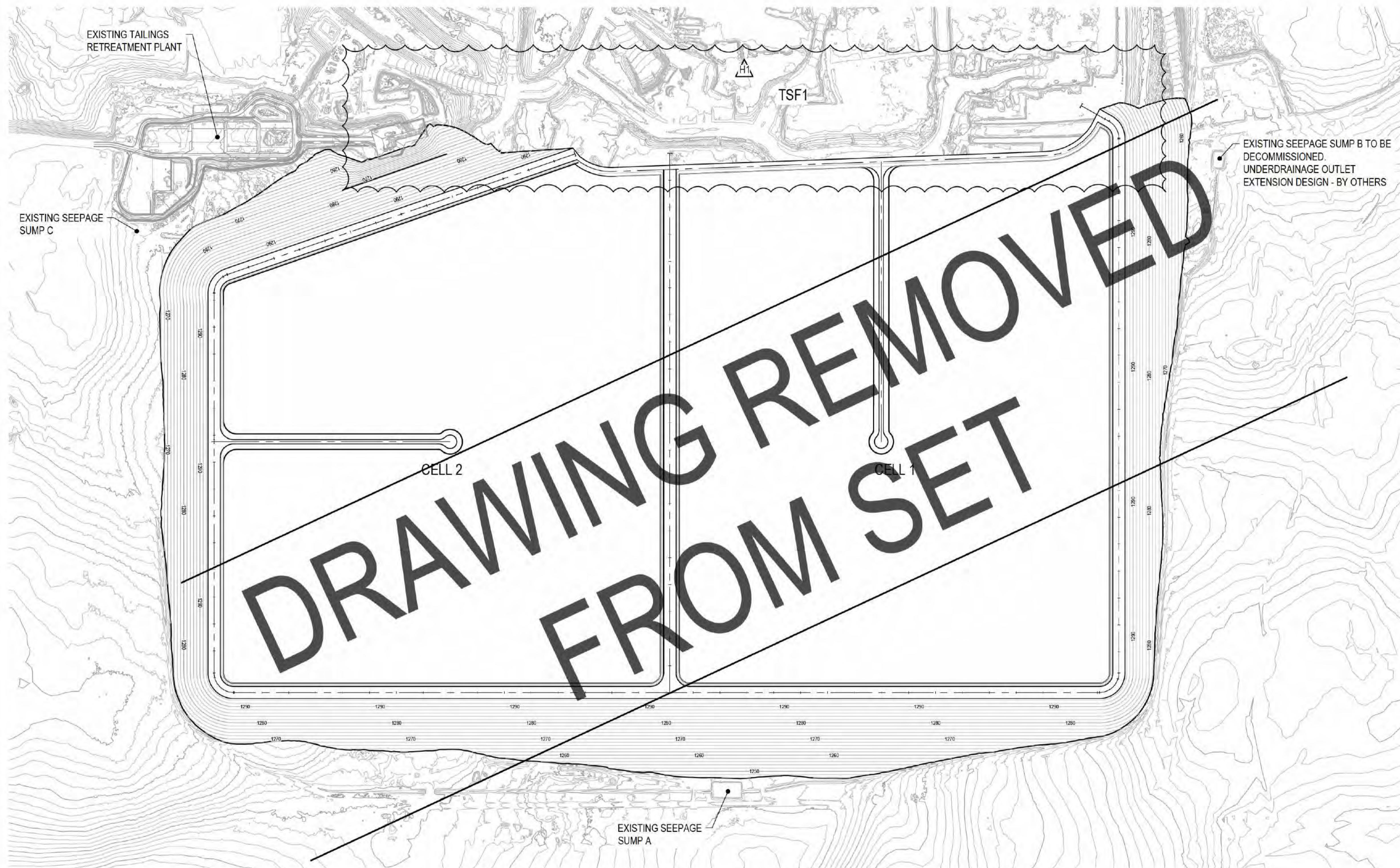
Project No.
12685165

Client TALISON LITHIUM PTY LTD
 Project TSF4 CELL 1 REDESIGN TO RL1277.5m
 Status FOR CONSTRUCTION



Size
A1

Drawing No. Rev



- NOTES:**
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 3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-00-00-DRG-C1".

ITEMS ON HOLD:
 H1 - EARTHWORKS INTERFACE TO BE FINALISED AT A LATER STAGE.

DRAWING REMOVED FROM SET

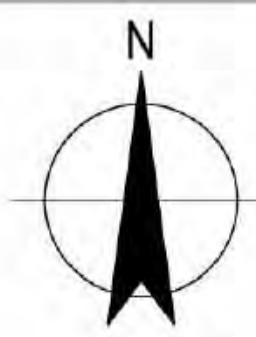
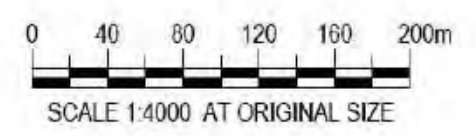
PLAN
SCALE 1:4000

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P02	DRAWING REMOVED FROM SET	SW	DC	23/04/26
P01	ISSUED FOR REVIEW	SW	DC	23/04/26
Rev	Description	Checked	Approved	Date



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Drawing Title

Size A1


Drawing No. Rev

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4. OFF SPECIFICATION MATERIAL TO BE REMOVED PRIOR TO THE CONSTRUCTION OF THE RL1277.5m EMBANKMENT.

LEGEND:

 FINAL CLEARING AREA EXTENT (ALREADY COMPLETED)



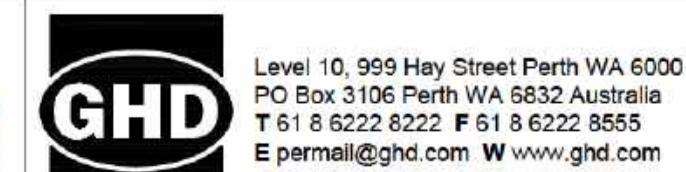
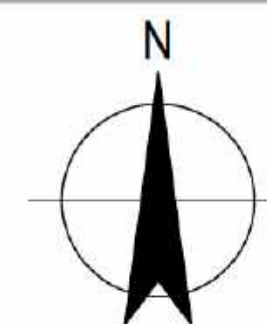
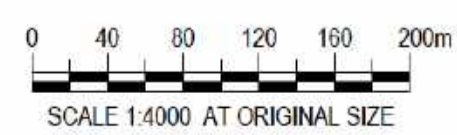
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SCALE 1:4000

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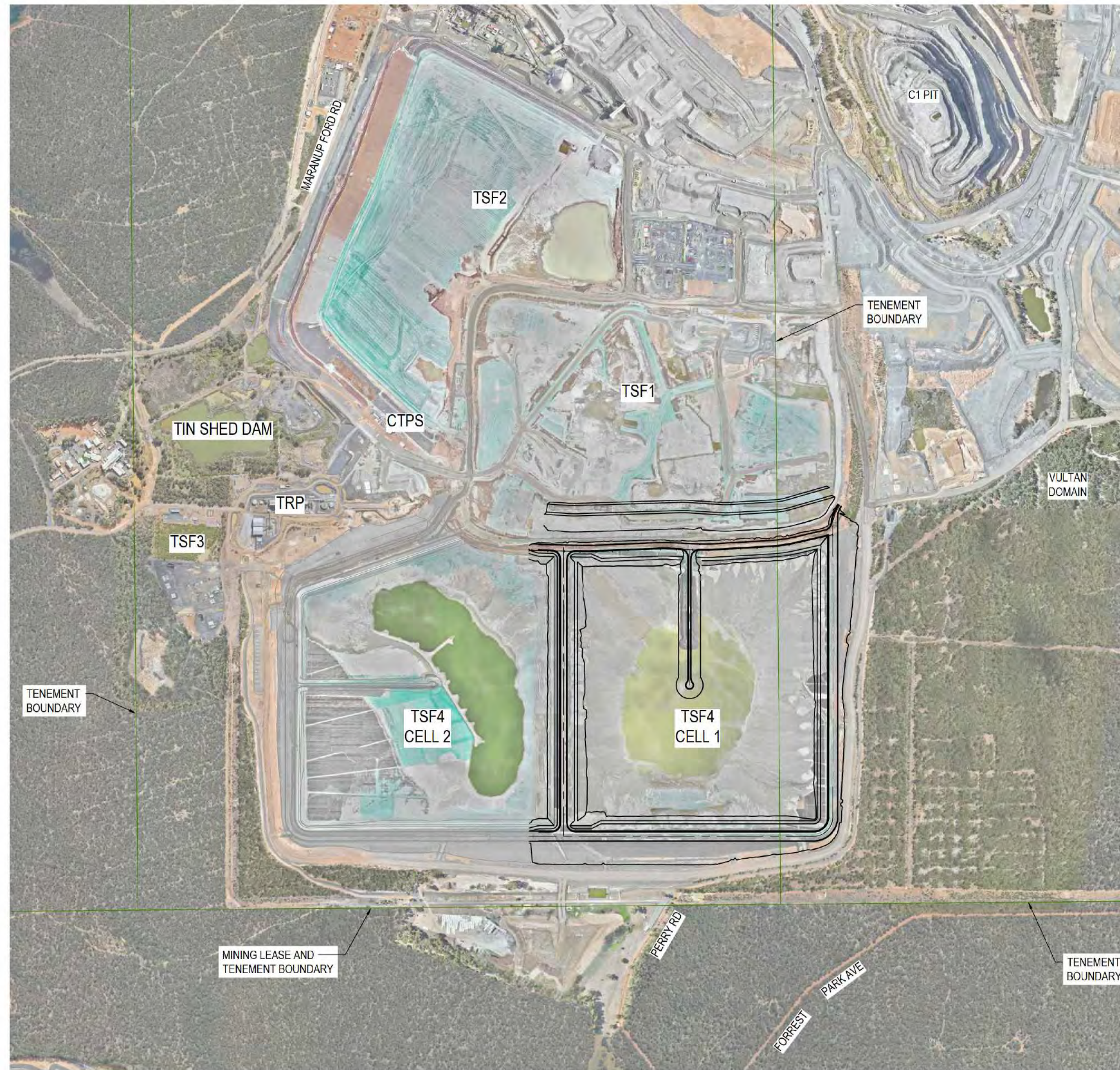
Project No.
12685165

Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
Status FOR CONSTRUCTION

Drawing Title

Size
A1

Drawing No. Rev



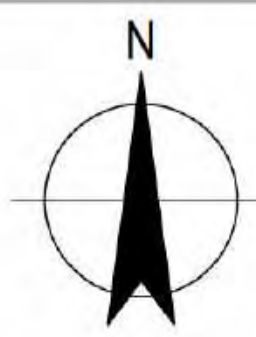
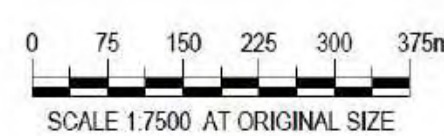
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CELL 1 DRAWINGS	
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12685165-GHD-01-77-DRG-CI-10001	COVER SHEET
12685165-GHD-01-77-DRG-CI-10002	GENERAL ARRANGEMENT
12685165-GHD-01-77-DRG-CI-10003	TYPICAL EMBANKMENT - SECTIONS AND DETAILS - SHEET 1
12685165-GHD-01-77-DRG-CI-10004	TYPICAL EMBANKMENT - SECTIONS AND DETAILS - SHEET 2
12685165-GHD-01-77-DRG-CI-10005	TYPICAL EMBANKMENT - INTERFACE SECTIONS AND DETAILS
12685165-GHD-01-77-DRG-CI-10006	DECANT PLAN AND DETAILS
12685165-GHD-01-77-DRG-CI-10007	BGM LINER - GENERAL ARRANGEMENT
12685165-GHD-01-77-DRG-CI-10008	TAILINGS ELEVATED DRAINS - GENERAL ARRANGEMENT
12685165-GHD-01-77-DRG-CI-10009	TAILINGS ELEVATED DRAINS - DETAILS
12685165-GHD-01-77-DRG-CI-10010	TAILINGS ELEVATED DRAINS - LONGITUDINAL SECTIONS - SHEET 1
12685165-GHD-01-77-DRG-CI-10011	TAILINGS ELEVATED DRAINS - LONGITUDINAL SECTIONS - SHEET 2
12685165-GHD-01-77-DRG-CI-10012	TAILINGS ELEVATED DRAINS - LONGITUDINAL SECTIONS - SHEET 3
12685165-GHD-01-77-DRG-CI-10013	TAILINGS ELEVATED DRAINS - LONGITUDINAL SECTIONS - SHEET 4
STANDARD DRAWINGS	
DRAWING NUMBER	DRAWING TITLE
12685165-GHD-00-00-DRG-CI-00001	GEOTECHNICAL PLAN
12685165-GHD-00-00-DRG-CI-00002	FOUNDATION PREPARATION
12685165-GHD-00-00-DRG-CI-00003	INSTRUMENTATION PLAN - PIEZOMETERS
12685165-GHD-00-00-DRG-CI-00004	INSTRUMENTATION PLAN - SURVEY MARKERS AND INCLINOMETERS
12685165-GHD-00-00-DRG-CI-00005	INSTRUMENTATION DETAILS
12685165-GHD-00-00-DRG-CI-00006	INSTRUMENTATION DETAILS - INCLINOMETER STAGING PLAN
12685165-GHD-00-00-DRG-CI-00007	BGM SECTIONS AND DETAILS
12685165-GHD-00-00-DRG-CI-00008	TSF FINAL HEIGHT - GENERAL ARRANGEMENT (DRAWING REMOVED FROM SET)
12685165-GHD-00-00-DRG-CI-00009	COMPLETED CLEARING PLAN

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Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
Status FOR CONSTRUCTION

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Size A1

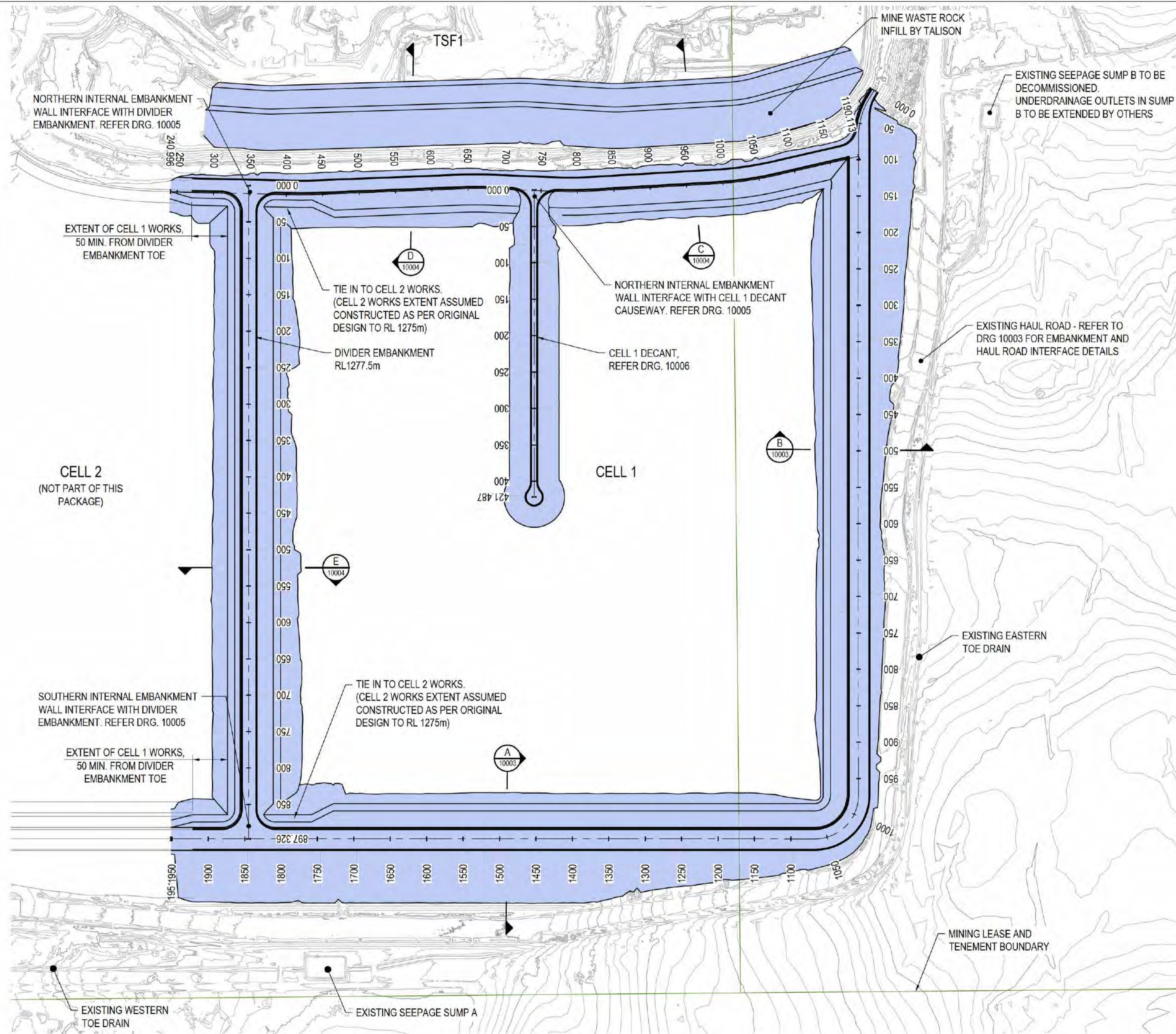
Drawing No. Rev

NOTES:

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3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-CI-".
4. FOR CELL 1 RL1275m RAISE FOUNDATION PREPARATION REFER TO STANDARD DRAWING 12685165-GHD-00-00-DRG-CI-00002.
5. WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE.
6. FOR SETOUT OF CELL 1 EARTHWORKS MODEL REFER TO 12685165-GHD-01-77-MDL-CI-00001.
7. REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.

LEGEND:

- TENEMENT BOUNDARY
- CELL 1 EMBANKMENT EXTENT

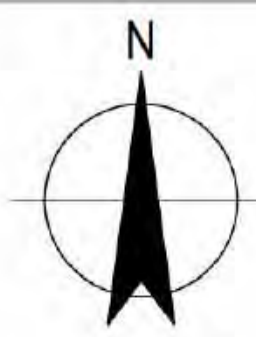
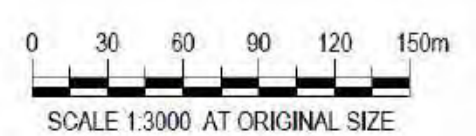


PLAN
SCALE 1:3000

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TALISON LITHIUM PTY LTD
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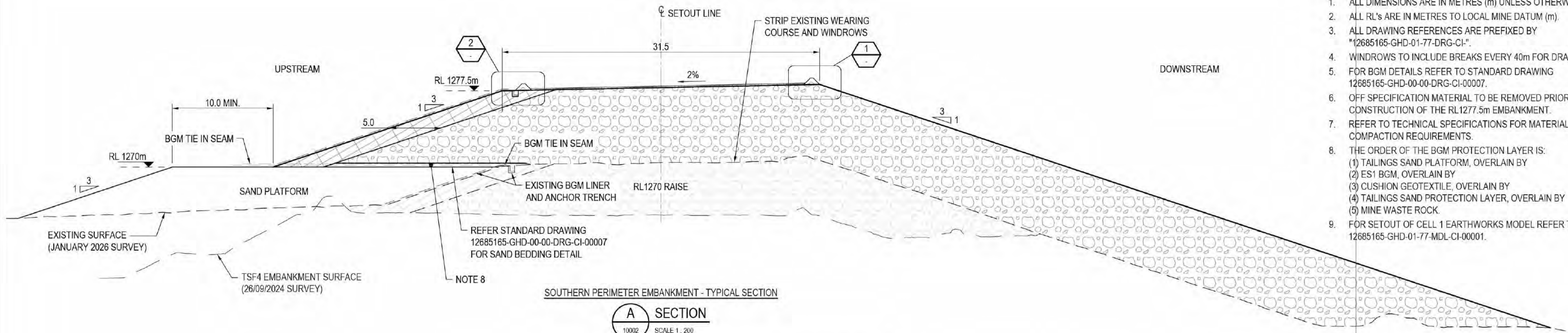


Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
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Drawing Title

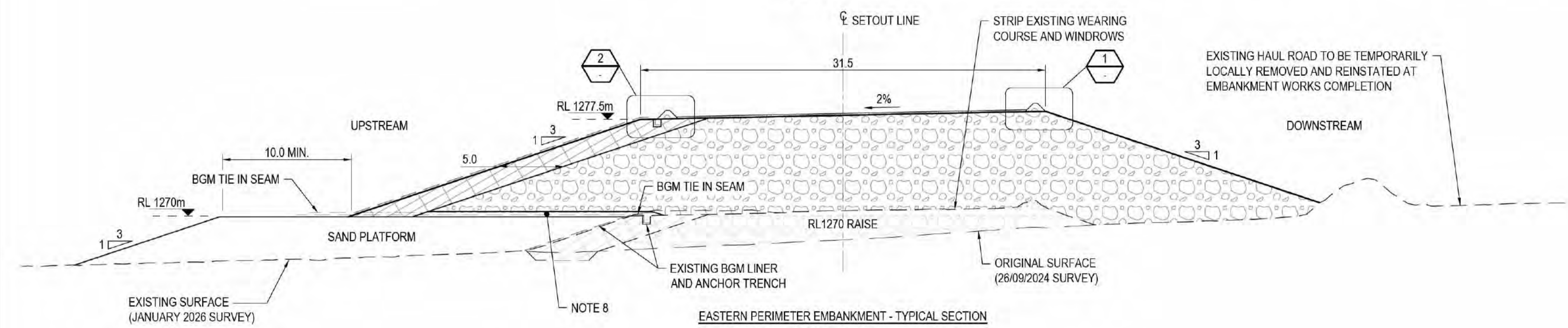
Size A1

Drawing No. Rev



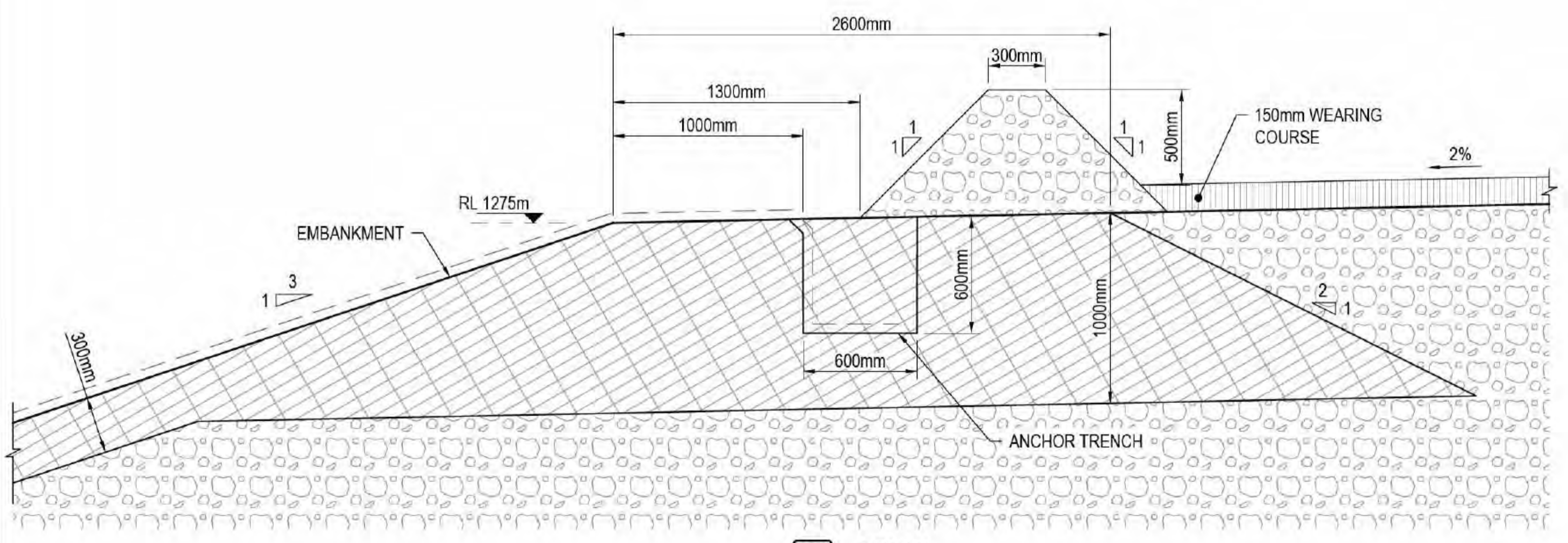
SOUTHERN PERIMETER EMBANKMENT - TYPICAL SECTION

A SECTION
10002 SCALE 1 : 200

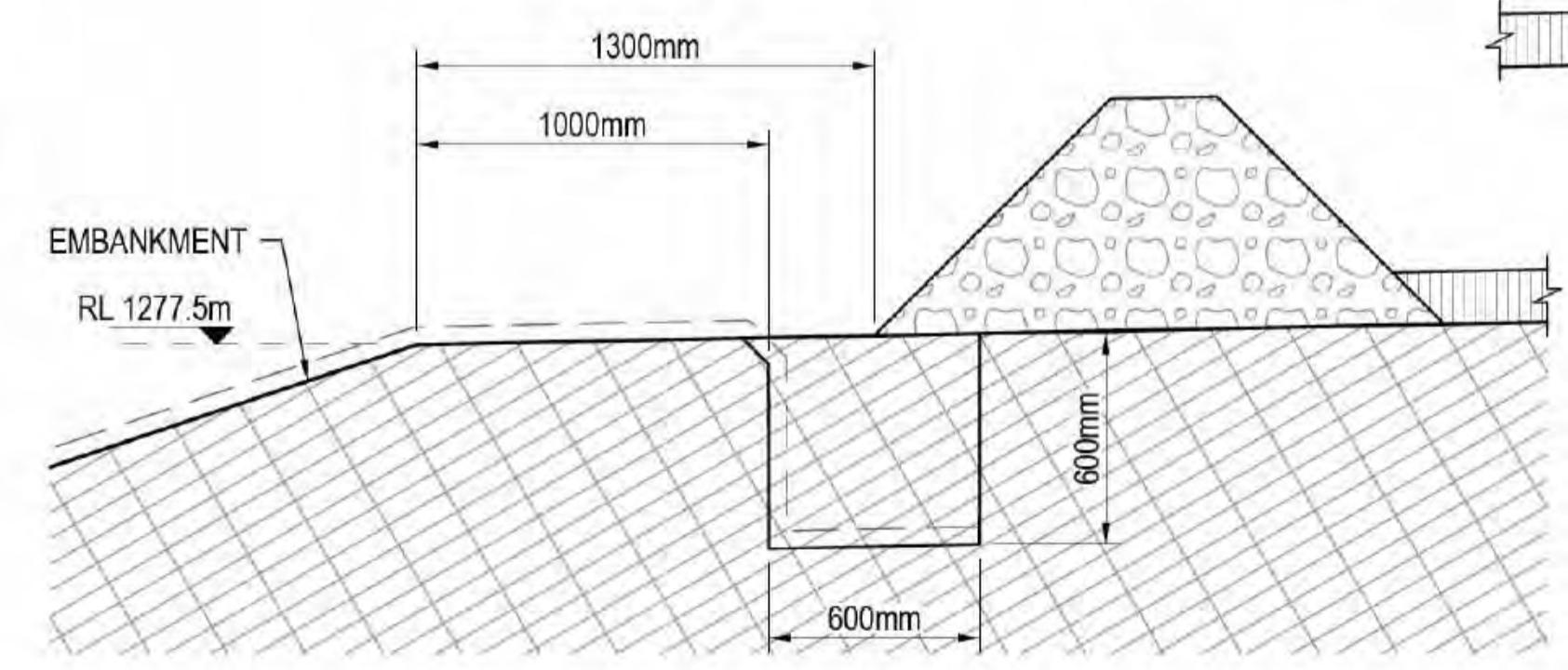


EASTERN PERIMETER EMBANKMENT - TYPICAL SECTION

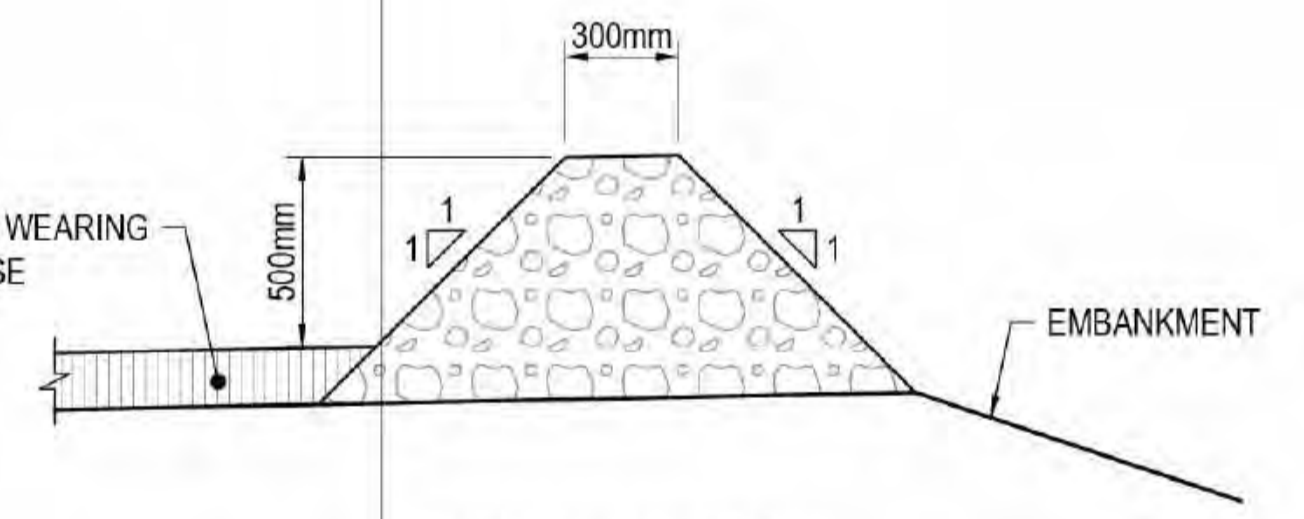
B SECTION
10002 SCALE 1 : 200



3 DETAIL
10004 SCALE 1 : 20



2 DETAIL
SCALE 1 : 20



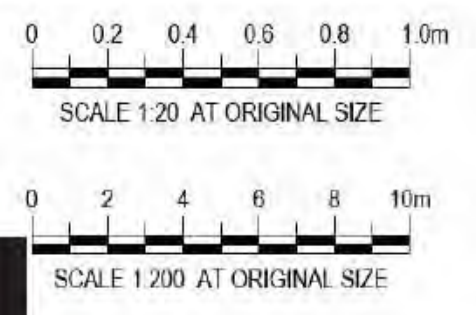
1 DETAIL
SCALE 1 : 20

- LEGEND:**
- BGM LINER (ES3)
 - BGM LINER (ES1)
 - CUSHION GEOTEXTILE
 - EXISTING GEOTEXTILE
 - [Pattern] MINE WASTE ROCK
 - [Pattern] BGM LINER SUBGRADE
 - [Pattern] WEARING COURSE
 - [Pattern] CLEAN TAILINGS SAND

- NOTES:**
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 4. WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE
 5. FOR BGM DETAILS REFER TO STANDARD DRAWING 12685165-GHD-00-00-DRG-CI-00007.
 6. OFF SPECIFICATION MATERIAL TO BE REMOVED PRIOR TO THE CONSTRUCTION OF THE RL1277.5m EMBANKMENT.
 7. REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.
 8. THE ORDER OF THE BGM PROTECTION LAYER IS:
(1) TAILINGS SAND PLATFORM, OVERLAIN BY
(2) ES1 BGM, OVERLAIN BY
(3) CUSHION GEOTEXTILE, OVERLAIN BY
(4) TAILINGS SAND PROTECTION LAYER, OVERLAIN BY
(5) MINE WASTE ROCK.
 9. FOR SETOUT OF CELL 1 EARTHWORKS MODEL REFER TO 12685165-GHD-01-77-MDL-CI-00001.

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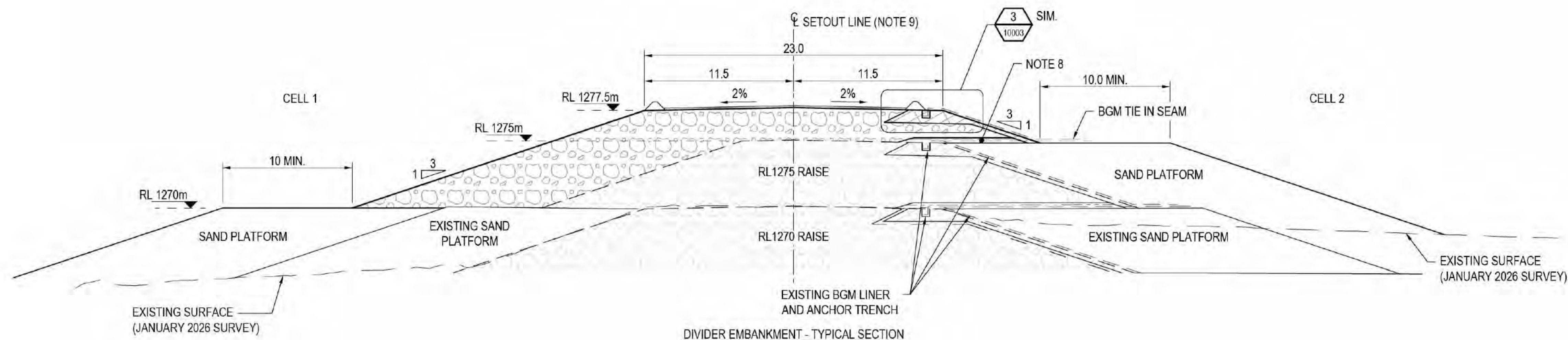


Project No.
12685165

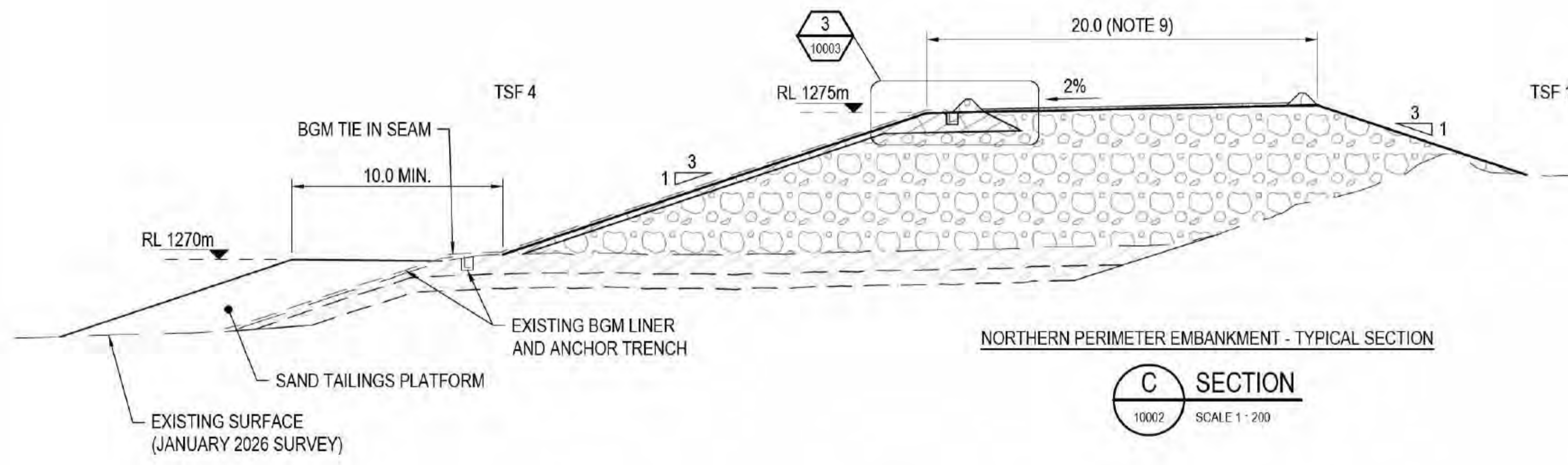
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Project TSF4 CELL 1 REDESIGN TO RL1277.5m
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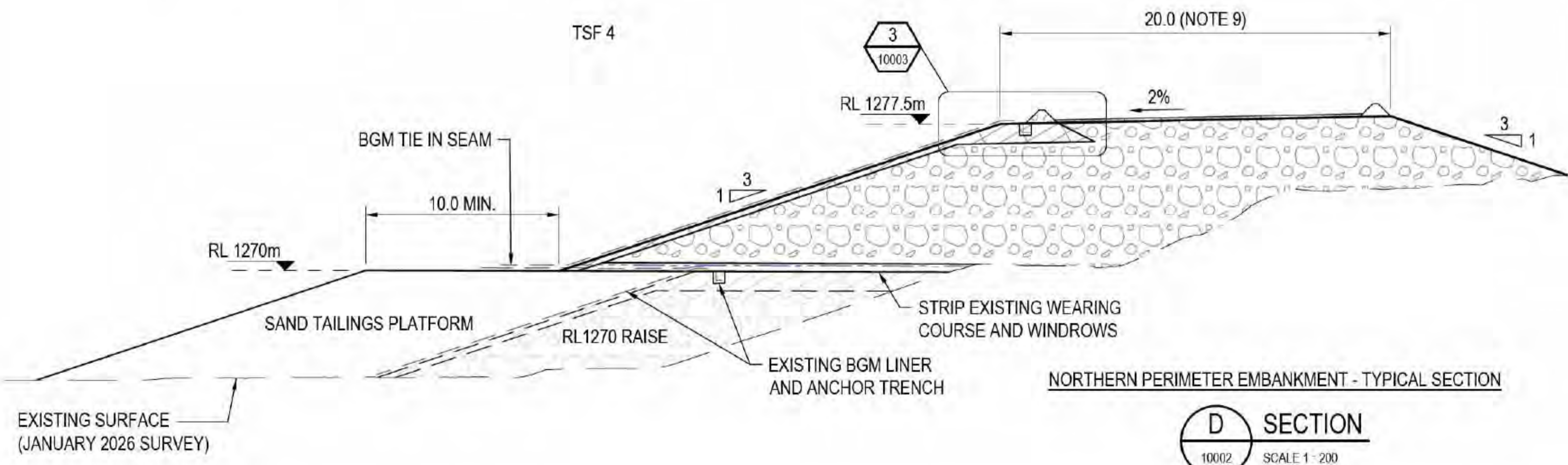
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Drawing No.	Rev



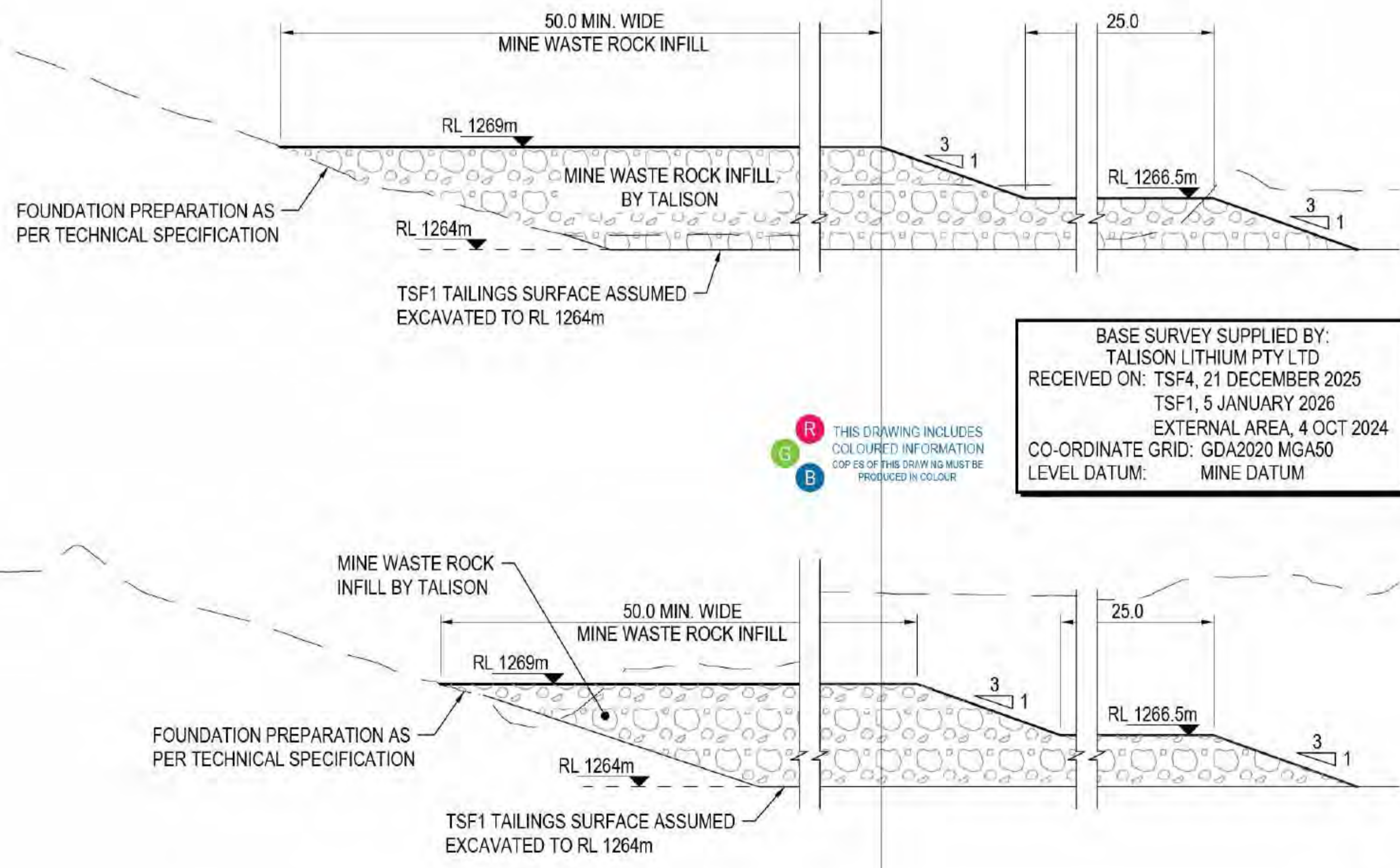
E SECTION
10002 SCALE 1:200



C SECTION
10002 SCALE 1:200



D SECTION
10002 SCALE 1:200

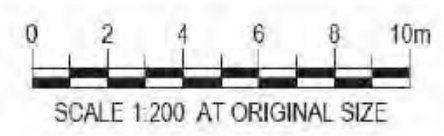


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 - WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE.
 - FOR BGM DETAILS REFER TO STANDARD DRAWING 12685165-GHD-00-00-DRG-CI-00007.
 - OFF SPECIFICATION MATERIAL TO BE REMOVED PRIOR TO THE CONSTRUCTION OF THE RL 1277.5m EMBANKMENT.
 - REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.
 - THE ORDER OF THE BGM PROTECTION LAYER IS:
(1) TAILINGS SAND PLATFORM, OVERLAIN BY
(2) ES1 BGM, OVERLAIN BY
(3) CUSHION GEOTEXTILE, OVERLAIN BY
(4) TAILINGS SAND PROTECTION LAYER, OVERLAIN BY
(5) MINE WASTE ROCK.
 - FOR SETOUT OF CELL 1 EARTHWORKS MODEL REFER TO 12685165-GHD-01-77-MDL-CI-00001.

- LEGEND:**
- BGM LINER (ES3)
 - BGM LINER (ES1)
 - CUSHION GEOTEXTILE
 - EXISTING GEOTEXTILE
 - MINE WASTE ROCK
 - BGM LINER SUBGRADE
 - WEARING COURSE
 - CLEAN TAILINGS SAND

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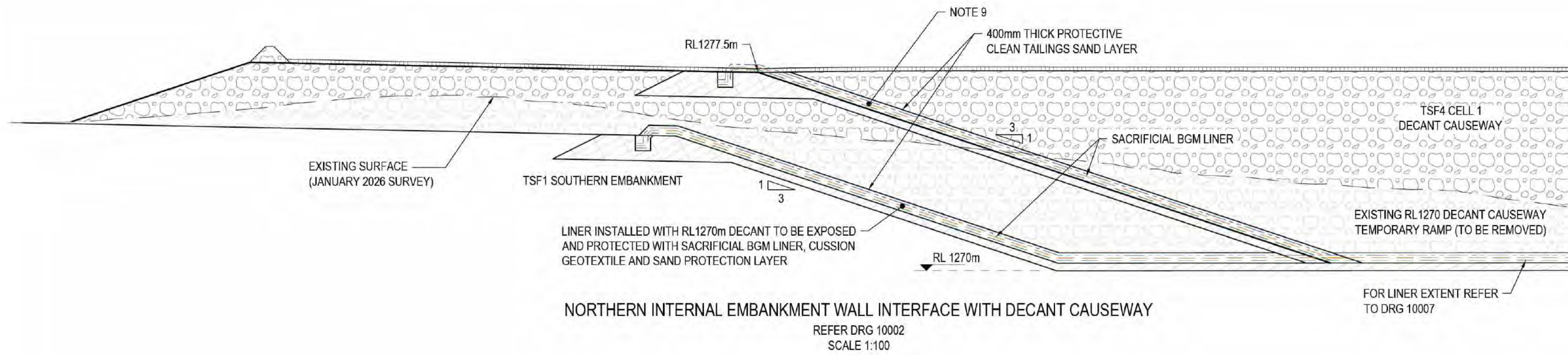
Project No.
12685165

Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO
RL1277.5m
Status FOR CONSTRUCTION

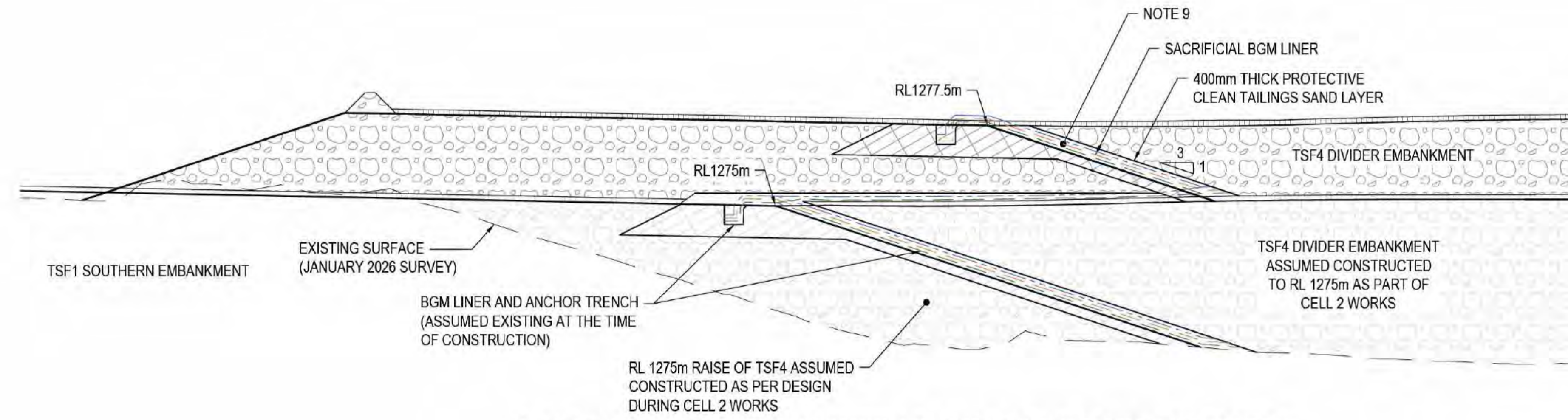
Drawing
Title

Size
A1

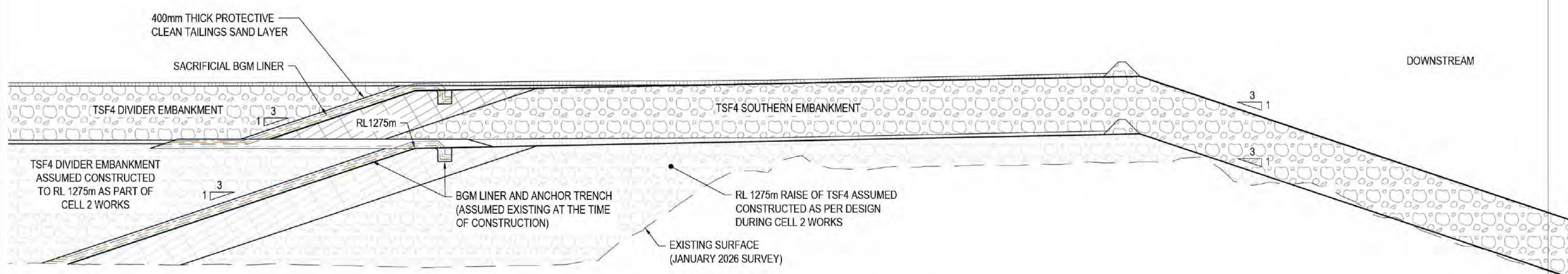
Rev	Description	Checked	Approved	Date
001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26



NORTHERN INTERNAL EMBANKMENT WALL INTERFACE WITH DECANT CAUSEWAY
REFER DRG 10002
SCALE 1:100



NORTHERN INTERNAL EMBANKMENT WALL INTERFACE WITH DIVIDER EMBANKMENT
REFER DRG 10002
SCALE 1:100

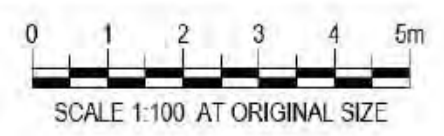


SOUTHERN INTERNAL EMBANKMENT WALL INTERFACE WITH DIVIDER EMBANKMENT
REFER DRG 10002
SCALE 1:100

- NOTES:**
1. ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
 2. ALL RL's ARE IN METRES TO LOCAL MINE DATUM (m).
 3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-00-00-DRG-CI-".
 4. WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE
 5. FOR BGM DETAILS REFER TO STANDARD DRAWING 12685165-GHD-00-00-DRG-CI-00007.
 6. OFF SPECIFICATION MATERIAL TO BE REMOVED PRIOR TO THE CONSTRUCTION OF THE RL1277.5m EMBANKMENT.
 7. REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.
 8. THE ORDER OF THE BGM PROTECTION LAYER IS:
(1) TAILINGS SAND PLATFORM, OVERLAIN BY
(2) ES1 BGM, OVERLAIN BY
(3) CUSHION GEOTEXTILE, OVERLAIN BY
(4) TAILINGS SAND PROTECTION LAYER, OVERLAIN BY
(5) MINE WASTE ROCK.
 9. THE ORDER OF THE BGM PROTECTION LAYER IS:
(1) BGM SUBGRADE, OVERLAIN BY
(2) ES3 BGM, OVERLAIN BY
(3) SACRIFICIAL BGM, OVERLAIN BY
(4) CUSHION GEOTEXTILE, OVERLAIN BY
(5) TAILINGS SAND PROTECTION LAYER, OVERLAIN BY
(6) MINE WASTE ROCK.

- LEGEND:**
- BGM LINER (ES3)
 - SACRIFICIAL BGM LINER
 - CUSHION GEOTEXTILE
 - EXISTING GEOTEXTILE
 - MINE WASTE ROCK
 - BGM LINER SUBGRADE
 - WEARING COURSE
 - CLEAN TAILINGS SAND

Rev	Description	Checked	Approved	Date
001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26



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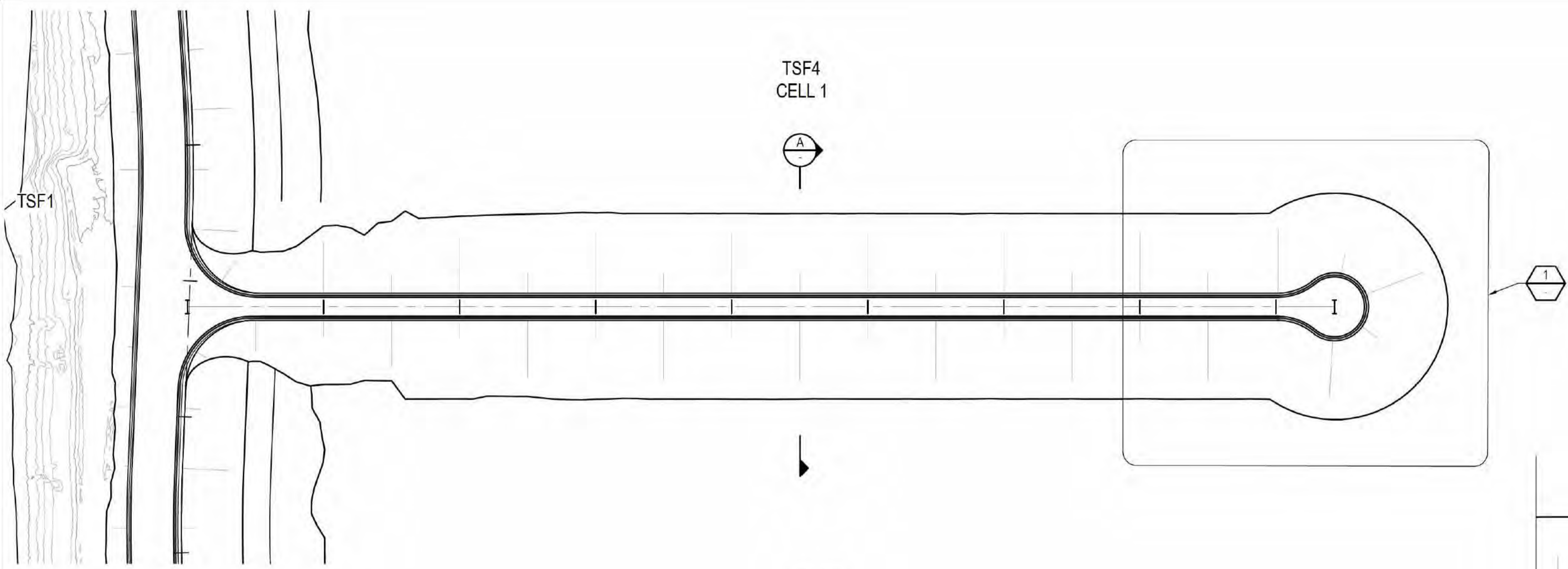


Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO RL1277.5m
Status FOR CONSTRUCTION



BASE SURVEY SUPPLIED BY:
TALISON LITHIUM PTY LTD
RECEIVED ON: TSF4, 21 DECEMBER 2025
TSF1, 5 JANUARY 2026
EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM

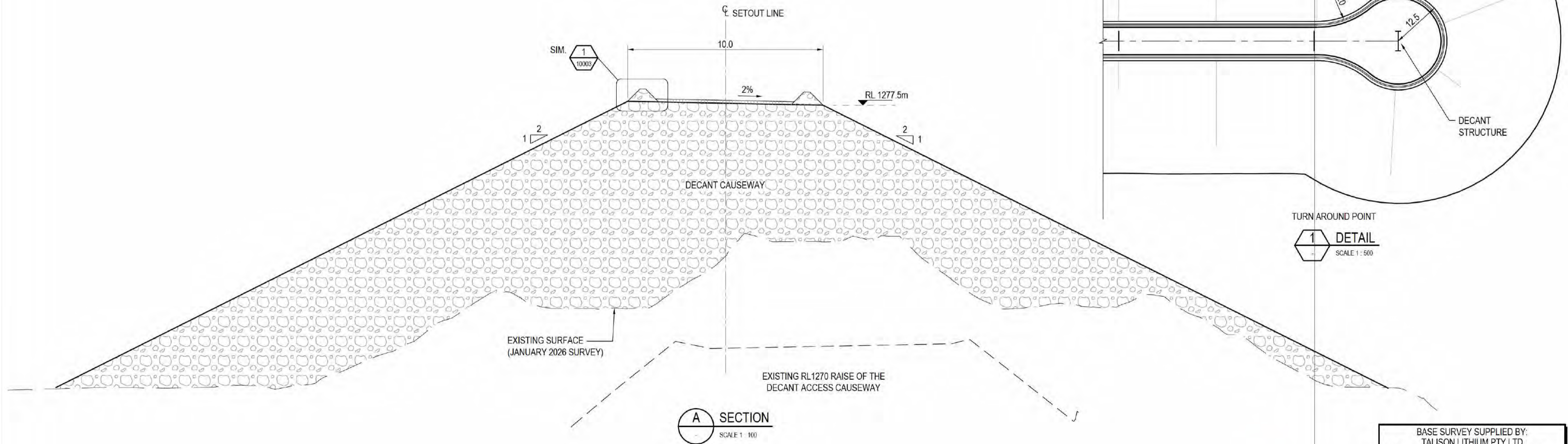
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A1



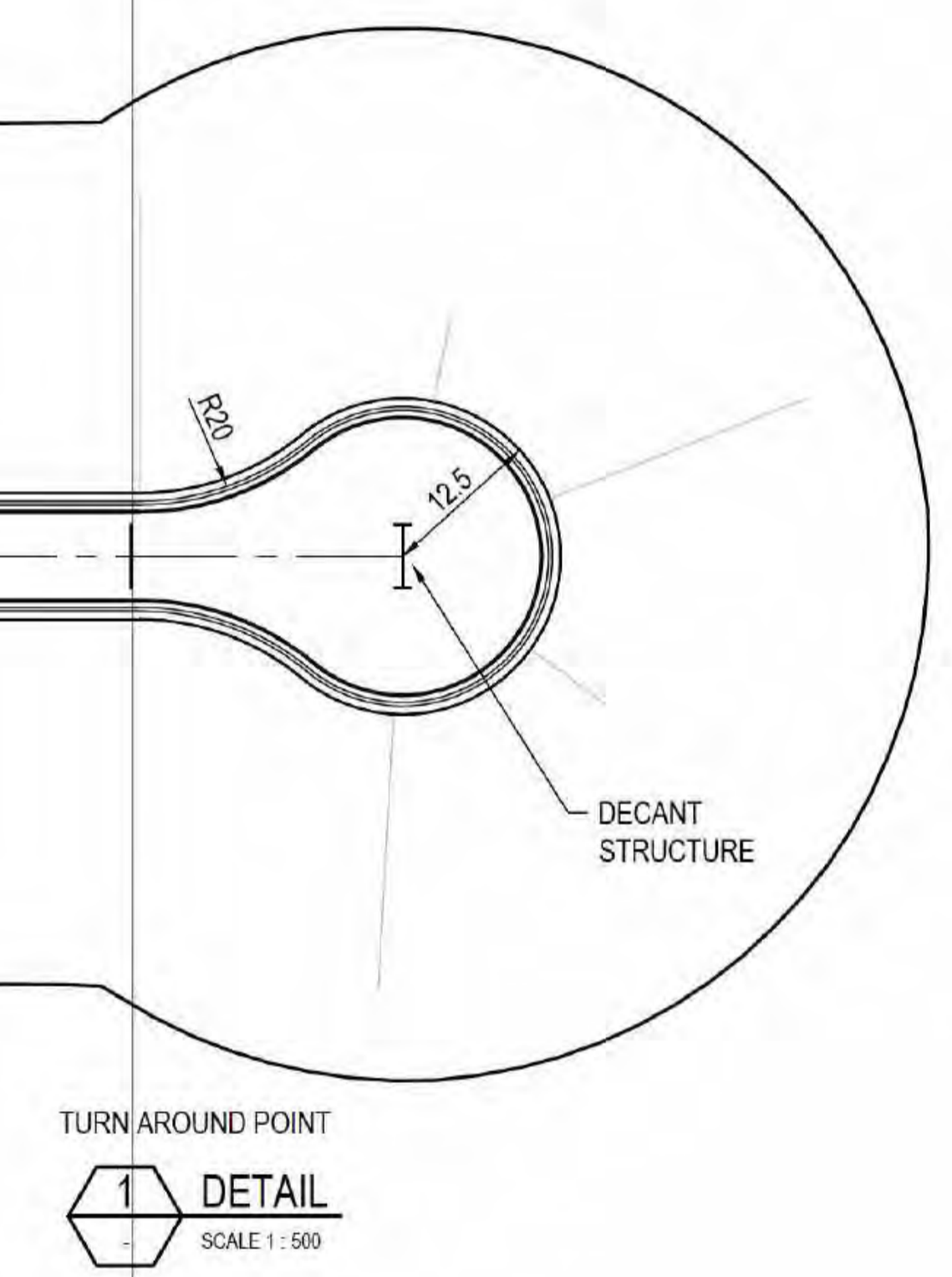
- NOTES:**
1. ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
 2. ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
 3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-C1".
 4. WINDROWS TO INCLUDE BREAKS EVERY 40m FOR DRAINAGE.
 6. REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.

- LEGEND:**
- MINE WASTE ROCK
 - WEARING COURSE
 - TAILINGS

PLAN
SCALE 1:1000



A SECTION
SCALE 1:100

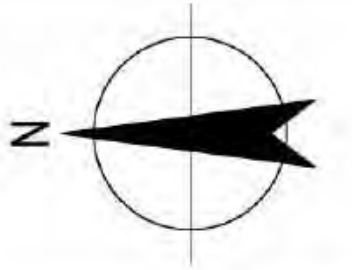
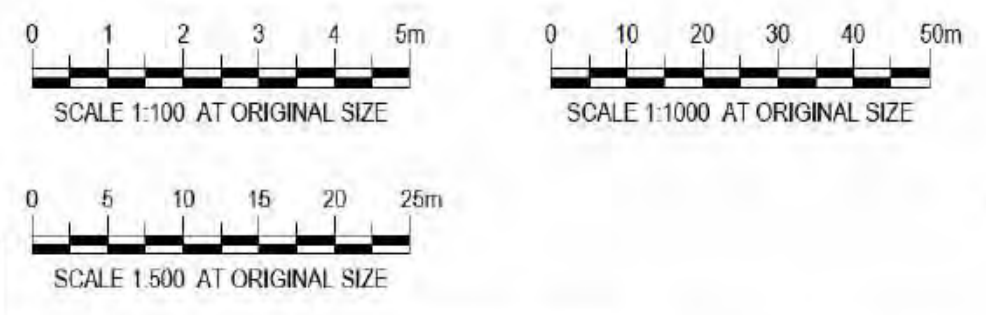


TURN AROUND POINT
1 DETAIL
SCALE 1:500

BASE SURVEY SUPPLIED BY:
TALISON LITHIUM PTY LTD
RECEIVED ON: TSF4, 21 DECEMBER 2025
TSF1, 5 JANUARY 2026
EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM

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001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
Rev	Description	Checked	Approved	Date



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Project No.
12685165

Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO
RL1277.5m
Status FOR CONSTRUCTION

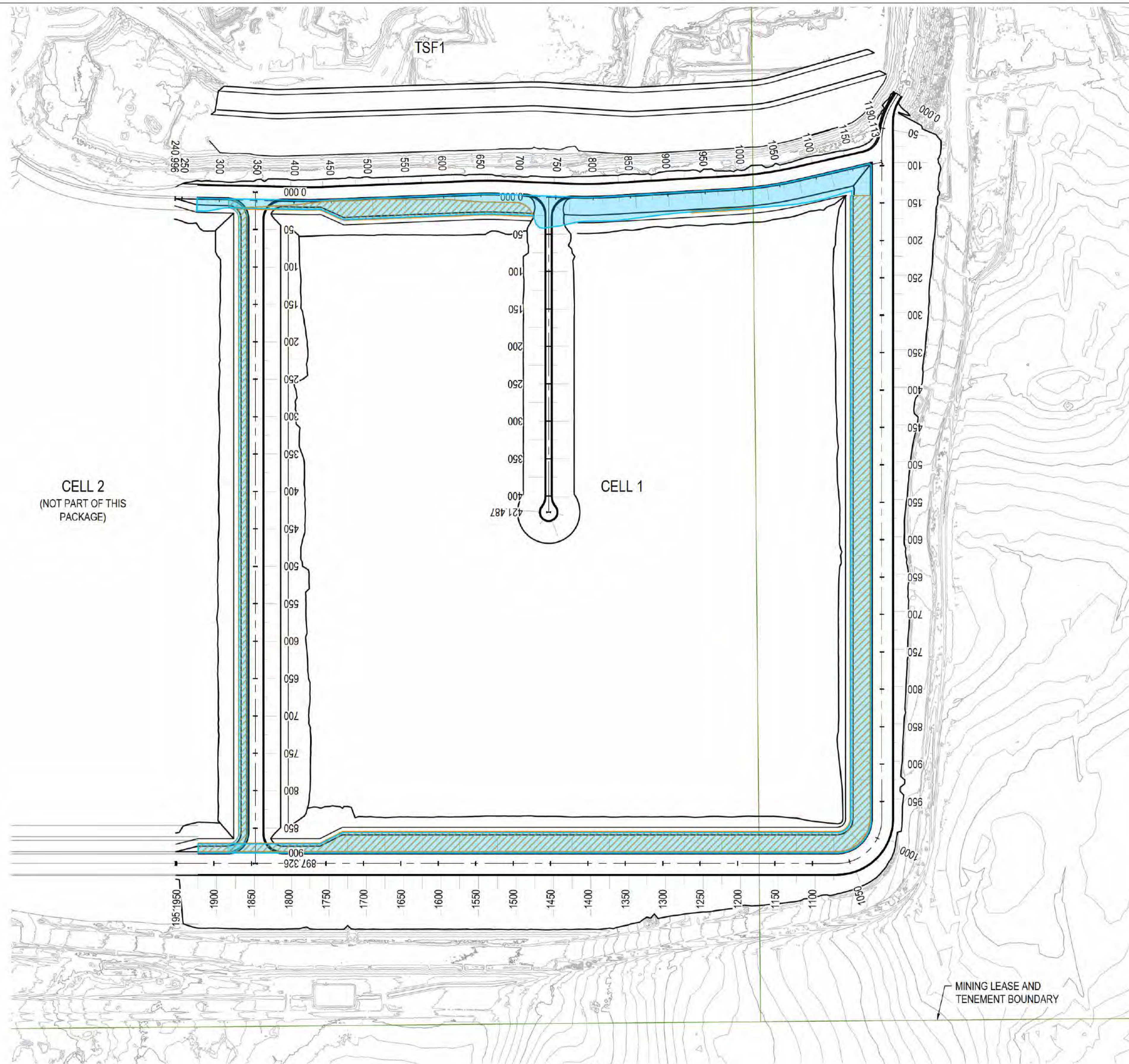
Drawing Title
Size A1
Drawing No. Rev

NOTES:

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2. ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-C1".
4. CELL 2 WORKS BY OTHERS
5. REFER TO TECHNICAL SPECIFICATIONS FOR BGM MATERIAL REQUIREMENTS
ES3 TO BE INSTALLED ON EMBANKMENT SLOPES, AND
ES1 TO BE INSTALLED ON THE TAILINGS PLATFORM FLOOR.
6. REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND
COMPACTION REQUIREMENTS.

LEGEND:

-  TENEMENT BOUNDARY
-  BGM LINER ES3
-  BGM LINER ES1

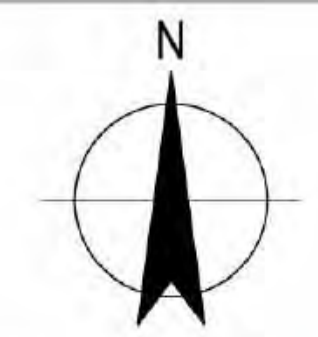
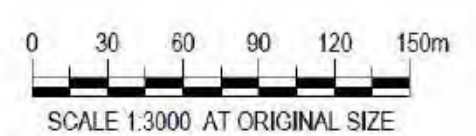


PLAN
SCALE 1:3000

BASE SURVEY SUPPLIED BY:
TALISON LITHIUM PTY LTD
RECEIVED ON: TSF4, 21 DECEMBER 2025
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EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM

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001 ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
Rev	Description	Checked	Approved



Client TALISON LITHIUM PTY LTD
Project TSF4 CELL 1 REDESIGN TO
RL1277.5m
Status FOR CONSTRUCTION

Drawing Title

Size
A1

Drawing No. Rev

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NOTES:

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3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-CI-".
4. CELL 2 WORKS BY OTHERS.
5. FOR SETOUT OF CELL 1 TAILINGS ELEVATED DRAINS MODEL REFER TO 12685165-GHD-01-77-MDL-CI-00002.

LEGEND:

- TENEMENT BOUNDARY
- TAILINGS ELEVATED DRAINS

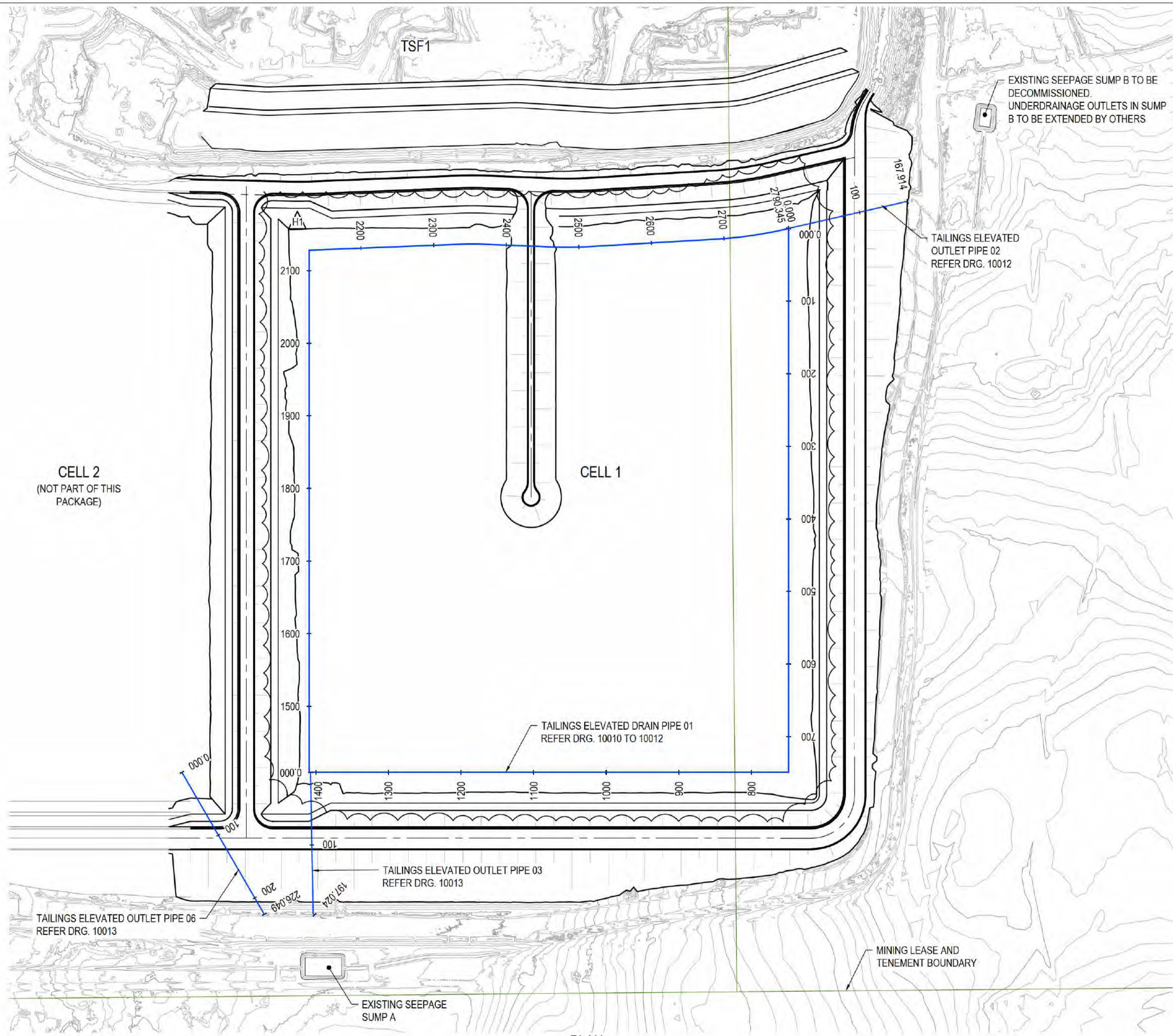
ITEMS ON HOLD:

- H1 - CONSTRUCTION OF ELEVATED DRAIN BY PRINCIPAL.

**FOR INFORMATION ONLY
NOT PART OF CONTRACT**

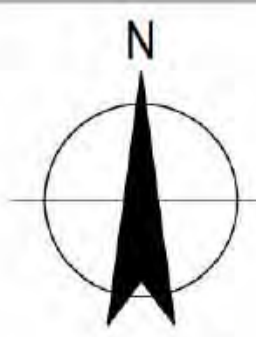
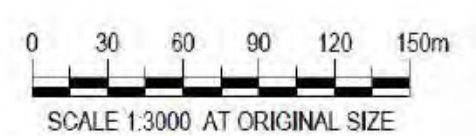
BASE SURVEY SUPPLIED BY:
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 RECEIVED ON: TSF4, 21 DECEMBER 2025
 TSF1, 5 JANUARY 2026
 EXTERNAL AREA, 4 OCT 2024
 CO-ORDINATE GRID: GDA2020 MGA50
 LEVEL DATUM: MINE DATUM

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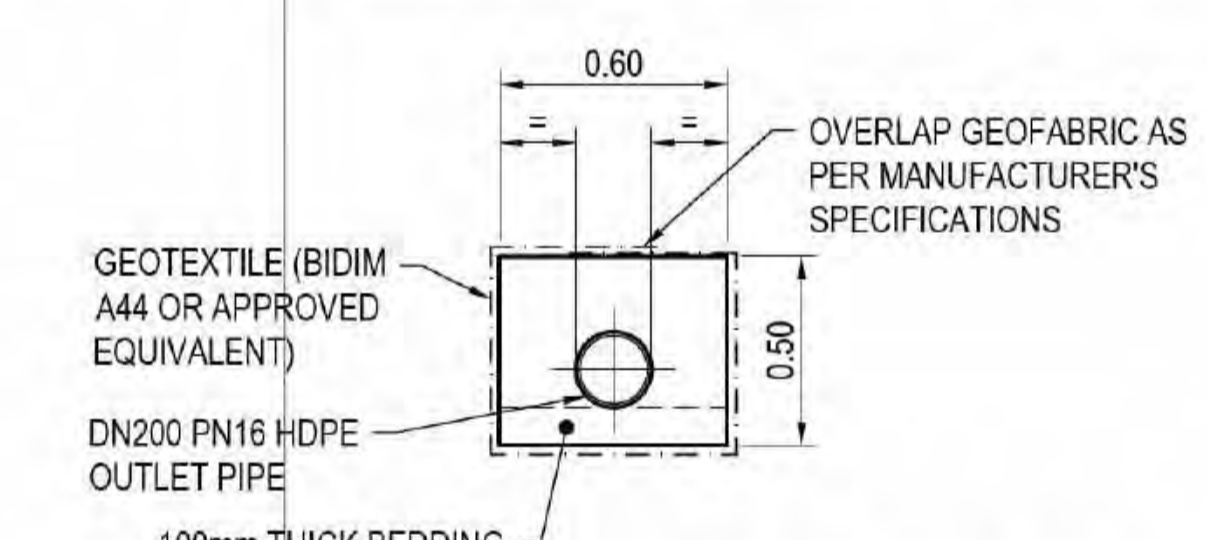
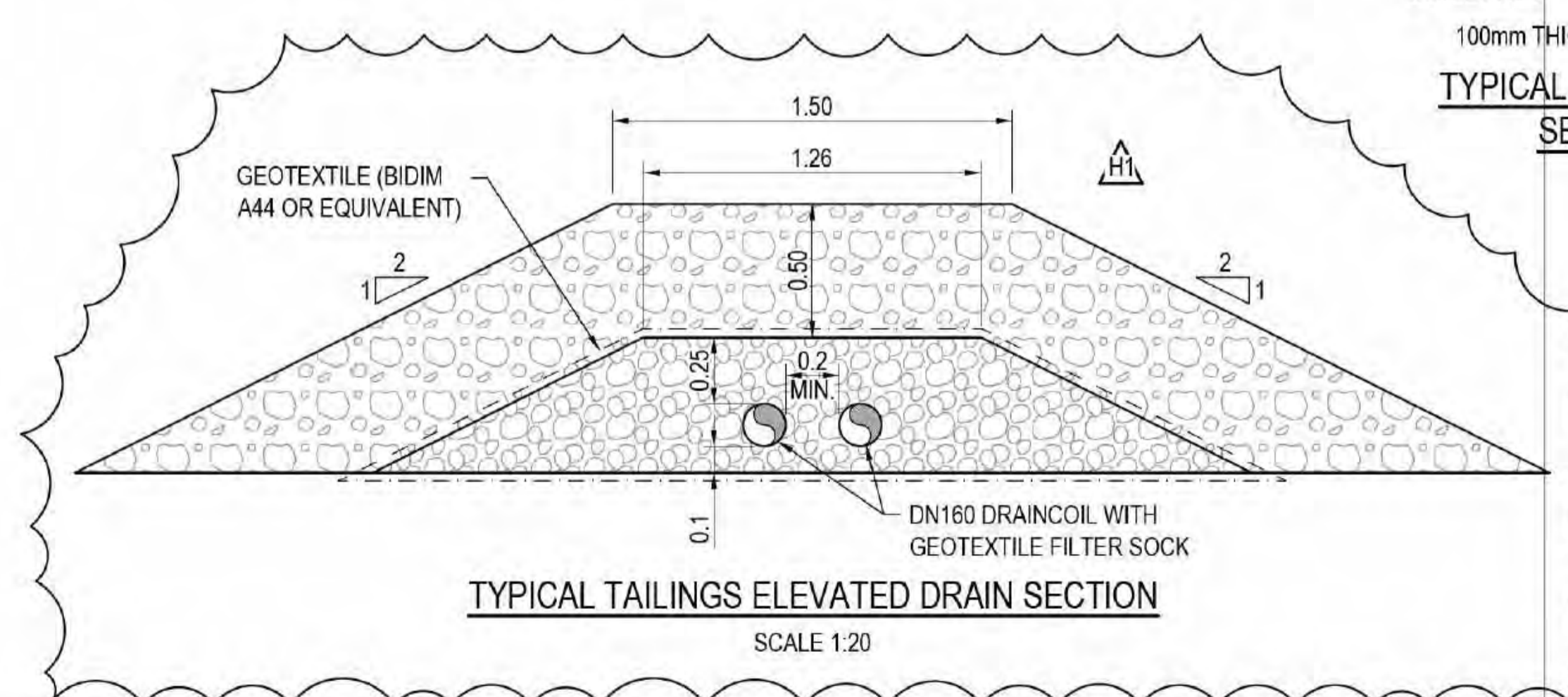
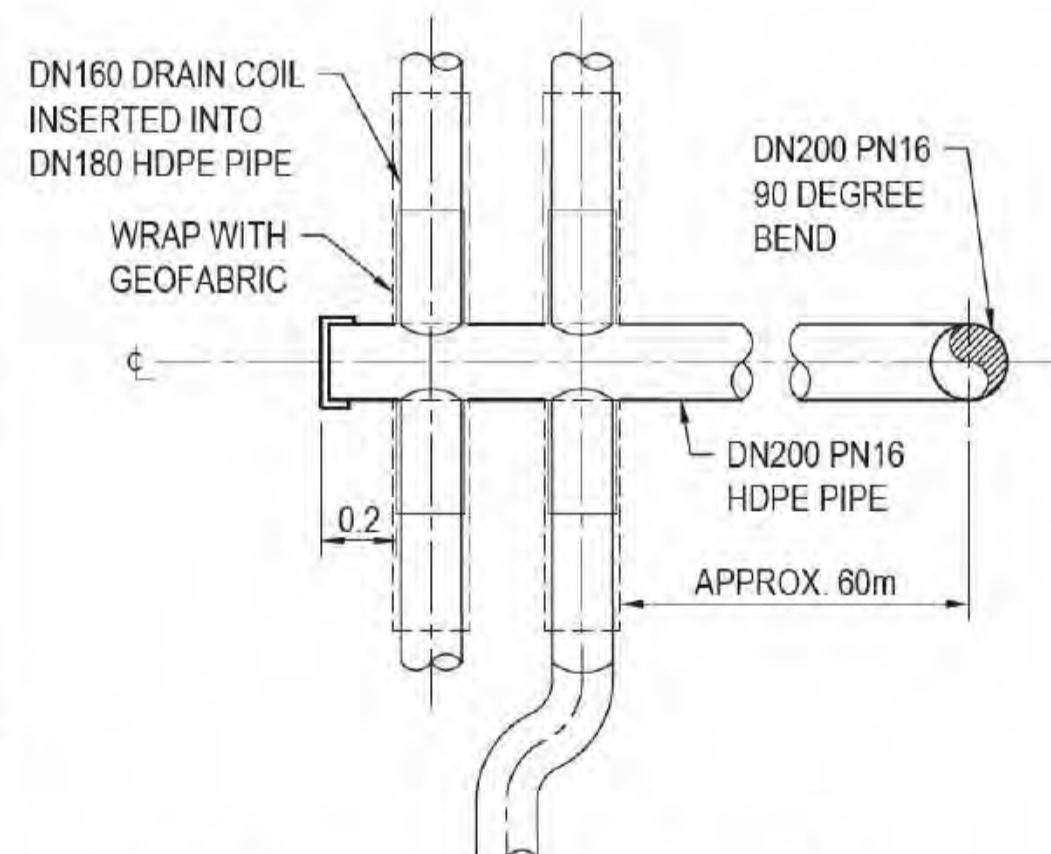
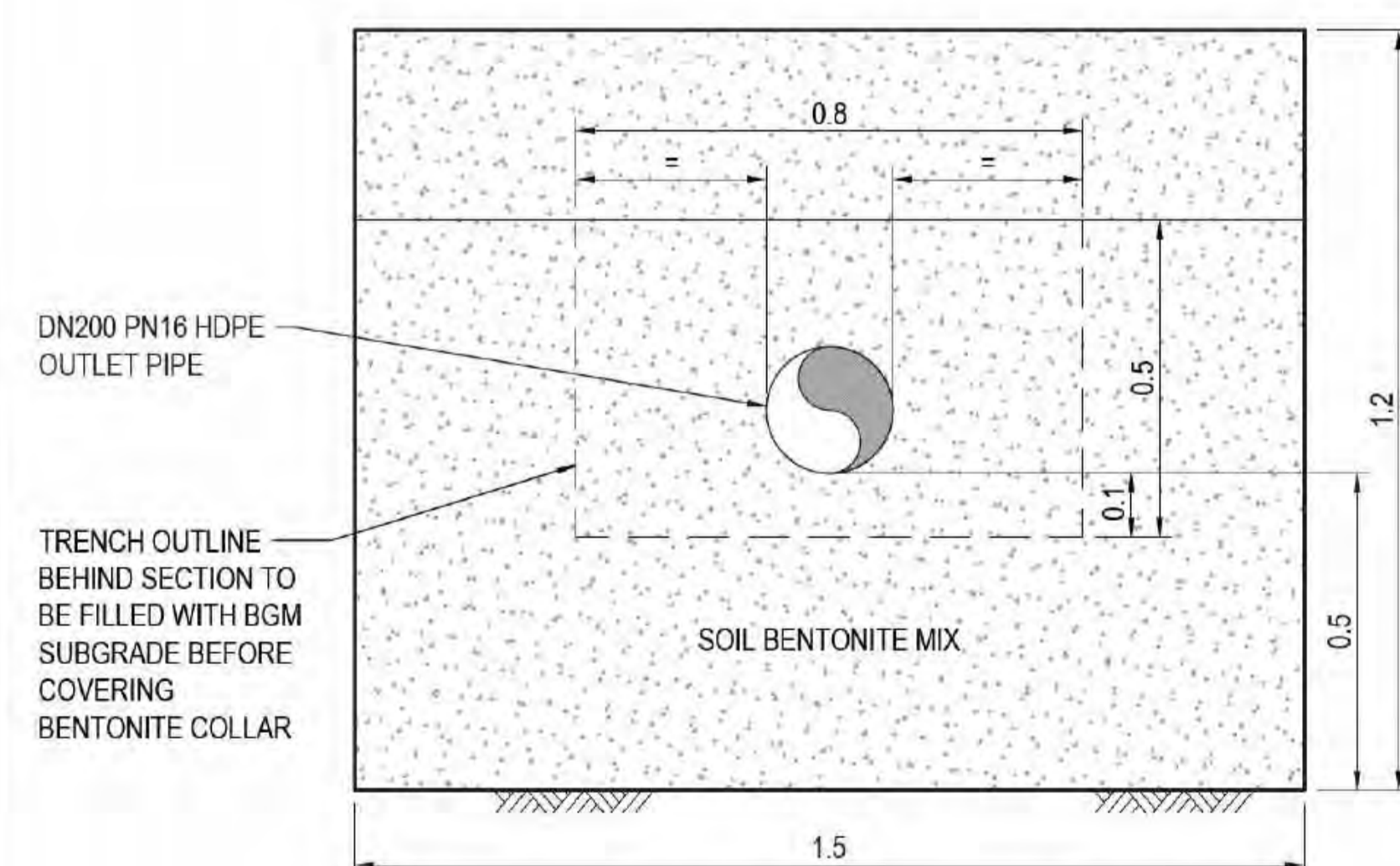
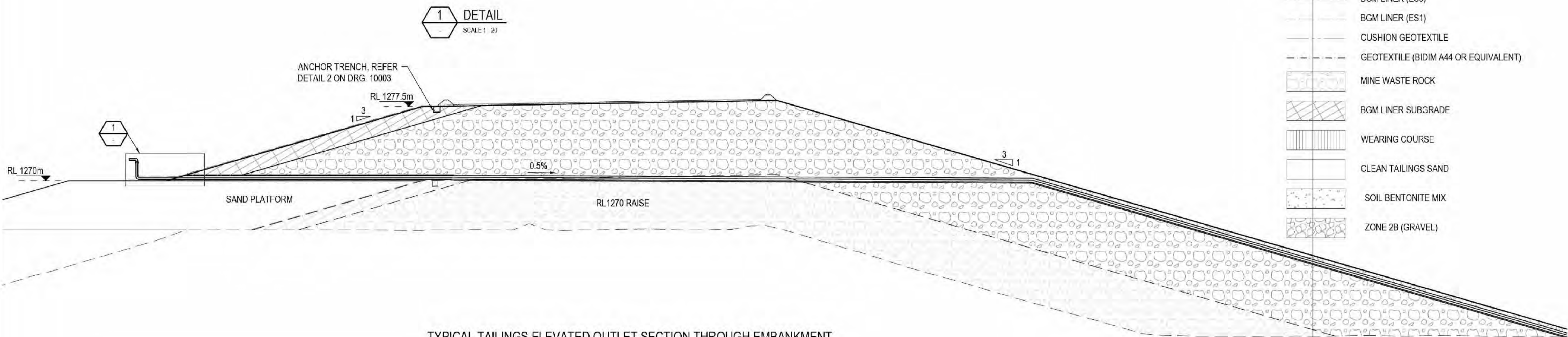
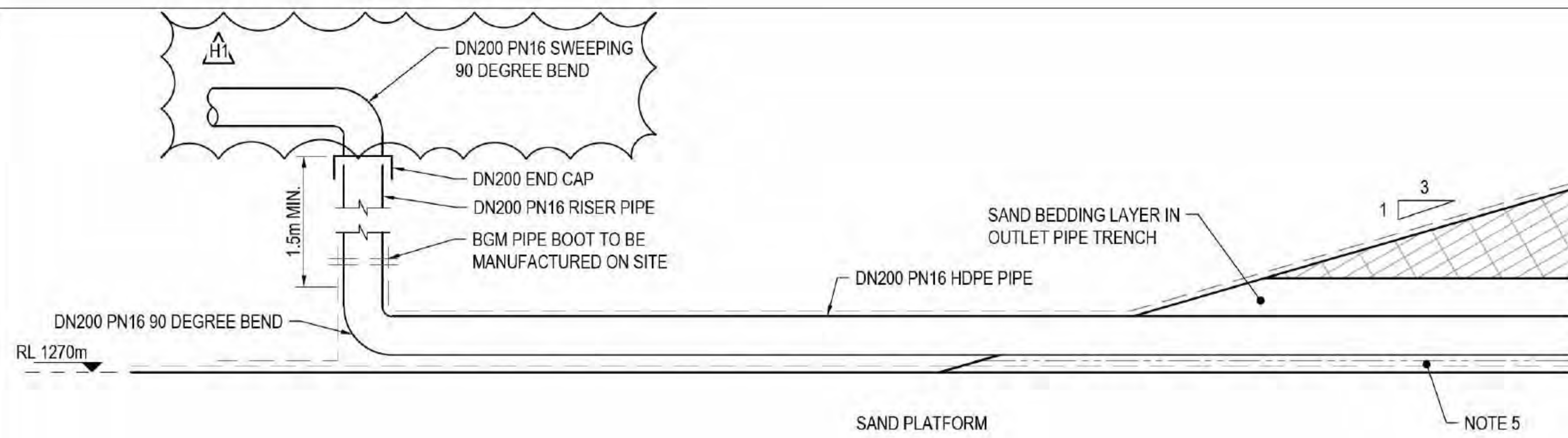
PLAN
SCALE 1:3000

Rev	Description	Checked	Approved	Date
001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26



Client	TALISON LITHIUM PTY LTD
Project	TSF4 CELL 1 REDESIGN TO RL1277.5m
Status	FOR CONSTRUCTION

Drawing Title	
Project No.	12685165
Size	A1
Drawing No.	
Rev	



ITEMS ON HOLD:
 H1 - CONSTRUCTION OF ELEVATED DRAIN BY PRINCIPAL.

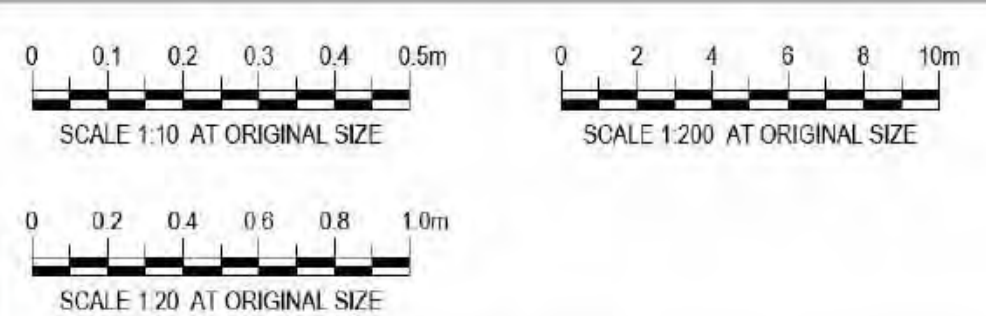
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BASE SURVEY SUPPLIED BY:
 TALISON LITHIUM PTY LTD
 RECEIVED ON: TSF4, 21 DECEMBER 2025
 TSF1, 5 JANUARY 2026
 EXTERNAL AREA, 4 OCT 2024
 CO-ORDINATE GRID: GDA2020 MGA50
 LEVEL DATUM: MINE DATUM

- NOTES:
- ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
 - ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
 - ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-C1".
 - REFER TO TECHNICAL SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.
 - THE ORDER OF THE BGM PROTECTION LAYER IS:
 (1) TAILINGS SAND PLATFORM, OVERLAIN BY
 (2) ES1 BGM, OVERLAIN BY
 (3) CUSHION GEOTEXTILE, OVERLAIN BY
 (4) SAND BEDDING LAYER, OVERLAIN BY
 (5) DN200 PN16 HDPE PIPE, OVERLAIN BY
 (6) SAND BEDDING LAYER, OVERLAIN BY
 (7) BGM SUBGRADE.

- LEGEND:
- BGM LINER (ES3)
 - BGM LINER (ES1)
 - CUSHION GEOTEXTILE
 - GEOTEXTILE (BIDIM A44 OR EQUIVALENT)
 - MINE WASTE ROCK
 - BGM LINER SUBGRADE
 - WEARING COURSE
 - CLEAN TAILINGS SAND
 - SOIL BENTONITE MIX
 - ZONE 2B (GRAVEL)

001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
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Client TALISON LITHIUM PTY LTD
 Project TSF4 CELL 1 REDESIGN TO RL1277.5m
 Status FOR CONSTRUCTION

Project No. 12685165
 Drawing No. A1
 Rev



JOINS DRG 10010

JOINS MATCHLINE B

DATUM RL. 1256.00

VERTICAL ALIGNMENT	L=703.76m G=0.3%										L=691.29m G=0.3%																					
HORIZONTAL ALIGNMENT	L=660.28m																															
LEVEL DIFFERENCE CUT - / FILL +	6.14	8.01	7.84	7.87	7.82	7.72	7.73	7.56	7.59	11.33	11.28	11.36	11.81	12.06	11.87	11.70	10.25	10.34	10.55	10.43	10.30	10.42	12.19	9.11	9.12	9.33	9.20	9.21				
DESIGN SURFACE LEVEL	1274.45	1272.36	1272.35	1272.27	1272.21	1272.15	1272.06	1272.03	1271.97	1271.91	1271.85	1271.83	1271.87	1271.92	1271.92	1271.85	1271.72	1271.52	1271.42	1271.42	1271.52	1271.52	1271.52	1271.62	1271.72	1271.72	1271.82	1271.82	1271.92			
EXISTING SURFACE LEVEL	1268.31	1264.38	1264.49	1264.41	1264.39	1264.44	1264.36	1264.48	1264.38	1263.99	1263.57	1263.47	1263.96	1263.67	1263.12	1263.35	1263.22	1262.03	1262.04	1262.01	1261.86	1262.04	1262.23	1262.17	1260.46	1263.67	1263.67	1263.72	1263.57	1263.75		
CHAINAGE	1200.00	1220.00	1240.00	1260.00	1280.00	1300.00	1320.00	1340.00	1360.00	1380.00	1400.00	1420.23	1420.00	1440.00	1460.00	1480.00	1500.00	1520.00	1540.00	1560.00	1580.00	1600.00	1620.00	1640.00	1660.00	1680.00	1700.00	1720.00	1740.00	1760.00	1780.00	1800.00

LONGITUDINAL SECTION - PIPE 01

HORZ 1:1000 VERT 1:200

JOINS MATCHLINE B

JOINS DRG 10012

DATUM RL. 1258.00

VERTICAL ALIGNMENT	L=891.29m G=0.3%										L=691.83m G=0.3%																											
HORIZONTAL ALIGNMENT	L=719.07m										L=199.47m																											
LEVEL DIFFERENCE CUT - / FILL +	9.21	9.30	9.28	9.28	9.27	9.38	9.50	9.63	10.21	11.76	9.61	9.82	9.67	9.61	9.57	9.51	9.52	9.25	9.05	8.99	9.09	9.13	9.39	9.41	9.44	9.57	9.70	9.84	10.06	10.19	10.17	10.17	10.06	10.02	10.11	5.70		
DESIGN SURFACE LEVEL	1273.01	1273.07	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12	1273.12
EXISTING SURFACE LEVEL	1263.81	1263.77	1263.86	1263.84	1263.85	1263.83	1263.88	1263.88	1263.91	1263.96	1264.00	1264.03	1264.06	1264.06	1264.18	1264.28	1264.40	1264.38	1264.59	1264.77	1264.60	1264.69	1264.60	1264.28	1264.19	1264.10	1263.91	1263.72	1263.84	1263.85	1263.85	1263.85	1263.85	1263.85	1263.85	1263.85	1263.85	1263.85
CHAINAGE	1800.00	1820.00	1840.00	1860.00	1880.00	1900.00	1920.00	1940.00	1960.00	1980.00	2000.00	2020.00	2040.00	2060.00	2080.00	2098.52	2100.00	2120.00	2128.30	2136.85	2140.00	2160.00	2180.00	2200.00	2220.00	2240.00	2260.00	2280.00	2300.00	2320.00	2336.32	2340.00	2360.00	2370.26	2380.00	2400.00		

LONGITUDINAL SECTION - PIPE 01

HORZ 1:1000 VERT 1:200

NOTES:

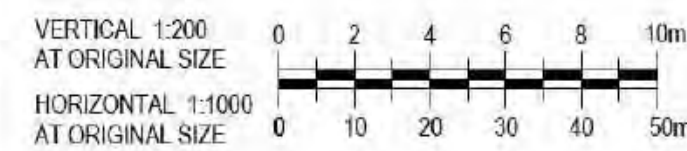
1. ALL DIMENSIONS ARE IN METRES (m) UNLESS OTHERWISE NOTED.
2. ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-CI-".
4. FOR SETOUT OF CELL 1 TAILINGS ELEVATED DRAINS MODEL REFER TO 12685165-GHD-01-77-MDL-CI-00002.

ITEMS ON HOLD:

H1 - CONSTRUCTION OF ELEVATED DRAIN BY PRINCIPAL.



BASE SURVEY SUPPLIED BY:
TALISON LITHIUM PTY LTD
RECEIVED ON: TSF4, 21 DECEMBER 2025
TSF1, 5 JANUARY 2026
EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM



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Client TALISON LITHIUM PTY LTD

Project TSF4 CELL 1 REDESIGN TO RL1277.5m

Status FOR CONSTRUCTION

Drawing Title

Size A1

Drawn: No Rev

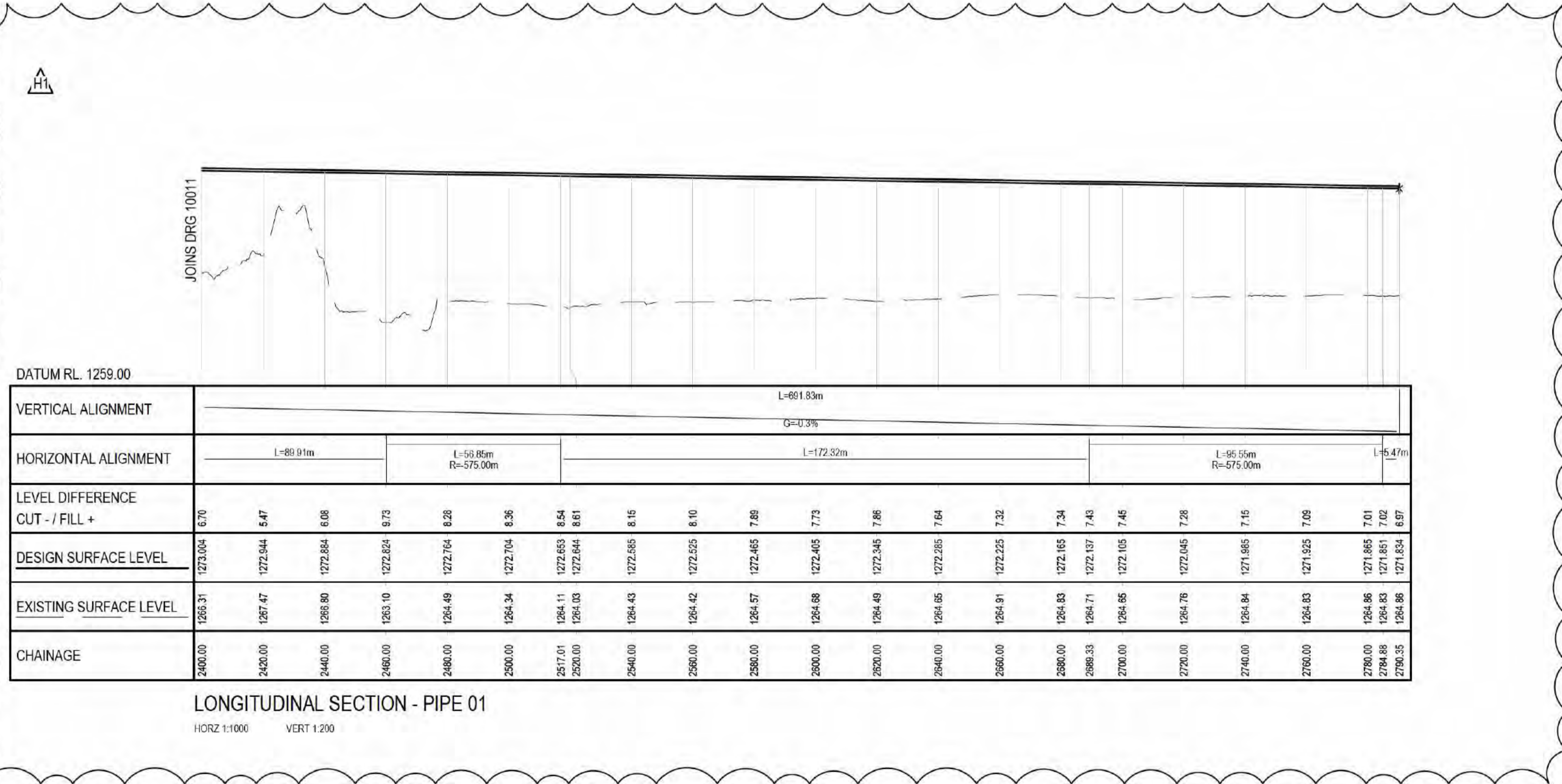
001 ISSUED FOR CONSTRUCTION SW DC 23/04/26

NOTES:

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2. ALL RL'S ARE IN METRES TO LOCAL MINE DATUM (m).
3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-CI".
4. FOR SETOUT OF CELL 1 TAILINGS ELEVATED DRAINS MODEL REFER TO 12655165-GHD-01-77-MDL-CI-00002.

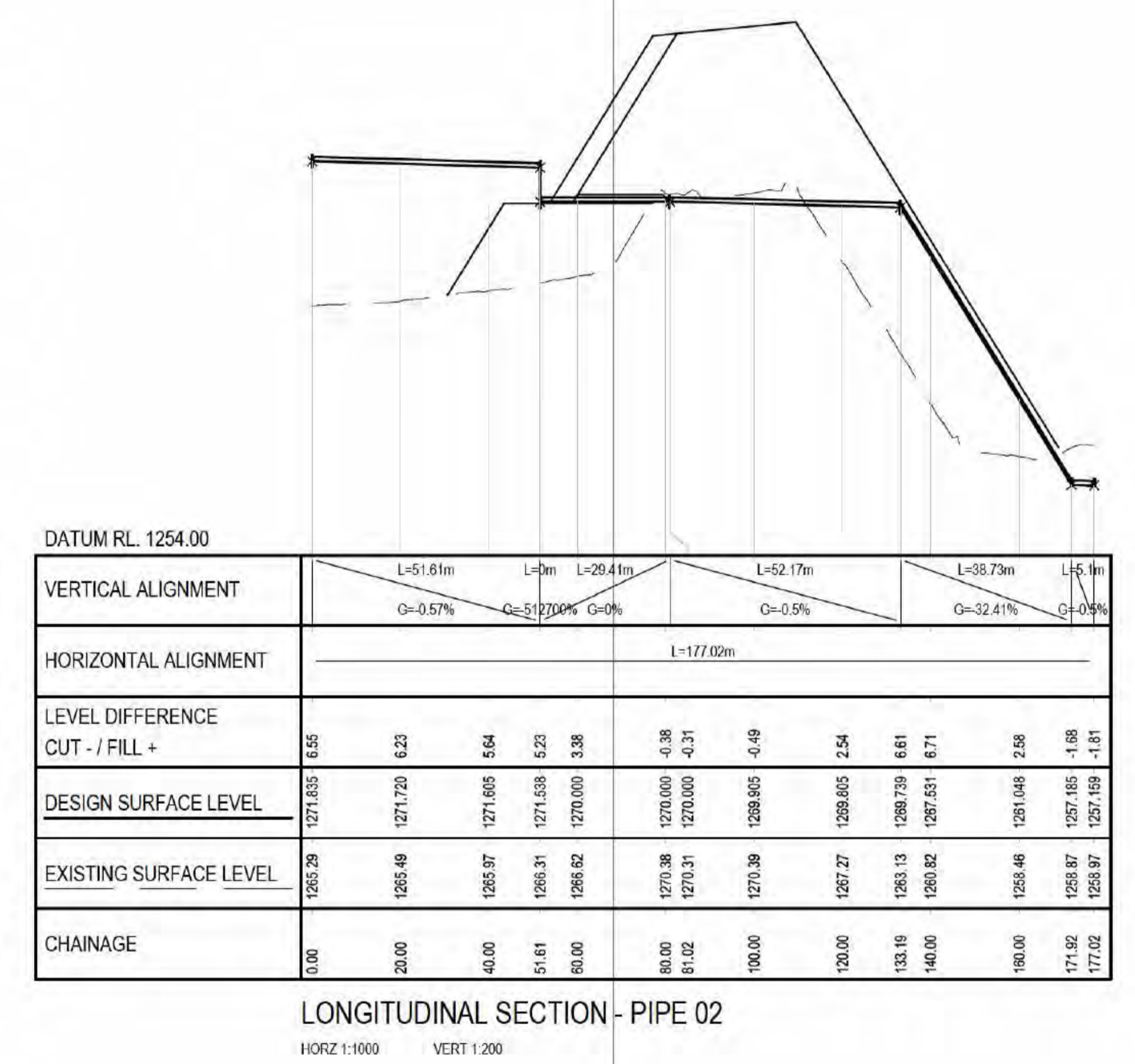
ITEMS ON HOLD:

H1 - CONSTRUCTION OF ELEVATED DRAIN BY PRINCIPAL.



LONGITUDINAL SECTION - PIPE 01

HORZ 1:1000 VERT 1:200



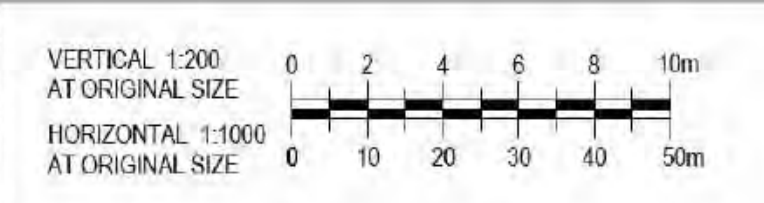
LONGITUDINAL SECTION - PIPE 02

HORZ 1:1000 VERT 1:200

BASE SURVEY SUPPLIED BY:
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RECEIVED ON: TSF4, 21 DECEMBER 2025
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EXTERNAL AREA, 4 OCT 2024
CO-ORDINATE GRID: GDA2020 MGA50
LEVEL DATUM: MINE DATUM



001	ISSUED FOR CONSTRUCTION	SW	DC	23/04/26
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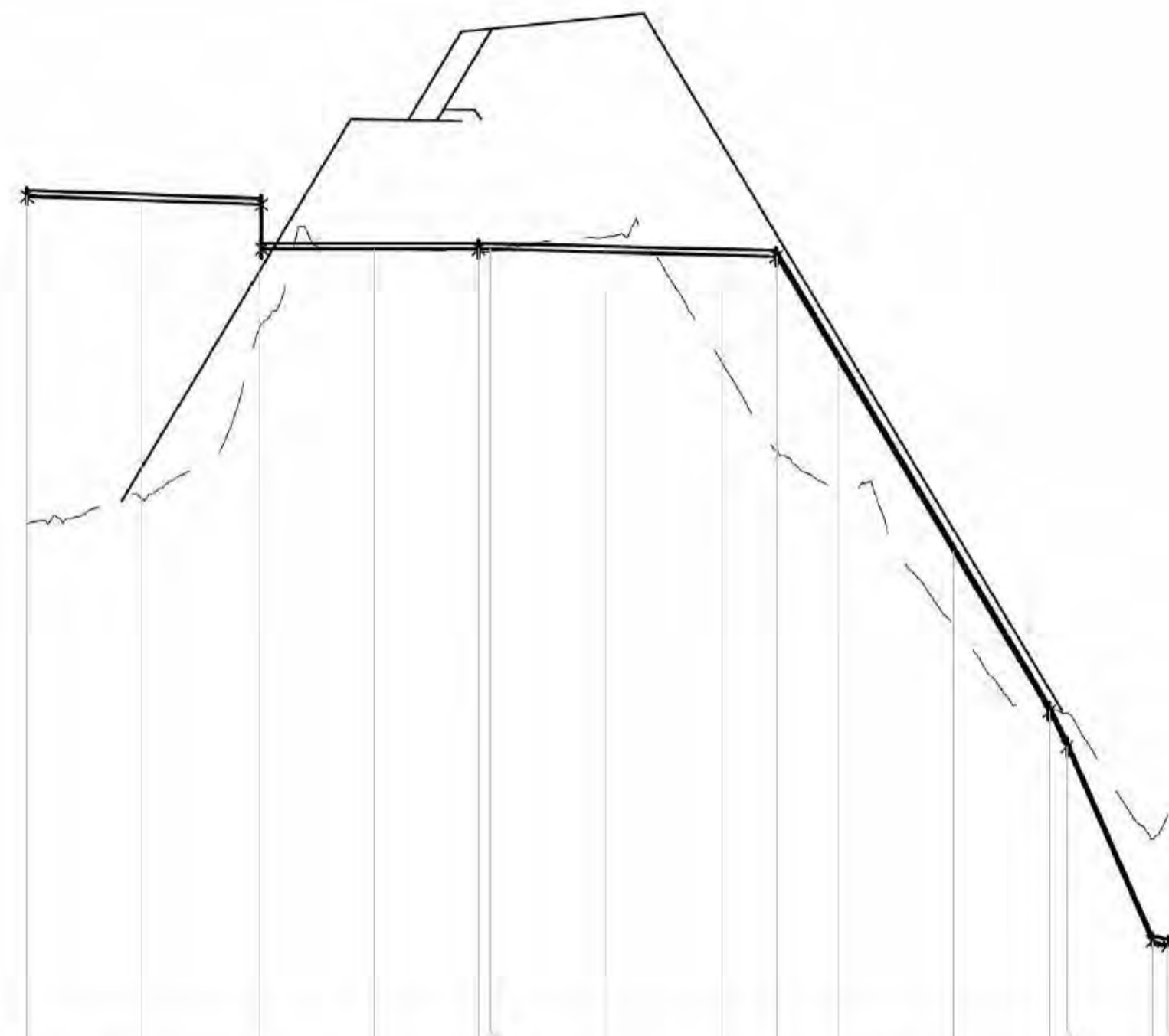


Client	TALISON LITHIUM PTY LTD
Project	TSF4 CELL 1 REDESIGN TO RL1277.5m
Status	FOR CONSTRUCTION

Drawing Title	
Project No.	12685165
Size	A1
Drawn By	
Rev	

NOTES:

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2. ALL RL's ARE IN METRES TO LOCAL MINE DATUM (m).
3. ALL DRAWING REFERENCES ARE PREFIXED BY "12685165-GHD-01-77-DRG-CI-".
4. FOR SETOUT OF CELL 1 TAILINGS ELEVATED DRAINS MODEL REFER TO 12655165-GHD-01-77-MDL-CI-00002.

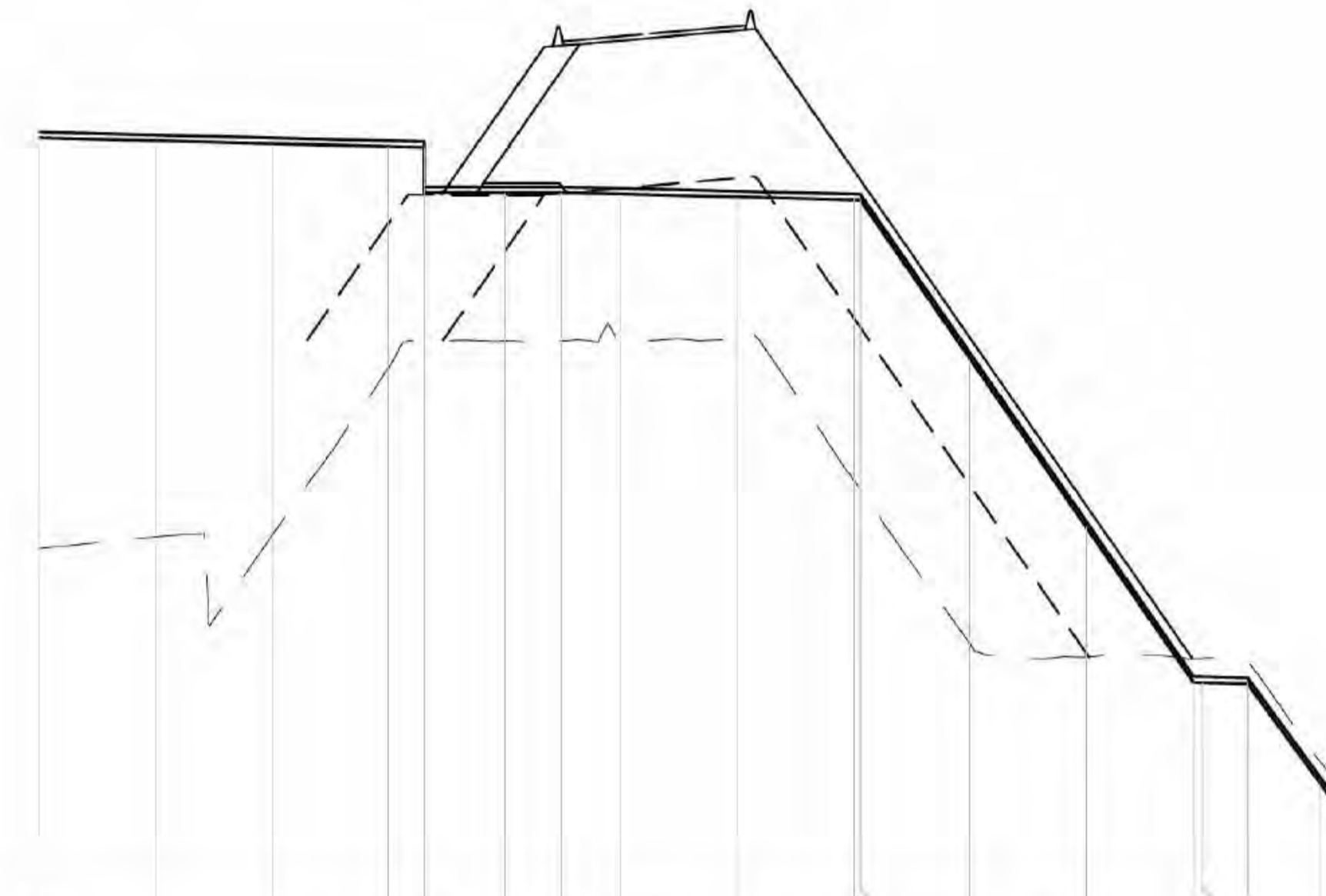


DATUM RL. 1242.00

VERTICAL ALIGNMENT	L=40.56m L=0m L=37.45m L=51.46m L=47.09m L=3.02m L=14.68m L=2.73m G=-0.7% G=-2.18% G=1.14% G=0% G=-0.5% G=-33.35% G=-0.62% G=5.85% G=-5%														
HORIZONTAL ALIGNMENT	L=194.29m L=2.73m														
LEVEL DIFFERENCE CUT - / FILL +	11.35	10.34	4.36	0.01	0.15	0.09	-0.53	3.70	6.70	4.55	2.53	-0.05	-1.18	3.54	4.51
DESIGN SURFACE LEVEL	1271.822	1271.852	1271.542	1270.000	1270.000	1269.990	1269.990	1269.790	1269.743	1269.231	1259.951	1254.037	1252.268	1246.984	1245.947
EXISTING SURFACE LEVEL	1260.47	1261.34	1267.19	1269.99	1269.85	1269.90	1270.42	1266.09	1263.04	1261.68	1257.03	1254.08	1253.96	1246.63	1250.45
CHAINAGE	0.00	20.00	40.00	60.00	76.01	80.00	100.00	120.00	129.47	140.00	160.00	175.56	179.63	194.26	197.02

LONGITUDINAL SECTION - PIPE 03

HORZ 1:1000 VERT 1:200

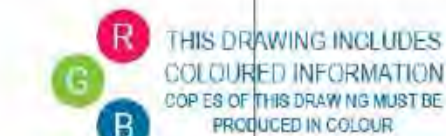


DATUM RL. 1245.00

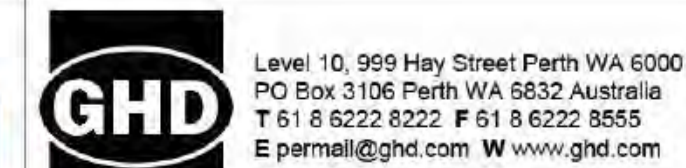
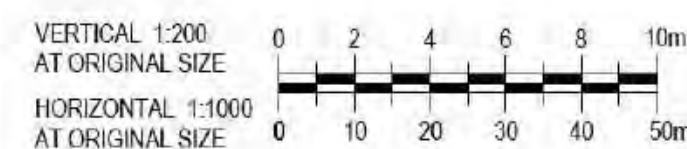
VERTICAL ALIGNMENT	L=66.11m L=23.9m L=51.43m L=57.24m L=9.35m L=18.32m G=-0.5% G=0% G=-0.5% G=-28.97% G=-0.33% G=-27.42%																	
HORIZONTAL ALIGNMENT	L=225.05m																	
LEVEL DIFFERENCE CUT - / FILL +	14.13	13.62	13.46	7.60	6.67	5.10	5.10	5.08	4.96	9.64	9.95	9.81	4.46	-0.80	-0.82	-0.78	-0.73	-2.45
DESIGN SURFACE LEVEL	1271.885	1271.785	1271.885	1271.585	1271.584	1270.000	1270.000	1269.949	1269.949	1269.749	1269.743	1264.280	1258.486	1253.160	1253.152	1253.113	1248.001	1245.54
EXISTING SURFACE LEVEL	1257.75	1258.17	1258.22	1263.99	1264.89	1264.90	1264.90	1265.19	1265.19	1260.11	1259.80	1254.47	1254.03	1253.95	1253.98	1253.89	1250.45	1250.54
CHAINAGE	0.00	20.00	40.00	60.00	66.11	80.00	89.71	100.00	120.00	140.00	141.14	160.00	180.00	198.38	200.00	207.73	220.00	225.05

LONGITUDINAL SECTION - PIPE 06

HORZ 1:1000 VERT 1:200



BASE SURVEY SUPPLIED BY:
 TALISON LITHIUM PTY LTD
 RECEIVED ON: TSF4, 21 DECEMBER 2025
 TSF1, 5 JANUARY 2026
 EXTERNAL AREA, 4 OCT 2024
 CO-ORDINATE GRID: GDA2020 MGA50
 LEVEL DATUM: MINE DATUM



Client TALISON LITHIUM PTY LTD
 Project TSF4 CELL 1 REDESIGN TO RL1277.5m
 Status FOR CONSTRUCTION

Drawing Title

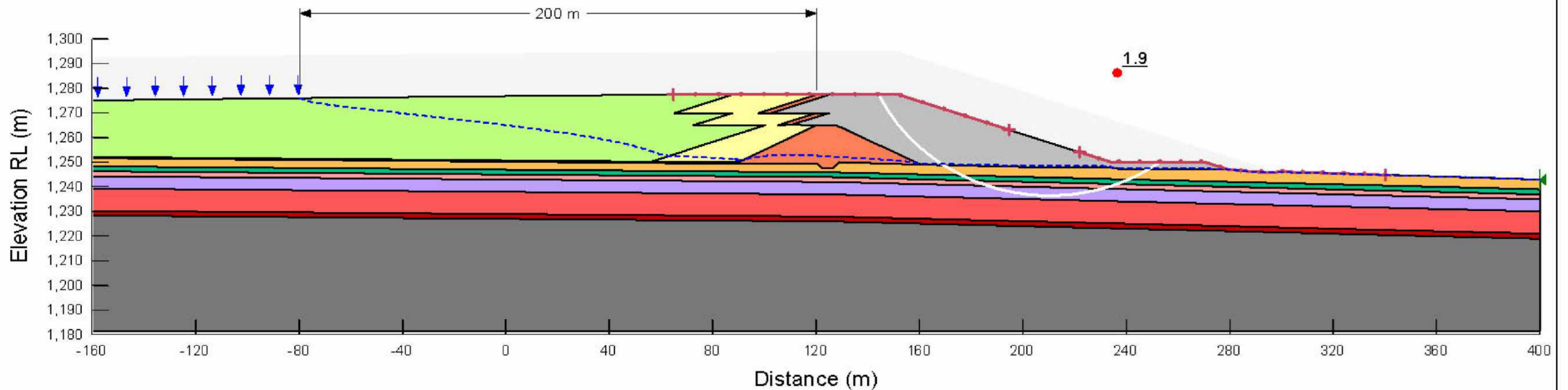
Size A1

001 ISSUED FOR CONSTRUCTION SW DC 23/04/26

Appendix B

Seepage and stability analysis

Color	Name	Slope Stability Material Model	Unit Weight (kNm ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterite s aprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Red	Slope_F3 - Pallid s aprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Purple	Slope_F4 - soft layer_Longterm	Shear/Normal Fn.	17			0	F4_Longterm	
Dark Red	Slope_F5 - Saprolite_Longterm	Shear/Normal Fn.	18			0	F5_Longterm	
Dark Red	Slope_F6 - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	F6_Longterm	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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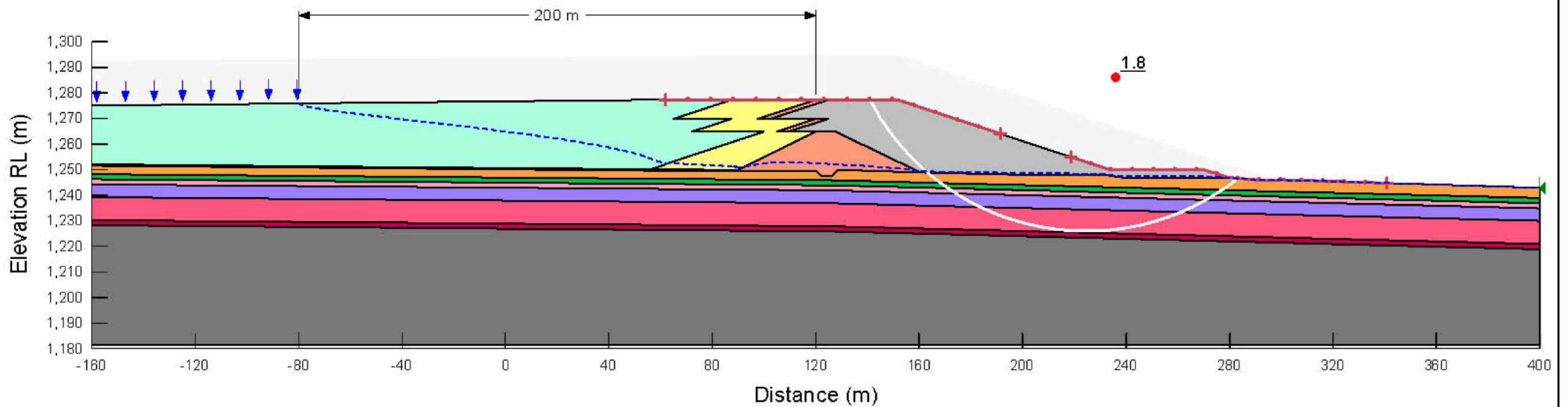
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1277.5 m
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 1

Color	Name	Slope Stability Material Model	Unit Weight (kNm ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Poststatic	SHANSEP	19					0.55	10	
Light Blue	Slope_F1 - laterite - sandy gravel_Poststatic	Shear/Normal Fn.	21			0	F1_Poststatic			
Green	Slope_F2 - Laterised saprolite_Poststatic	Shear/Normal Fn.	20			0	F2_Poststatic			
Pink	Slope_F3 - Palid saprolite_Poststatic	SHANSEP	17					0.33	10	
Light Purple	Slope_F4 - soft layer_Poststatic	SHANSEP	17					0.41	10	
Red	Slope_F5 - Saprolite_Poststatic	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Poststatic	Shear/Normal Fn.	21			0	F6_Poststatic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Poststatic	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Poststatic	Mohr-Coulomb	16	0	28	0				



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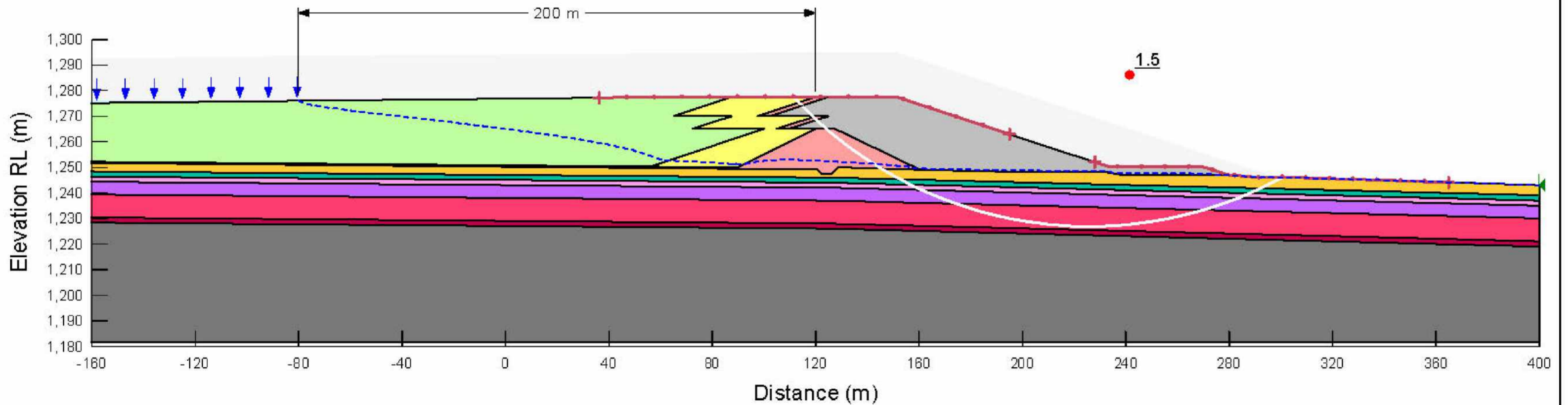
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1277.5 m
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 2

Color	Name	Slope Stability Material Model	Unit Weight (kNm ⁻³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Red	Slope_Clay starter/face_Post seismic	SHANSEP	19					0.44	10	
Yellow	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Green	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Palis saprolite_Post seismic	SHANSEP	17					0.264	10	
Purple	Slope_F4 - soft layer_Post seismic	SHANSEP	17					0.328	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Minewaste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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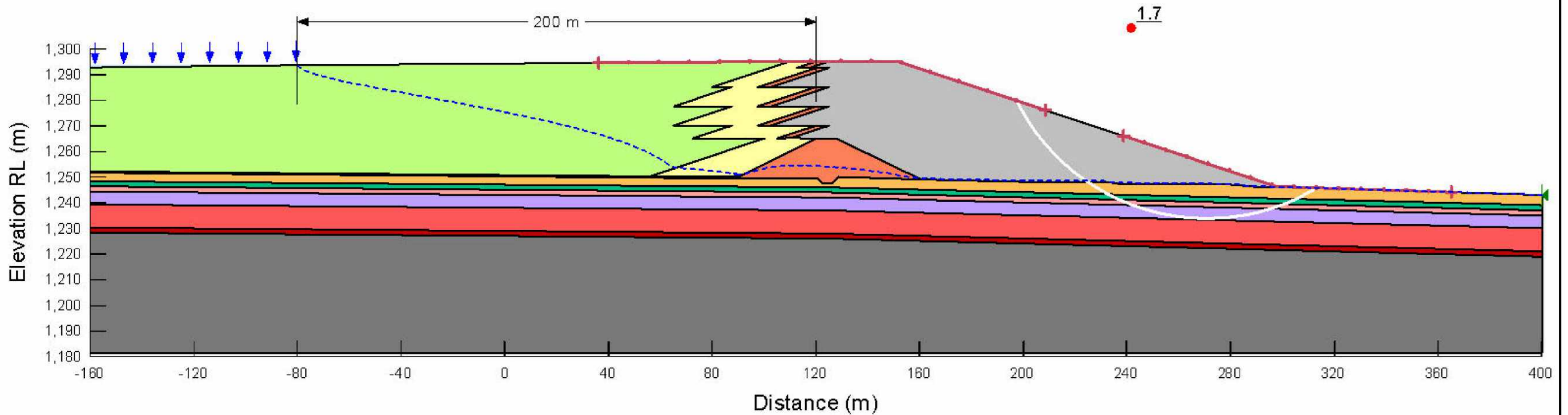
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1277.5 m
 Case: Post Seismic Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 3

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear /Normal Strength Fn.	Undrained Shear Strength (k Pa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterised saprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Pink	Slope_F3 - Palid saprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Purple	Slope_F4 - soft layer_Longterm	Shear/Normal Fn.	17			0	F4_Longterm	
Red	Slope_F5 - Saprolite_Longterm	Shear/Normal Fn.	16			0	F5_Longterm	
Dark Red	Slope_FB - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	FB_Longterm	
Light Grey	Slope_Minewaste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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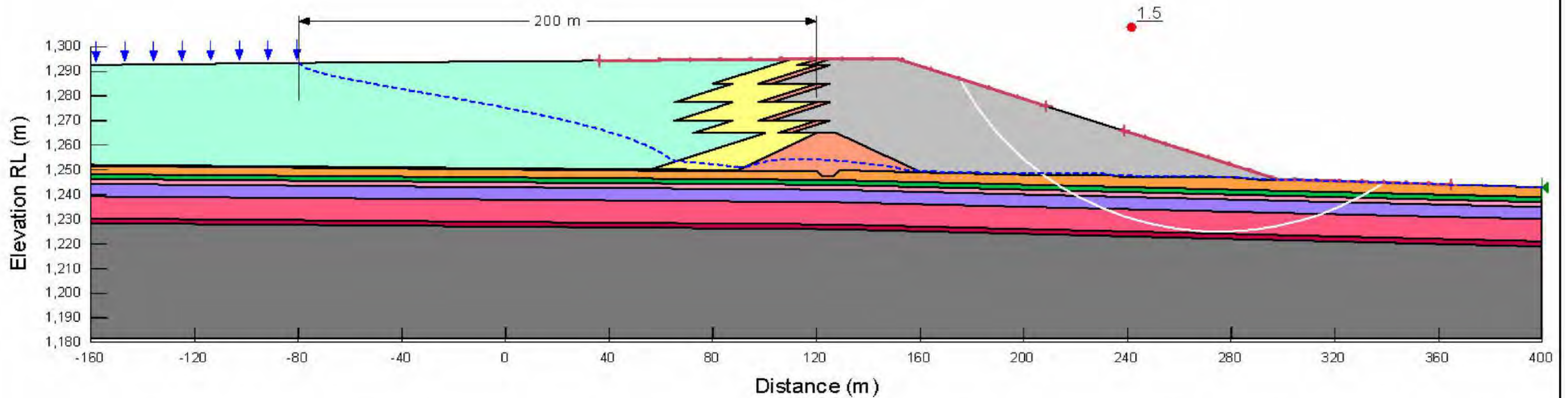
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1295 m
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 4

Color	Name	Slope Stability Material Model	Unit Weight (kNm ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock(Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Post static	SHANSEP	19					0.55	10	
Light Orange	Slope_F1 - laterite - sandy gravel_Post static	Shear/Normal Fn.	21			0	F1_Post static			
Green	Slope_F2 - Laterised saprolite_Post static	Shear/Normal Fn.	20			0	F2_Post static			
Pink	Slope_F3 - Pallid saprolite_Post static	SHANSEP	17					0.33	10	
Purple	Slope_F4 - soft layer_Post static	SHANSEP	17					0.41	10	
Red	Slope_F5 - Saprolite_Post static	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post static	Shear/Normal Fn.	21			0	F6_Post static			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0				



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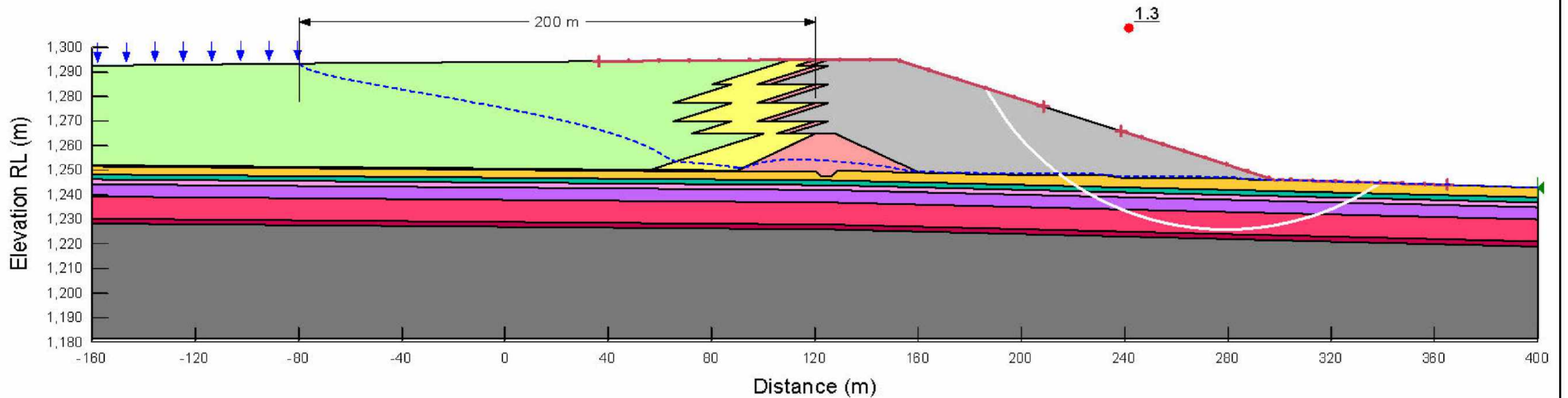
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1295 m
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 5

Color	Name	Slope Stability Material Model	Unit Weight (kNm ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Light Red	Slope_Clay starter/face_Post seismic	SHANSEP	19					0.44	10	
Yellow	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Light Green	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Pallid saprolite_Post seismic	SHANSEP	17					0.264	10	
Purple	Slope_F4 - soft layer_Post seismic	SHANSEP	17					0.328	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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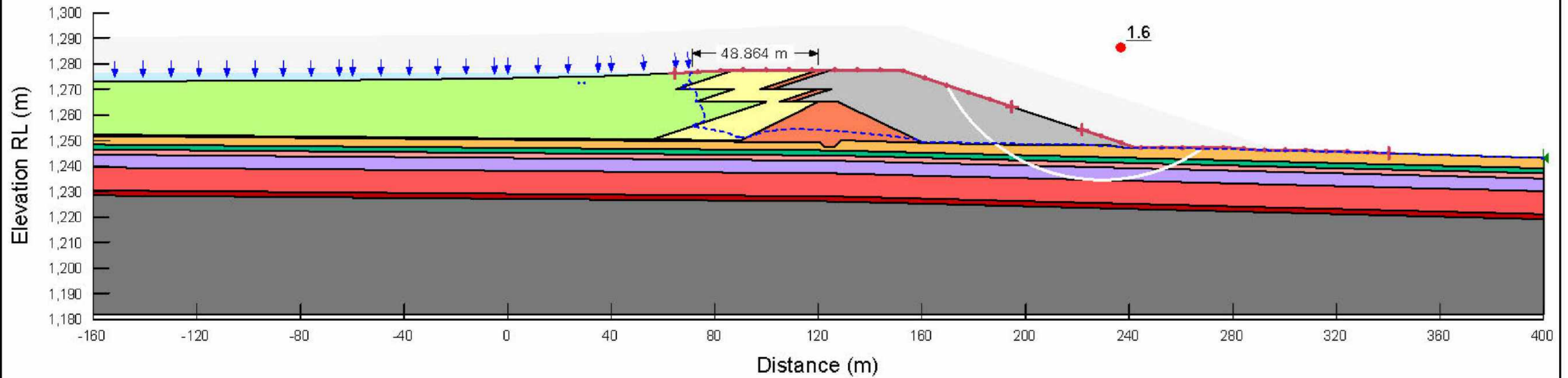
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A RL 1295 m
 Case: Post Seismic Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 6

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterised saprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Pink	Slope_F3 - Pallid saprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Purple	Slope_F4 - soft layer_Longterm	Shear/Normal Fn.	17			0	F4_Longterm	
Red	Slope_F5 - Saprolite_Longterm	Shear/Normal Fn.	18			0	F5_Longterm	
Dark Red	Slope_F6 - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	F6_Longterm	
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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Notes:

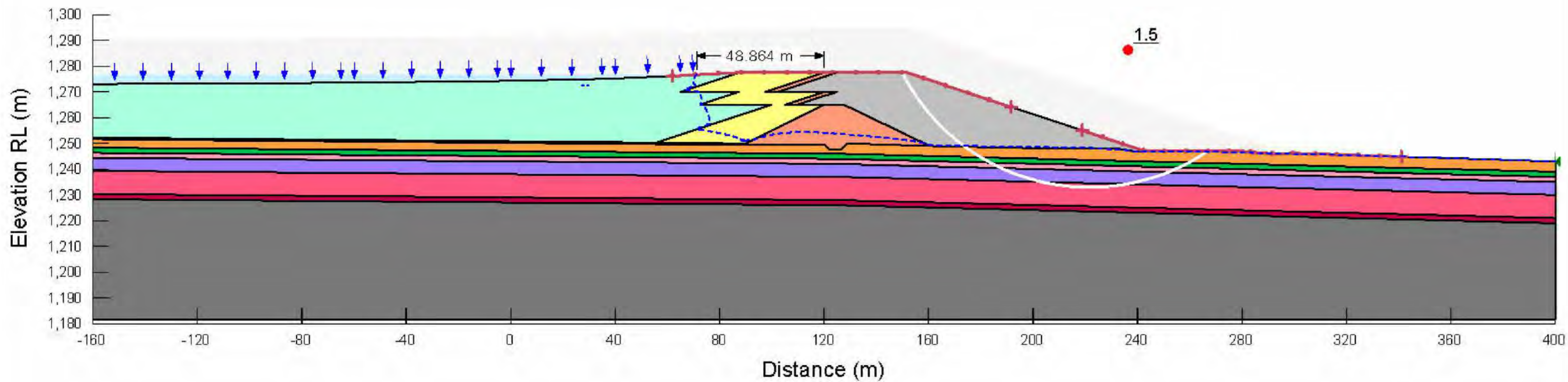
Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 - Section A Sensitivity RL 1277.5 m
Case: Long Term Circular

Project Nro.: 12685165
Revision: A
Date: 30/01/2025

Figure 7

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Post static	SHANSEP	19					0.55	10	
Light Orange	Slope_F1 - laterite - sandy gravel_Post static	Shear/Normal Fn.	21			0	F1_Post static			
Green	Slope_F2 - Laterised saprolite_Post static	Shear/Normal Fn.	20			0	F2_Post static			
Pink	Slope_F3 - Pallid saprolite_Post static	SHANSEP	17					0.33	10	
Purple	Slope_F4 - soft layer_Post static	SHANSEP	17					0.41	10	
Red	Slope_F5 - Saprolite_Post static	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post static	Shear/Normal Fn.	21			0	F6_Post static			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0				



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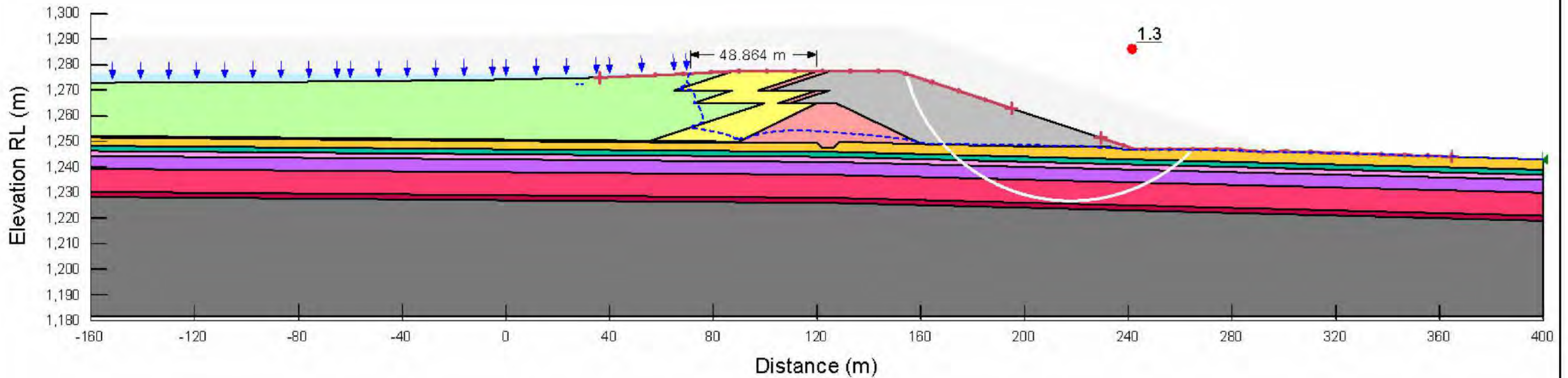
Notes:
 Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section A Sensitivity RL 1277.5 m
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 8

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Red	Slope_Clay starter/face_Post seismic	SHANSEP	19					0.44	10	
Yellow	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Green	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Pallid saprolite_Post seismic	SHANSEP	17					0.264	10	
Purple	Slope_F4 - soft layer_Post seismic	SHANSEP	17					0.328	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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Notes:

Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

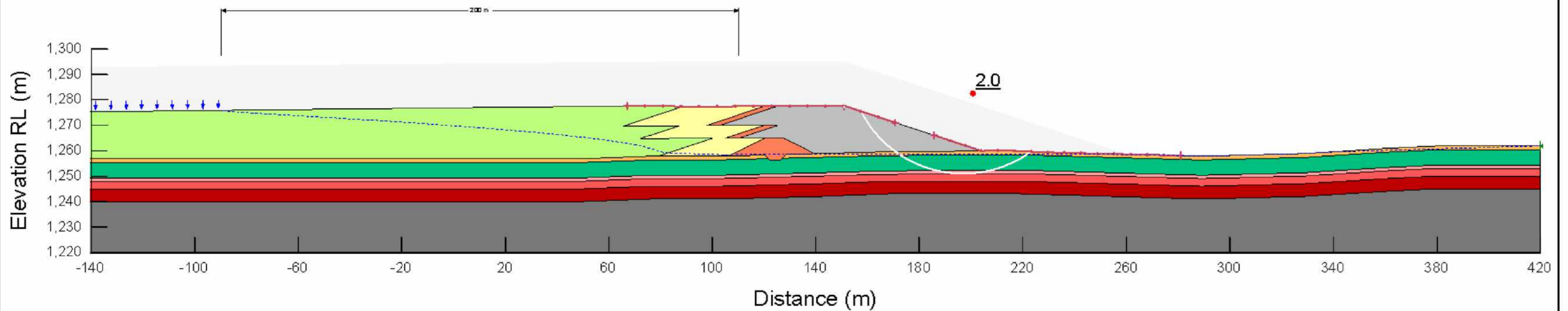
Section: Cell 1 - Section A Sensitivity RL 1277.5 m

Case: Post Seismic Circular

Project Nro. 12685165
Revision A
Date 30/01/2025

Figure 9

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterised saprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Pink	Slope_F3 - Pallid saprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Red	Slope_F5 - Saprolite_Longterm	Shear/Normal Fn.	18			0	F5_Longterm	
Dark Red	Slope_F6 - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	F6_Longterm	
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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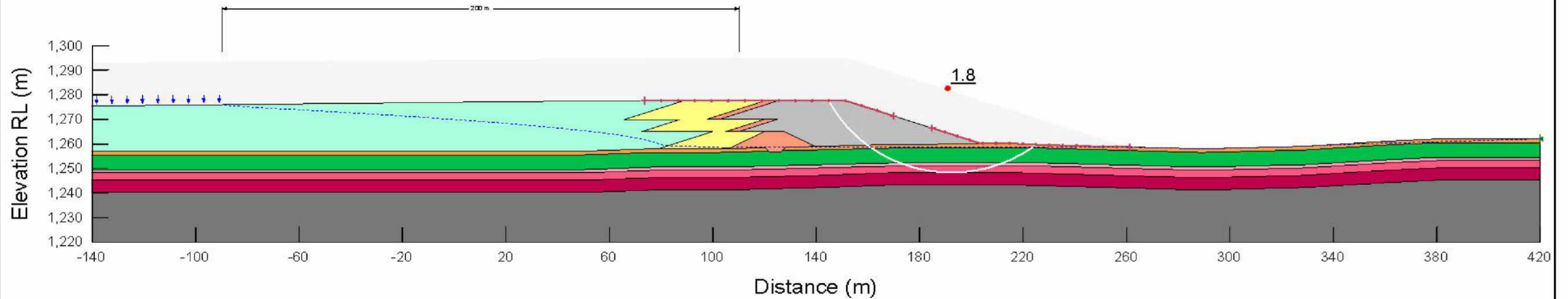
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section B RL 1277.5 m
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 10

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Post static	SHANSEP	19					0.55	10	
Light Orange	Slope_F1 - laterite - sandy gravel_Post static	Shear/Normal Fn.	21			0	F1_Post static			
Green	Slope_F2 - Laterized saprolite_Post static	Shear/Normal Fn.	20			0	F2_Post static			
Pink	Slope_F3 - Pallid saprolite_Post static	SHANSEP	17					0.33	10	
Red	Slope_F5 - Saprolite_Post static	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post static	Shear/Normal Fn.	21			0	F6_Post static			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0				



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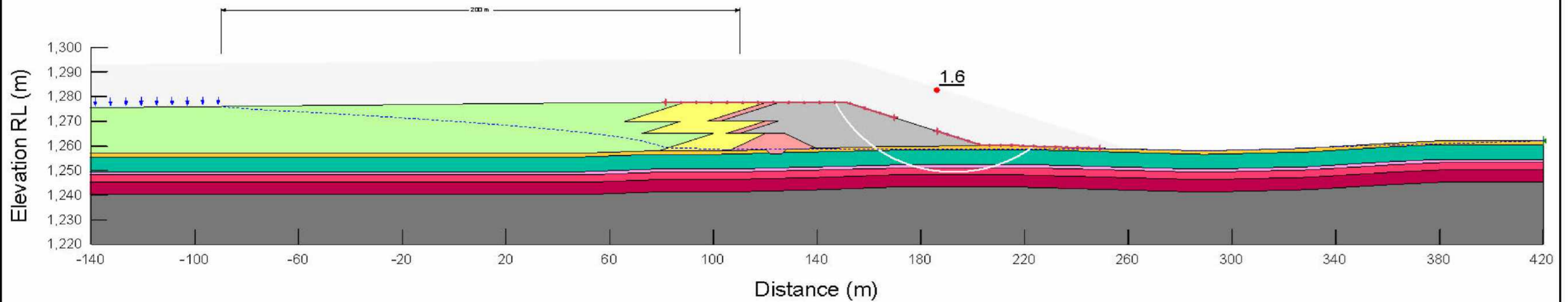
Notes:

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 - Section B RL 1277.5 m
Case: Post Static Circular

Project Nro. 12685165
Revision A
Date 30/01/2025

Figure 11

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Red	Slope_Clay starter/face_Post seismic	SHANSEP	19					0.44	10	
Orange	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Green	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Pallid saprolite_Post seismic	SHANSEP	17					0.264	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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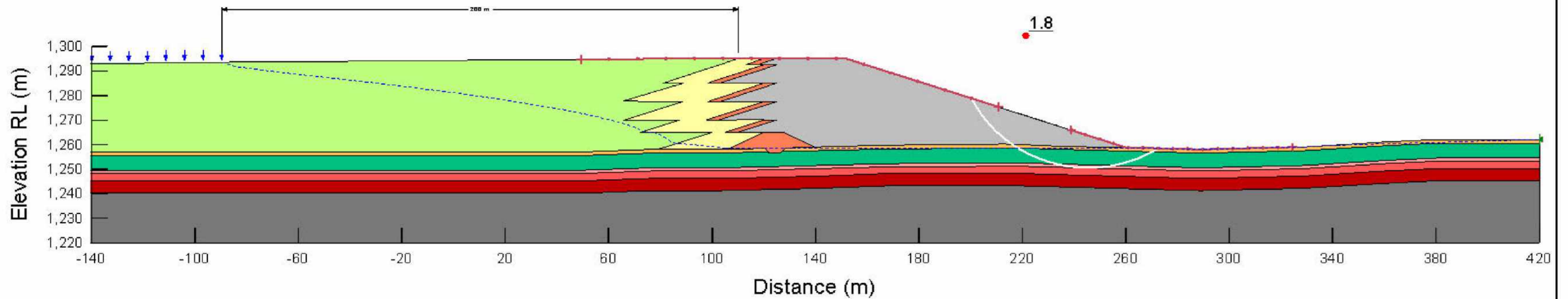
Notes:

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 - Section B RL 1277.5 m
Case: Post Seismic Circular

Project Nro.: 12685165
Revision: A
Date: 30/01/2025

Figure 12

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterised saprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Pink	Slope_F3 - Pallid saprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Red	Slope_F6 - Saprolite_Longterm	Shear/Normal Fn.	18			0	F6_Longterm	
Dark Red	Slope_F6 - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	F6_Longterm	
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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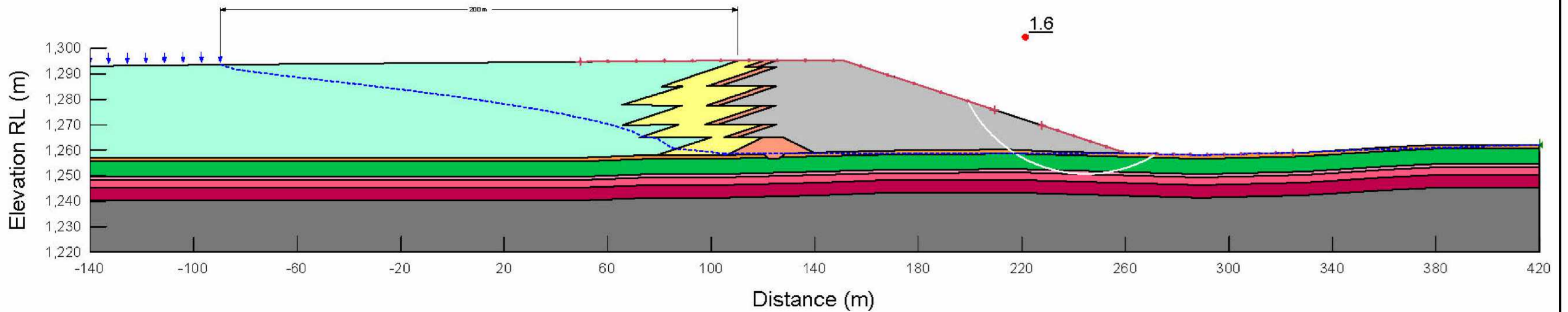
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section B RL 1295 m
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 13

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Post static	SHANSEP	19					0.55	10	
Light Orange	Slope_F1 - laterite - sandy gravel_Post static	Shear/Normal Fn.	21			0	F1_Post static			
Green	Slope_F2 - Laterised saprolite_Post static	Shear/Normal Fn.	20			0	F2_Post static			
Pink	Slope_F3 - Pallid saprolite_Post static	SHANSEP	17					0.33	10	
Red	Slope_F5 - Saprolite_Post static	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post static	Shear/Normal Fn.	21			0	F6_Post static			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Blue	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0				



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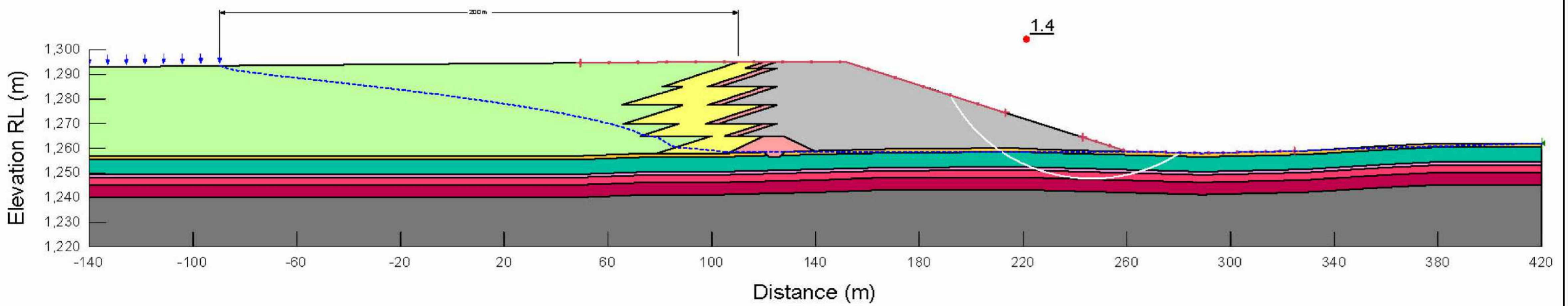
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section B RL 1295 m
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 14

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Light Red	Slope_Clay starter/face_Post seismic	SHANSEP	19					0.44	10	
Yellow	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Teal	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Pallid saprolite_Post seismic	SHANSEP	17					0.264	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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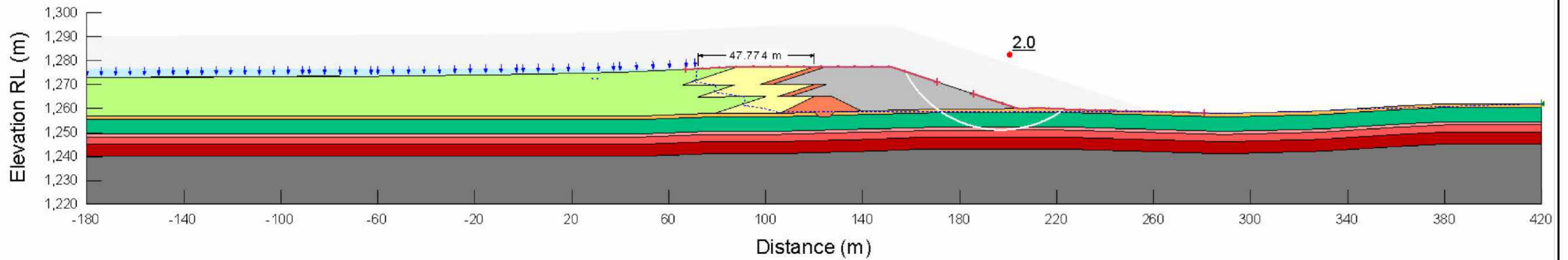
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 - Section B RL 1295 m
 Case: Post Seismic Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 15

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24					500
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0		
Yellow	Slope_F1 - laterite - sandy gravel_Longterm	Shear/Normal Fn.	21			0	F1_Longterm	
Green	Slope_F2 - Laterised saprolite_Longterm	Shear/Normal Fn.	20			0	F2_Longterm	
Light Red	Slope_F3 - Pallid saprolite_Longterm	Shear/Normal Fn.	17			0	F3_Longterm	
Red	Slope_F5 - Saprolite_Longterm	Shear/Normal Fn.	18			0	F5_Longterm	
Dark Red	Slope_F6 - Extremely weathered rock_Longterm	Shear/Normal Fn.	21			0	F6_Longterm	
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0		
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0		
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0		



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Notes:

Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

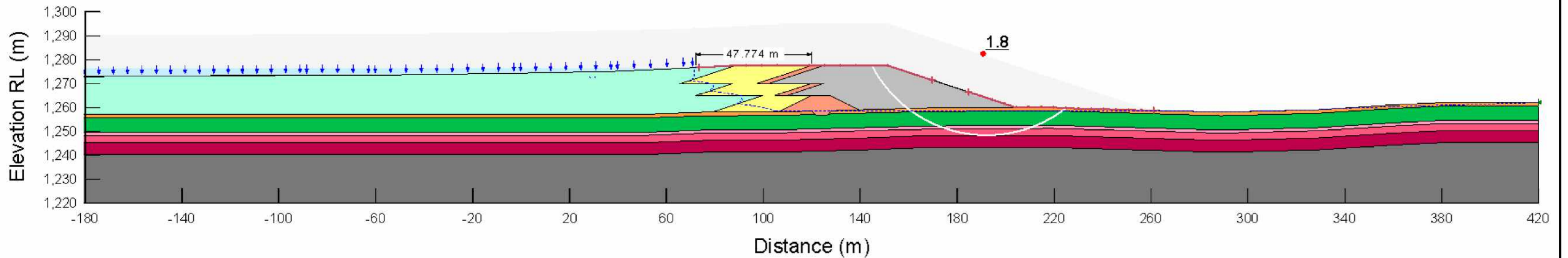
Section: Cell 1 - Section B Sensitivity RL 1277.5 m

Case: Long Term Circular

Project Nro.	12685165
Revision	A
Date	30/01/2025

Figure 16

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Orange	Slope_Clay starter/face_Post static	SHANSEP	19					0.55	10	
Light Orange	Slope_F1 - laterite - sandy gravel_Post static	Shear/Normal Fn.	21			0	F1_Post static			
Green	Slope_F2 - Laterised saprolite_Post static	Shear/Normal Fn.	20			0	F2_Post static			
Pink	Slope_F3 - Pallid saprolite_Post static	SHANSEP	17					0.33	10	
Red	Slope_F5 - Saprolite_Post static	SHANSEP	18					0.43	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post static	Shear/Normal Fn.	21			0	F6_Post static			
Light Grey	Slope_Mine waste rock (embankment)	Mohr-Coulomb	23	0	40	0				
Light Blue	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1					0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0				



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Notes:

Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

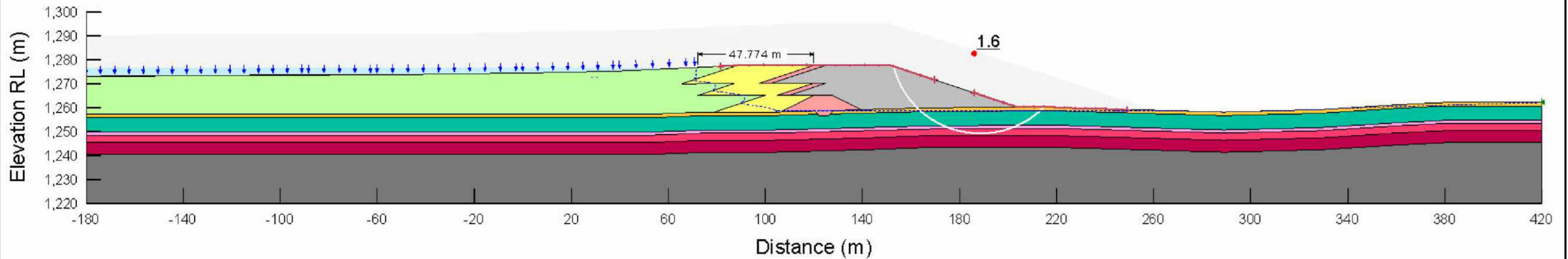
Section: Cell 1 - Section B Sensitivity RL 1277.5 m

Case: Post Static Circular

Project Nro.	12685165
Revision	A
Date	30/01/2025

Figure 17

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Shear/Normal Strength Fn.	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Grey	Slope_Bedrock (Dolerite)	Undrained (Phi=0)	24							500
Red	Slope_Clay starter/Face_Post seismic	SHANSEP	19					0.44	10	
Yellow	Slope_F1 - laterite - sandy gravel_Post seismic	Shear/Normal Fn.	21			0	F1_Post seismic			
Green	Slope_F2 - Laterised saprolite_Post seismic	Shear/Normal Fn.	20			0	F2_Post seismic			
Pink	Slope_F3 - Pallid saprolite_Post seismic	SHANSEP	17					0.264	10	
Red	Slope_F5 - Saprolite_Post seismic	SHANSEP	18					0.344	10	
Dark Red	Slope_F6 - Extremely weathered rock_Post seismic	Shear/Normal Fn.	21			0	F6_Post seismic			
Light Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0				
Light Green	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1					0.08	10	
Yellow	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0				



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Notes:

Sensitivity with realistic beach slopes
 and pond at MOL

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

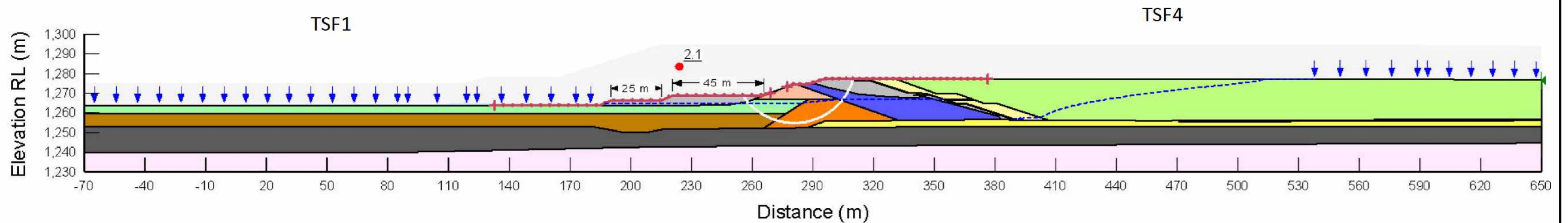
Section: Cell 1 - Section B Sensitivity RL 1277.5 m

Case: Post Seismic Circular

Project Nro. 12685165
Revision A
Date 30/01/2025

Figure 18

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Undrained Shear Strength (kPa)
Light Purple	Slope_bedrock	Undrained (Phi=0)	24				450
Orange	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0	
Brown	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0	
Light Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0	
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0	
Dark Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0	
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0	
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0	
Light Green	Slope_Loose tailings	Mohr-Coulomb	19	0	25	0	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0	
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0	
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0	



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Notes:

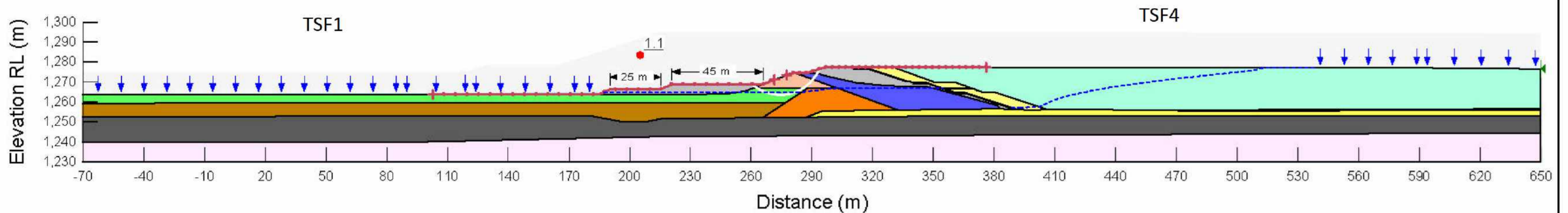
Disclaimer: Buttressing on TSF1 side shall be subject to additional assessment of the loading rate to support dissipation of pore water pressures

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 interfase with TSF1 - Section C RL 1277.5 m
Case: Long Term Circular

Project Nro. 12685165
Revision A
Date 30/01/2025

Figure 19

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Light Pink	Slope_bedrock	Undrained (Phi=0)	24						450
Light Orange	Slope_Clay starter/face_Post static	SHANSEP	19				0.55	10	
Dark Orange	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0			
Light Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
Light Green	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
Light Cyan	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1				0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0			



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Notes:

Disclaimer: Buttressing on TSF1 side shall be subject to additional assessment of the loading rate to support dissipation of pore water pressures

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

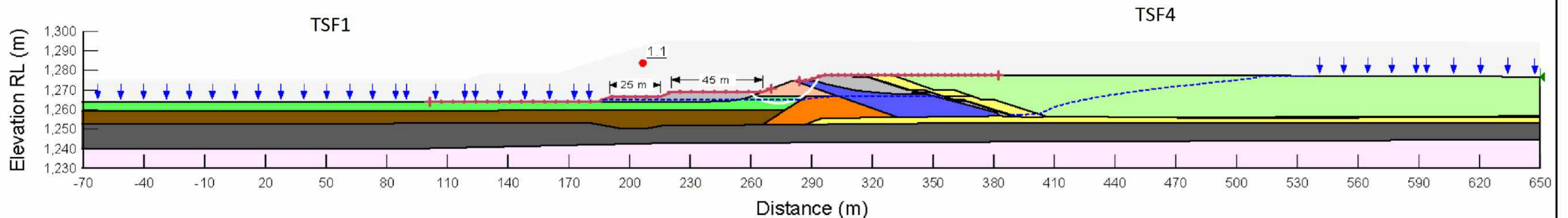
Section: Cell 1 interfase with TSF1 - Section C RL 1277.5 m

Case: Post Static Circular

Project No.: 12685165
Revision: A
Date: 30/01/2025

Figure 20

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
	Slope_bedrock	Undrained (Phi=0)	24						450
	Slope_Clay starter/face_Post seismic	SHANSEP	19				0.44	10	
	Slope_Clayey Tailings_Post seismic	SHANSEP	20				0.09	10	
	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1				0.08	10	
	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0			



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Notes:

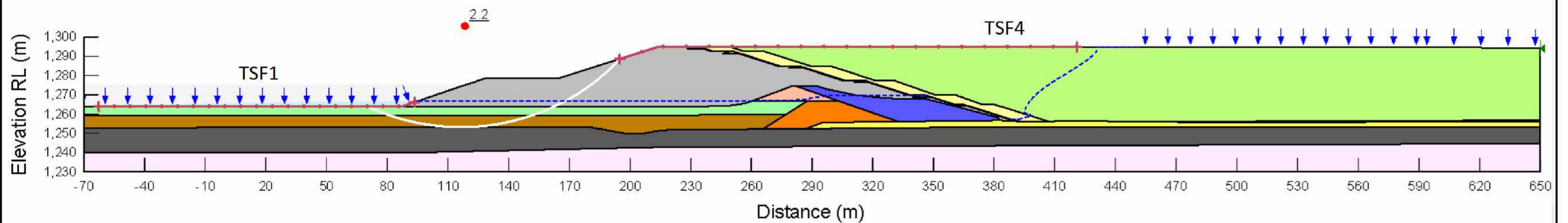
Disclaimer: Buttressing on TSF1 side shall be subject to additional assessment of the loading rate to support dissipation of pore water pressures

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 interfase with TSF1 - Section C RL 1277.5 m
Case: Post Seismic Circular

Project Nro.: 12685165
Revision: A
Date: 30/01/2025

Figure 21

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Undrained Shear Strength (kPa)
Light Pink	Slope_bedrock	Undrained (Phi=0)	24				450
Red	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0	
Brown	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0	
Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0	
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0	
Dark Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0	
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0	
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0	
Light Green	Slope_Loose tailings	Mohr-Coulomb	19	0	25	0	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0	
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0	
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0	



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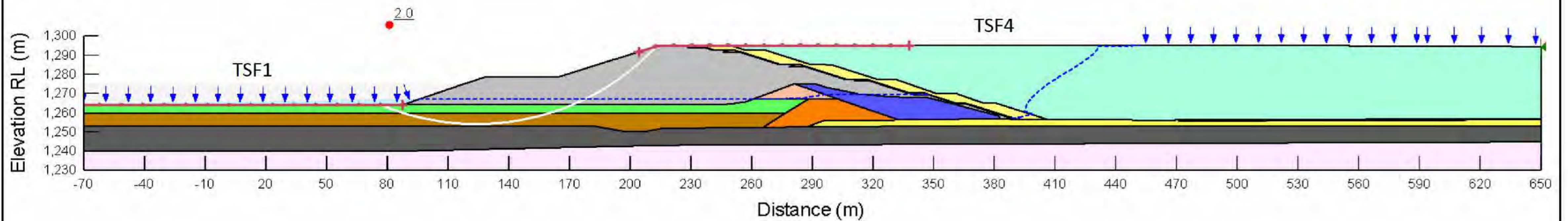
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C RL 1295 m
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 22

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
	Slope_bedrock	Undrained (Phi=0)	24						450
	Slope_Clay starter/face_Post static	SHANSEP	19				0.55	10	
	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0			
	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1				0.08	5	
	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0			



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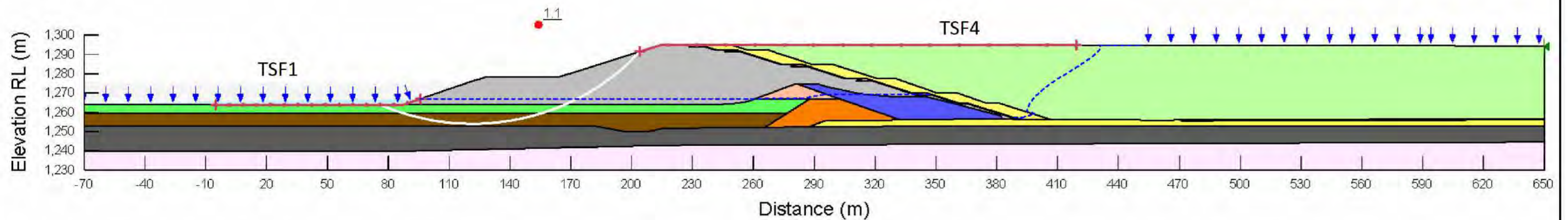
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C RL 1295 m
 Case: Post Static Circular

Project No. 12685165
 Revision A
 Date 30/01/2025

Figure 23

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
	Slope_bedrock	Undrained (Phi=0)	24						450
	Slope_Clay starter/face_Post seismic	SHANSEP	19				0.44	10	
	Slope_Clayey Tailings_Post seismic	SHANSEP	20				0.09	10	
	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1				0.08	10	
	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0			



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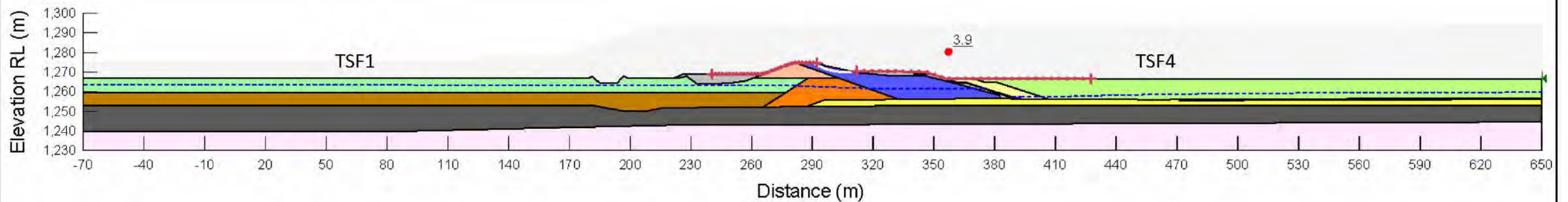
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C RL 1295 m
 Case: Post Seismic Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 24

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Undrained Shear Strength (kPa)
Light Pink	Slope_bedrock	Undrained (Phi=0)	24				450
Red	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0	
Brown	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0	
Light Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0	
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0	
Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0	
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0	
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0	
Light Green	Slope_Loose tailings	Mohr-Coulomb	19	0	25	0	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0	
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0	
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0	



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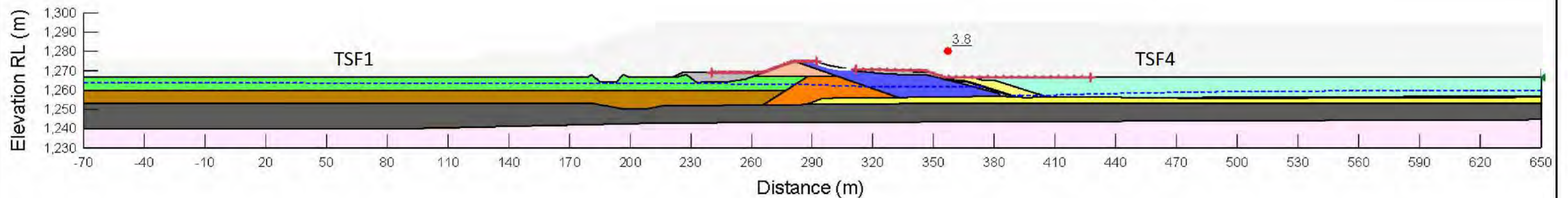
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C
 Existing state plus buttress
 Case: Long Term Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 25

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
	Slope_bedrock	Undrained (PH=0)	24						450
	Slope_Clay starter/face_Post static	SHANSEP	19				0.55	10	
	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0			
	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
	Slope_Tailings (Inner sandy silt)_Post static	SHANSEP	14.1				0.08	5	
	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0			



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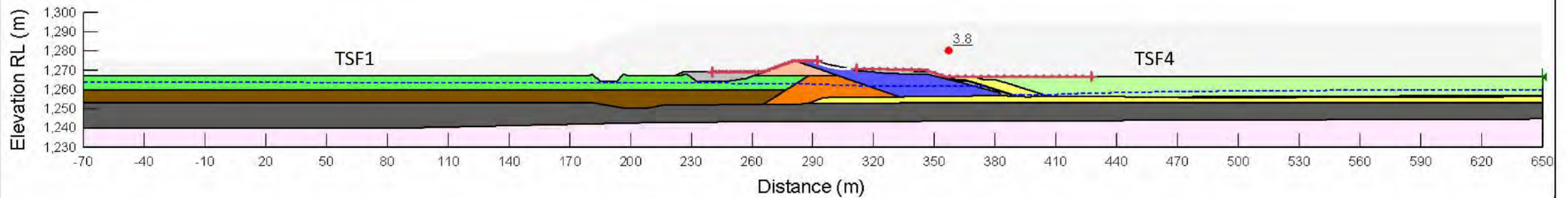
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C
 Existing state plus buttress
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 26

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
	Slope_bedrock	Undrained (Phi=0)	24						450
	Slope_Clay starter/face_Post seismic	SHANSEP	19				0.44	10	
	Slope_Clayey Tailings_Post seismic	SHANSEP	20				0.09	10	
	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
	Slope_Tailings (inner sandy silt)_Post seismic	SHANSEP	14.1				0.08	10	
	Slope_Tailings (outer beach sand)_Post seismic	Mohr-Coulomb	16	0	22.4	0			



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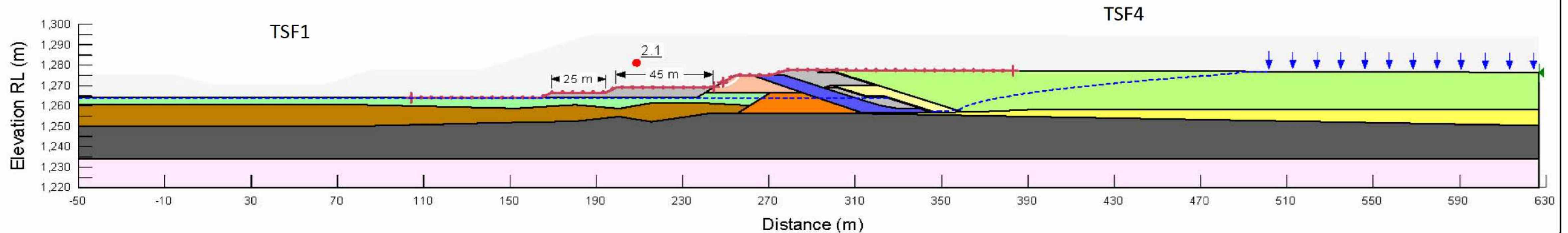
Notes:

Client: Talison Lithium Pty Ltd
 Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
 Section: Cell 1 interfase with TSF1 - Section C
 Existing state plus buttress
 Case: Post Static Circular

Project Nro. 12685165
 Revision A
 Date 30/01/2025

Figure 27

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Undrained Shear Strength (kPa)
Light Pink	Slope_bedrock	Undrained (Phi=0)	24				450
Red	Slope_Clay starter/face_Longterm	Mohr-Coulomb	19	0	28	0	
Brown	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0	
Light Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0	
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0	
Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0	
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0	
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0	
Light Green	Slope_Loose tailings	Mohr-Coulomb	19	0	25	0	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0	
Light Green	Slope_Tailings (inner sandy silt)_Longterm	Mohr-Coulomb	14.1	0	26	0	
Yellow	Slope_Tailings (outer beach sand)_Longterm	Mohr-Coulomb	16	0	28	0	



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Notes:

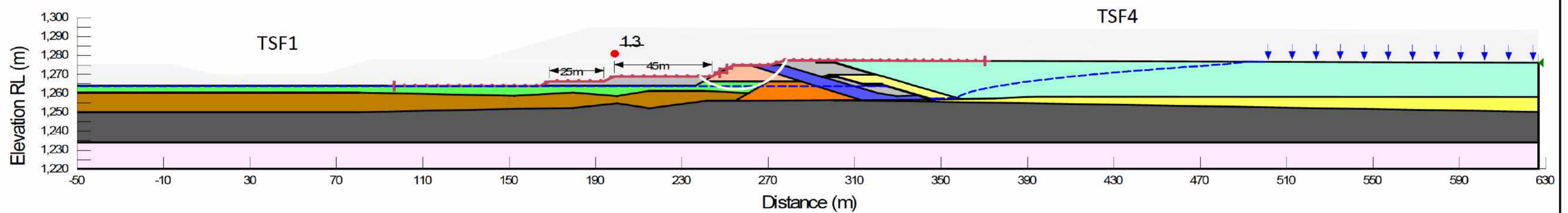
Disclaimer: Buttressing on TSF1 side shall be subject to additional assessment of the loading rate to support dissipation of pore water pressures

Client: Talison Lithium Pty Ltd
Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m
Section: Cell 1 interfase with TSF1 - Section D RL 1277.5 m
Case: Long Term Circular

Project Nro. 12685165
Revision A
Date 30/01/2025

Figure 28

Color	Name	Slope Stability Material Model	Unit Weight (kN/m ³)	Effective Cohesion (kPa)	Effective Friction Angle (°)	Phi-B (°)	Tau/Sigma Ratio	Minimum Shear Strength (kPa)	Undrained Shear Strength (kPa)
Light Pink	Slope_bedrock	Undrained (Phi=0)	24						450
Light Orange	Slope_Clay starter/face_Post static	SHANSEP	19				0.55	10	
Brown	Slope_Clayey Tailings_Longterm	Mohr-Coulomb	20	0	20	0			
Light Orange	Slope_Compacted Tailings Embankment_Long term	Mohr-Coulomb	19	10	30	0			
Yellow	Slope_Dredge deposit (MH)	Mohr-Coulomb	21	0	30	0			
Orange	Slope_Earthfill (CL-ML)	Mohr-Coulomb	21	40	5	0			
Blue	Slope_Embankment Fill (rockfill)	Mohr-Coulomb	21	0	40	0			
Dark Grey	Slope_Foundation (residual soil_clayey silt_MH)_long term	Mohr-Coulomb	20	0	29	0			
Light Green	Slope_Loose tailings_Post seismic	SHANSEP	19				0.07	10	
Grey	Slope_Mine waste rock (embankments)	Mohr-Coulomb	23	0	40	0			
Light Cyan	Slope_Tailings (inner sandy silt)_Post static	SHANSEP	14.1				0.08	5	
Yellow	Slope_Tailings (outer beach sand)_Post static	Mohr-Coulomb	16	0	28	0			



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Notes:

Disclaimer: Buttressing on TSF1 side shall be subject to additional assessment of the loading rate to support dissipation of pore water pressures

Client: Talison Lithium Pty Ltd

Project: TSF4 Cell 1 Redesign to a height of RL 1277.5 m

Section: Cell 1 interfase with TSF1 - Section D RL 1277.5 m

Case: Post Static Circular

Project Nro.: 12685165
Revision: A
Date: 30/01/2025

Figure 29